# PRACTICE FINAL EXAMINATION updated 12/10/07

## **ENDS 231. Practice Final Examination**

Aids Allowed: Two marked 8.5" x 11" crib pages

(2 pages written on one side only or 1 page written on both sides)

Silent **non-programmable** Calculator Reference Formulas

(Provided at exam time)

Clearly show all your work and record your final answers with the units specified in the boxes.

# Problem 1) Worth 45%

- A 12 ft beam with live and dead loading is shown in Figure 1a.
- The cross section geometry of the beam is shown in Figure 1c.
- The material is Giggium steel with E=32,000 ksi,  $F_b=60$  ksi,  $F_v=35$  ksi,  $F_y=45$  ksi, and  $F_u=65$  ksi.
- The beam end connection is detailed in Figures 1d and 1e. The angle legs are 3/8" thick, with 7/8 in diameter bolts of A325-N and standard holes.
- The weld material is E70XX

#### FIND:

- a) The completed bending moment diagram in Figure 1b, and  $M_{max}$ .
- b) The moment of inertia for the cross section by completing the chart of Figure 1f.
- c) The maximum bending stress in the beam.
- d) The maximum shear stress in the beam.
- e) The deflection at the free end *D* due to *live load only*.
- f) The number of bolts required for shear at the beam shear connection at end A.
- g) The bearing force allowed at the beam shear connection with 4 bolts.
- h) The minimum weld size required for shear for the angles to the column if the length on each angle is 1.5 in..

Figure 1f		A (in²)	I <sub>x</sub> (in <sup>4</sup> )	d <sub>y</sub> (in)	Ad <sub>y</sub> <sup>2</sup> (in <sup>3</sup> )	
Figure 1f.	semicircle					
	web	12.75	76.77	2.31	68.04	

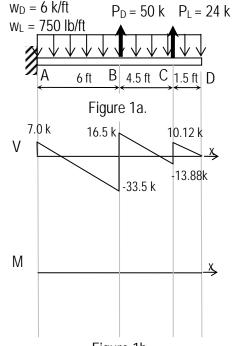


Figure 1b.

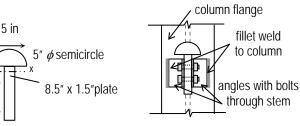


Figure 1d. (elevation)

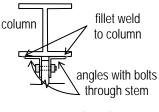


Figure 1e. (plan)

a)		b)	248.6 in <sup>4</sup>	c)	
d)	2.9 ksi	e)	-2.02 in (up)	f) 1 bolt (0.28 required)	
g)		0	Disclaimer: Ans	wers have NOT been painstakingly researd	ched.

Figure 1c.

.100 lb

800 lb

2 bolts

## Problem 2) Worth 49%

- A parallel chord truss is shown in the Figure 2a has the following support reactions:
  - $A_x = 100 \text{ lb}, A_y = 267.5 \text{ lb}, E = -267.5 \text{ lb}.$
- Wind load is considered.
- The truss is constructed with glu-lam lumber having E =1.85 x  $10^6$  psi,  $\alpha = 3.8 \times 10^{-6}$  /°F,  $F_c = 1700$  psi (*no* adjustment factors applied), allowable tension stress  $F'_t = 1200$  psi (adjustment factors applied), and allowable bearing stress  $F'_p = 650$  psi (adjustment factors applied).
- The truss is constructed with 5.125"x10.5" timbers with  $I_x = 109.9 \text{ in}^4$ , and  $I_v = 19.1 \text{ in}^4$ .
- The bottom chord that is continuous the length of the truss is connected as shown in Figure 2b. The joints are considered pinned for analysis.
- The bottom chord is laterally braced at each end and across the span as show in Figure 2c.

#### FIND:

- i) The member forces in <u>AB and AH</u> using the method of joints.
- j) The member forces in <u>BG and BC</u> using the method of sections.
- A H G F E

  11 ft 12 ft-4 in. 12 ft-8 in.

Figure 2b.

600 lb

4 @ 9 ft = 36 ft

Figure 2a.

10.5 in <sup>ქ</sup>

В

 $H \downarrow_{200 \text{ lb}} G$ 

- Figure 2c. (top view of bracing)
- k) The minimum area required for member HG if the tension force is 6.3 kips.
- 1) The stress value and type in member AB resulting *only* from a temperature decrease of 15° F if the member can only shorten by 0.003 inches.
- m) The critical value of  $F_{CE}$  for the lower chord.
- n) The allowable buckling load for the lower chord if  $C_p$  has been determined to be 0.310 for the weak axis and 0.167 for the strong axis.
- o) The minimum bolt diameter allowed with two bolts if the maximum tension force in either member in the connection is 7.5 kips and the hole is 1/16" larger than the bolt. (hint –net area and bearing area are important)

j)		j)	k)	
1)	63.4 psi tension	m)	n)	24,444 lb
o)	1.13 in			