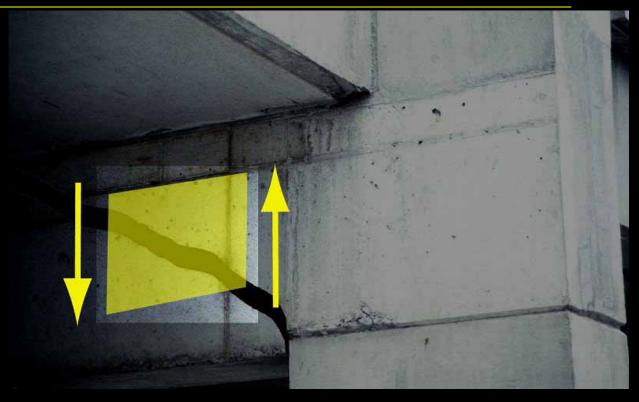
# ARCHITECTURAL STRUCTURES I:

STATICS AND STRENGTH OF MATERIALS

**ENDS 231** 

DR. ANNE NICHOLS
SPRING 2008

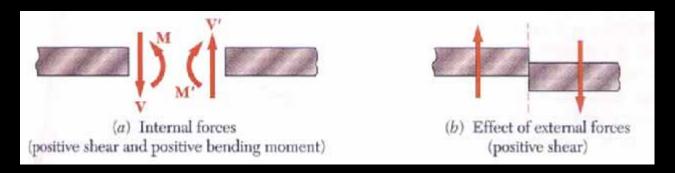
twenty



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# beams: shear stress

### Transverse Loading and Shear



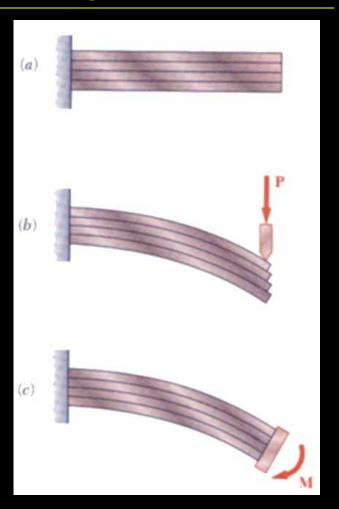
- perpendicular loading
- internal shear
- along with bending moment

# Bending vs. Shear in Design

 bending stresses dominate

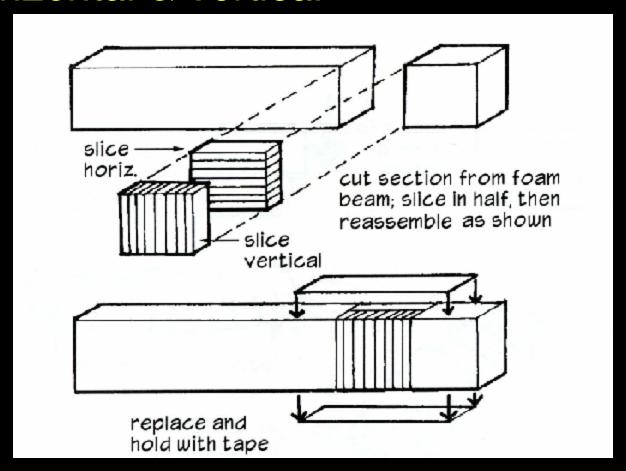
 shear stresses exist horizontally with shear

