ARCHITECTURAL STRUCTURES I:

STATICS AND STRENGTH OF MATERIALS

ENDS 231

DR. ANNE NICHOLS SPRING 2007

lecture seventeen



torsion & thermal effects

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Shear Stress Distribution

- depend on the deformation
- ϕ = angle of twist - measure
- can prove planar section doesn't distort



r=RADIUS

& =ANGLE

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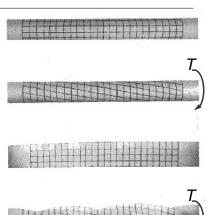
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Torsional Stress & Strain

 can see torsional stresses & twisting of axi-symmetrical cross sections



- remain plane
- undistorted
- rotates
- not true for square sections....



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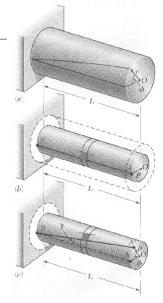
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Shearing Strain

• related to ϕ

$$\gamma = \frac{\rho \phi}{L}$$

- ρ is the radial distance from the centroid to the point under strain
- shear strain varies linearly along the radius: γ_{max} is at outer diameter



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Torsional Stress - Strain

• know $f_v = \tau = G \cdot \gamma$ and $\gamma = \frac{\rho \phi}{L}$

• so $\tau = G \cdot \frac{\rho \phi}{L}$

• where G is the Shear Modulus

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Shear Stress

- τ_{max} happens at <u>outer diameter</u>
- combined shear and axial stresses
 - maximum shear stress at 45° "twisted" plane







Torsional Stress - Strain

• from

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$$T = \Sigma \tau(\rho) \Delta A$$

• can derive



where J is the polar moment of inertia

elastic range



(b)

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Shear strain

- knowing $\tau = G \cdot \frac{\rho \phi}{L}$ and $\tau = \frac{T\rho}{J}$
- solve: $\phi = \frac{TL}{JG}$
- composite shafts: $\phi = \sum_{i} \frac{T_{i}L_{i}}{J_{i}G_{i}}$

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Noncircular Shapes

- torsion depends on J
- plane sections don't remain plane
- τ_{max} is still at outer diameter

$$\tau_{\text{max}} = \frac{T}{c_1 a b^2} \quad \phi = \frac{TL}{c_2 a b^3 G}$$

- where a is longer side (> b)

TORRUE TO HAPPING FY LAPPING

TABLE 3.1.	Coefficients for
Rectangular	Bars in Torsion

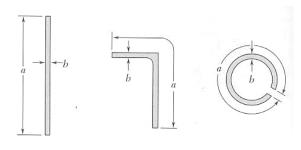
nectangular Dars III Tolston		
a/b	C 1	C ₂
1.0	0.208	0.1406
1.2	0.219	0.1661
1.5	0.231	0.1958
2.0	0.246	0.229
2.5	0.258	0.249
3.0	0.267	0.263
4.0	0.282	0.281
5.0	0.291	0.291
10.0	0.312	0.312
∞	0.333	0.333

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Open Thin-Walled Sections

with very large a/b ratios:



$$au_{\text{max}} = \frac{T}{\frac{1}{3}ab^2} \qquad \phi = \frac{TL}{\frac{1}{3}ab^3G}$$

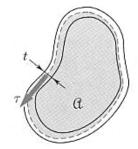
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Shear Flow in Closed Sections

q is the internal shear force/unit length

$$\tau = \frac{1}{2t\mathcal{Q}}$$

$$\phi = \frac{TL}{4t\mathcal{Q}^2} \sum_{i} \frac{s_i}{t_i}$$



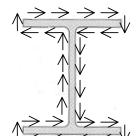
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- ullet ${\mathcal A}$ is the area bounded by the centerline
- s_i is the length segment, t_i is the thickness

Shear Flow in Open Sections

 each segment has proportion of T with respect to torsional rigidity,

$$\tau_{\text{max}} = \frac{Tt_{\text{max}}}{\frac{1}{3}\Sigma b_i t_i^3}$$



• total angle of twist:

$$\phi = \frac{TL}{\frac{1}{3}G\Sigma b_i t_i^3}$$

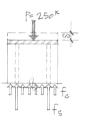
• I beams - web is thicker, so τ_{\max} is in web

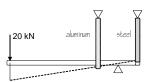
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Deformation Relationships

- physical movement
 - axially (same or zero)
 - rotations from axial changes





•
$$\delta = \frac{PL}{AE}$$
 relates δ to P

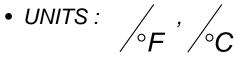
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Thermal Deformation

- α the rate of strain per degree



• length change:

$$\delta_T = \alpha(\Delta T)L$$

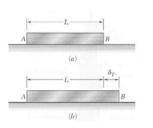
thermal strain:

$$\varepsilon_T = \alpha(\Delta T)$$

- no stress when movement allowed

Deformations from Temperature

- atomic chemistry reacts to changes in energy
- solid materials
 - can contract with decrease in temperature
 - can expand with increase in temperature
- linear change can be measured per degree



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Coefficients of Thermal Expansion

Material	Coefficients (α) [in	./in./°F]
Wood	3.0 x 10 ⁻⁶	
Glass	4.4 x 10 ⁻⁶	CONCRETE BEARING WALL
Concrete	5.5 x 10 ⁻⁶	JOINT
Cast Iron	5.9 x 10 ⁻⁶	No. of the last of
Steel	6.5 x 10 ⁻⁶	40
Wrought Iron	6.7 x 10 ⁻⁶	40
Copper	9.3 x 10 ⁻⁶	A P
Bronze	10.1 x 10 ⁻⁶	
Brass	10.4 x 10 ⁻⁶	
Aluminum	12.8 x 10 ⁻⁶	
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Stresses and Thermal Strains

- if thermal movement is restrained stresses are induced
- 1. bar pushes on supports

(b)

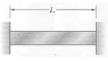
- 2. support pushes back
- 3. reaction causes internal

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Superposition Method



 total length change restrained to zero

constraint:
$$\delta_P + \delta_T = 0$$

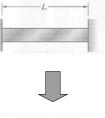
$$\delta_p = -\frac{PL}{AE}$$
 $\delta_T = \alpha (\Delta T)L$

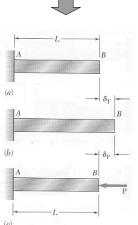
sub:
$$-\frac{PL}{AE} + \alpha (\Delta T)L = 0$$

$$f = -\frac{P}{A} = -\alpha (\Delta T)E$$

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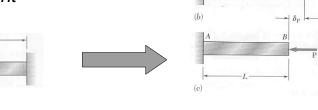
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Superposition Method

- can remove a support to make it look determinant
- replace the support with a reaction
- enforce the geometry constraint



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