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beams: shear stress

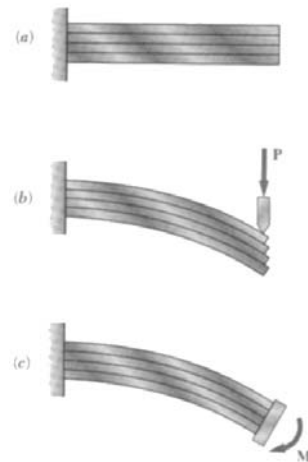
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Bending vs. Shear in Design

- bending stresses dominate
- shear stresses exist horizontally with shear
- no shear stresses with pure bending

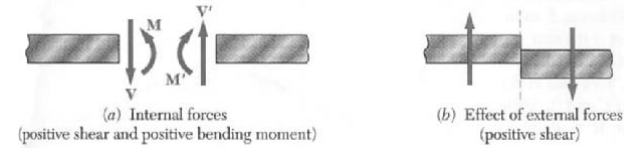


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Transverse Loading and Shear



- perpendicular loading
- internal shear
- along with bending moment

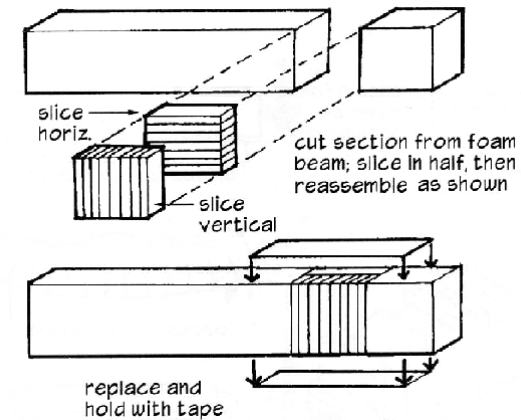
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Shear Stresses

- horizontal & vertical



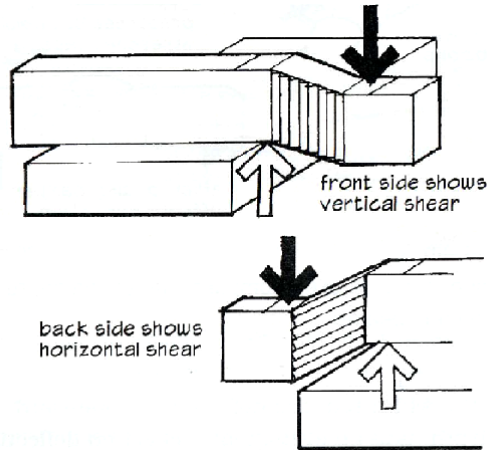
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Shear Stresses

- horizontal & vertical



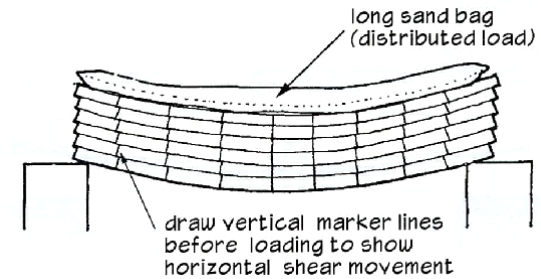
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Beam Stresses

- horizontal with bending



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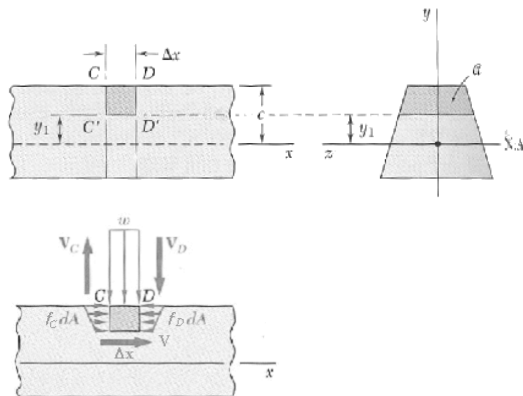
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Equilibrium

- horizontal force V needed

$$V_{longitudinal} = \frac{V_T Q}{I} \Delta x$$



- Q is a moment area

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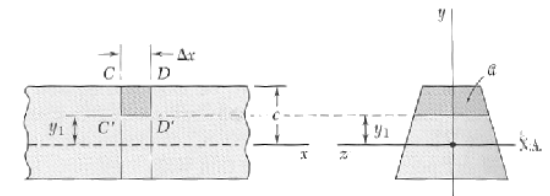
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Moment of Area

- Q is a moment area with respect to the n.a. of area above or below the horizontal

- Q_{max} at $y=0$ (n.a.)