 no shear stresses with pure bending



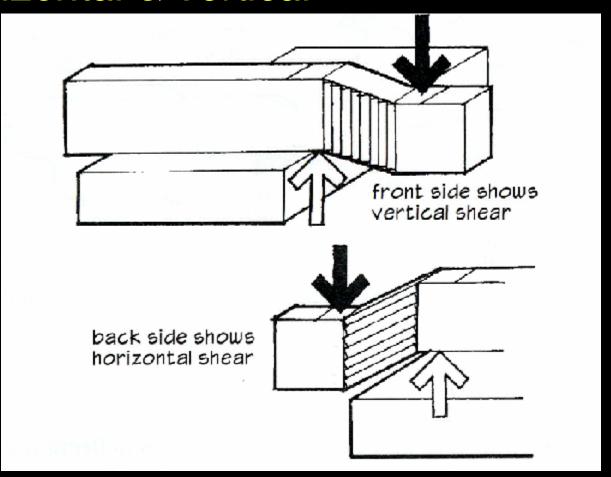
### Shear Stresses

#### horizontal & vertical



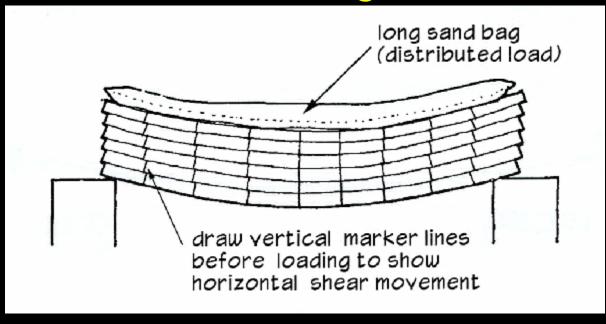
### Shear Stresses

### horizontal & vertical



### Beam Stresses

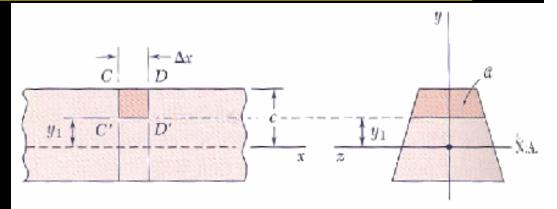
horizontal with bending

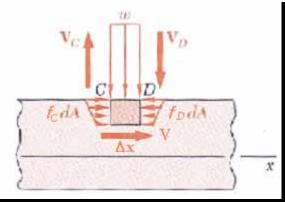


# **Equilibrium**

horizontal force V needed

$$V_{longitudinal} = \frac{V_T Q}{I} \Delta x$$



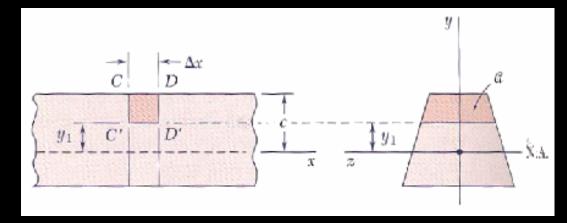


Q is a moment area

#### Moment of Area

 Q is a moment area with respect to the n.a. of area <u>above or below</u> the horizontal

 Q<sub>max</sub> at y=0 (neutral axis)



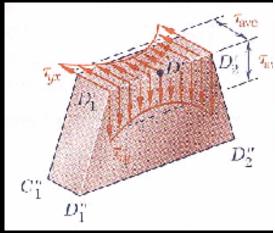
q is shear flow:

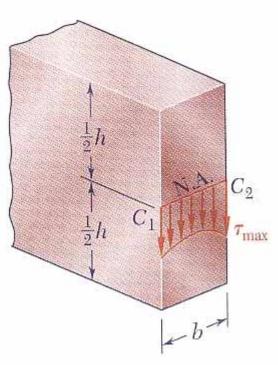
$$q = rac{V_{longitudinal}}{\Delta x} = rac{V_{T}Q}{I}$$

# Shearing Stresses

$$f_{v} = \frac{V}{\Delta A} = \frac{V}{b \cdot \Delta x}$$

$$f_{v-ave} = \frac{VQ}{Ib}$$





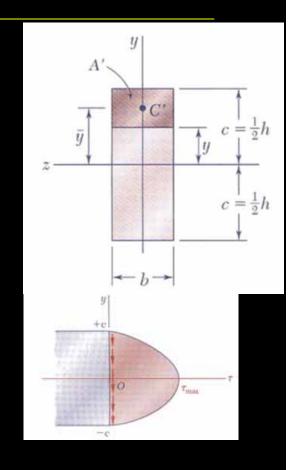
- $f_{v-ave} = 0$  on the top/bottom
- b min may not be with Q max
- with  $h/4 \ge b$ ,  $f_{v-max} \le 1.008 f_{v-ave}$