- q is shear flow:
$$q = \frac{V_{longitudinal}}{\Delta x} = \frac{V_T Q}{I}$$

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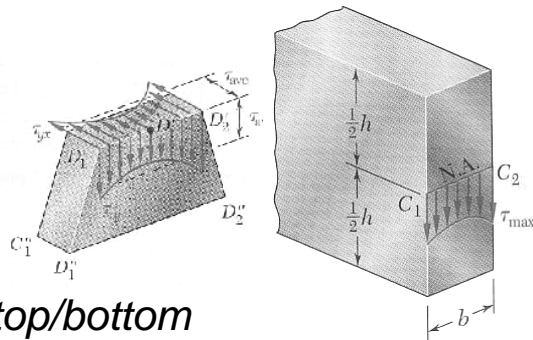
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Shearing Stresses

$$f_v = \frac{V}{\Delta A} = \frac{V}{b \cdot \Delta x}$$

$$f_{v-ave} = \frac{VQ}{Ib}$$



- $f_{v-ave} = 0$ on the top/bottom
- b min may not be with Q max
- with $h/4 \geq b$, $f_{v-max} \leq 1.008 f_{v-ave}$

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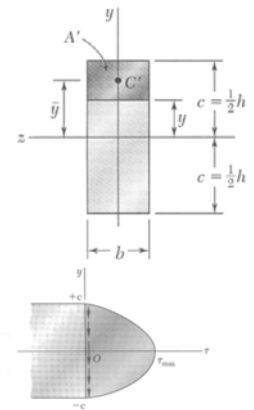
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Rectangular Sections

$$I = \frac{bh^3}{12} \quad Q = A\bar{y} = bh^2/8$$

$$f_v = \frac{VQ}{Ib} = \frac{3V}{2A}$$

- f_{v-max} occurs at n.a.



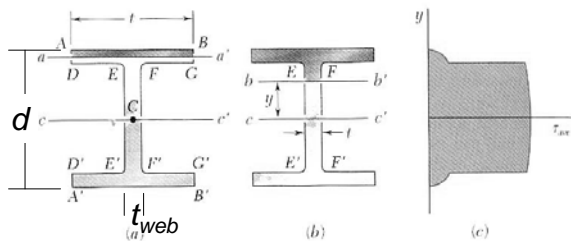
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Steel Beam Webs

- W and S sections
- b varies



– stress in flange negligible

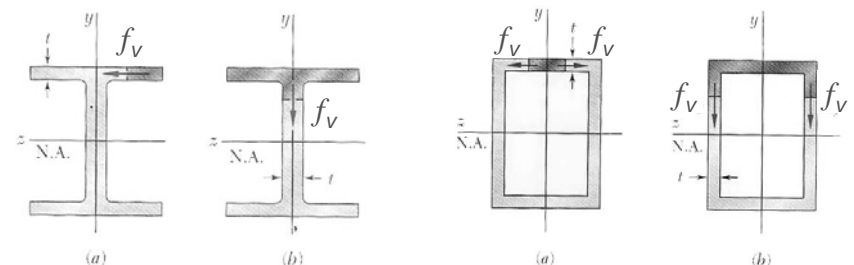
– presume constant stress in web

$$f_{v-max} = \frac{3V}{2A} \approx \frac{V}{A_{web}}$$

Shear Flow

- loads applied in plane of symmetry
- cut made perpendicular

$$q = \frac{VQ}{I}$$



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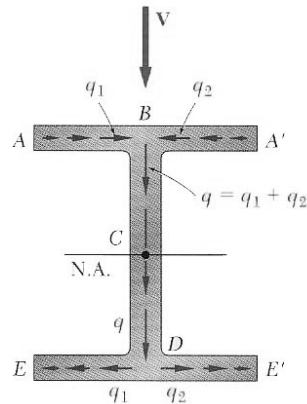
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Shear Flow Quantity

- sketch from Q

$$q = \frac{VQ}{I}$$



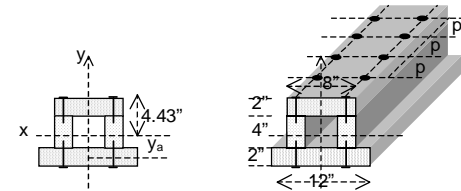
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Connectors Resisting Shear

- plates with
 - nails
 - rivets
 - bolts
- splices



$$\frac{V_{longitudinal}}{p} = \frac{VQ}{I}$$

$$nF_{connector} \geq \frac{VQ_{connected\ area}}{I} \cdot p$$

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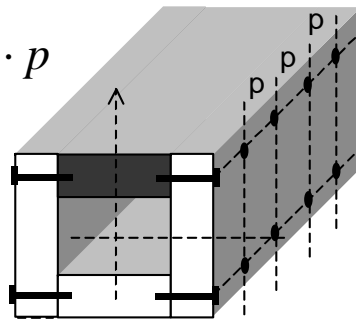
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Vertical Connectors

- isolate an area with vertical interfaces

$$nF_{connector} \geq \frac{VQ_{connected\ area}}{I} \cdot p$$



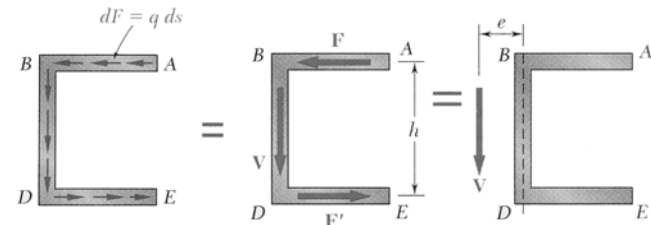
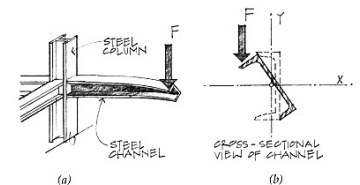
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Unsymmetrical Shear or Section

- member can bend and twist
 - not symmetric
 - shear not in that plane
- shear center
 - moments balance



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