# Rectangular Sections

$$I = \frac{bh^3}{12} \qquad Q = A\overline{y} = \frac{bh^2}{8}$$

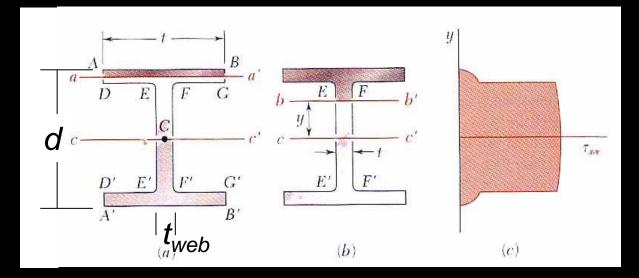
$$f_{v} = \frac{VQ}{Ib} = \frac{3V}{2A}$$

•  $f_{v\text{-max}}$  occurs at n.a.



#### Steel Beam Webs

- W and S sections
  - b varies

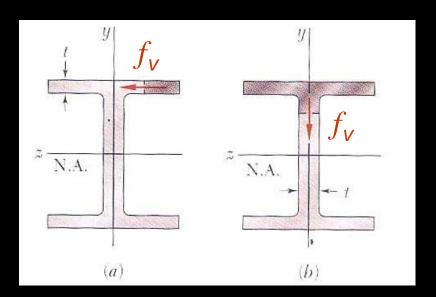


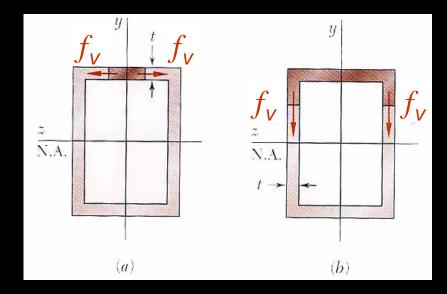
- stress in flange negligible
- presume constant stress in web

$$f_{v-\text{max}} = \frac{3V}{2A} \approx \frac{V}{A_{web}}$$

### Shear Flow

- loads applied in plane of symmetry
- cut made perpendicular  $q = \frac{VQ}{I}$

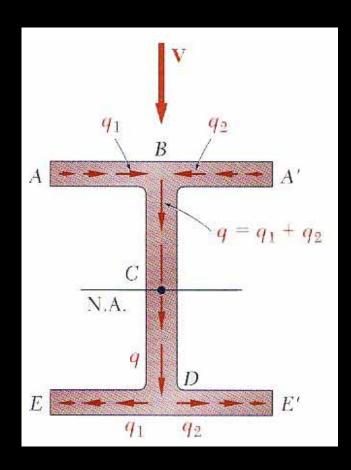




# Shear Flow Quantity

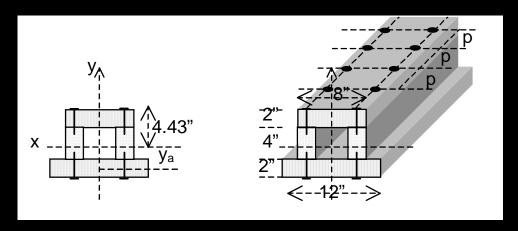
sketch from Q

$$q = \frac{VQ}{I}$$



## Connectors Resisting Shear

- plates with
  - nails
  - rivets
  - bolts
- splices



$$rac{V_{longitudinal}}{p} = rac{VQ}{I}$$
 $nF_{connector} \geq rac{VQ_{connected\ area}}{I} \cdot p$ 

### Vertical Connectors

• isolate an area with vertical interfaces

$$nF_{connector} \ge \frac{VQ_{connected\ area}}{I} \cdot p$$

## Unsymmetrical Shear or Section

- member can bend and twist
  - not symmetric
  - shear not in that plane
- shear center
  - moments balance

