ARCHITECTURAL STRUCTURES I:
STATICS AND STRENGTH OF MATERIALS
ENDS 231
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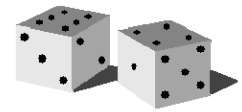
lecture
nineteen

LRFD design of steel beams

Load and Resistance Factor Design



- loads on structures are
  - not constant
  - can be more influential on failure
  - happen more or less often
  - UNCERTAINTY

$$\Sigma\gamma_i R_i \le \phi R_n$$

factored load combination

ϕ - resistance factor

γ - load factors for types of loads (R)

R_n – nominal strength

Load Types

- D = dead load
- L = live load
- L_r = live roof load
- W = wind load
- S = snow load
- E = earthquake load
- R = rainwater load or ice water load

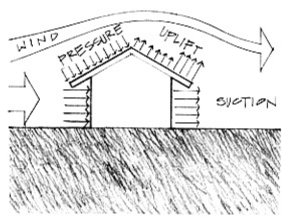
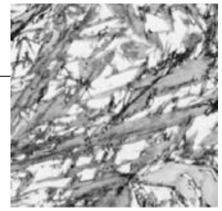


Figure 1.13 Wind loads on a structure.

Load Combinations

- "summation" means AND (combine)
  - 1.4D
  - 1.2D + 1.6L + 0.5(L_r or S or R)
  - 1.2D + 1.6(L_r or S or R) + (0.5L or 0.8W)
  - 1.2D + 1.3W + 0.5L + 0.5(L_r or S or R)
  - 1.2D + 1.5E + (0.5L or 0.2S)
  - 0.9D - (1.3W or 1.5E)

Steel Materials



- ASTM A36 – carbon
  - plates, angles
  - F_y = 36 ksi & F_u = 58 ksi
- ASTM A572 – high strength low-alloy
  - some beams
  - F_y = 60 ksi & F_u = 75 ksi
- ASTM A992 – for building framing
  - most beams
  - F_y = 50 ksi & F_u = 65 ksi

Flexure

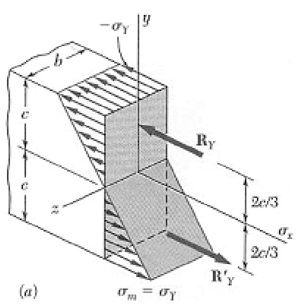
- limit is in plastic stress range

$$\Sigma\gamma_i R_i = M_u \le \phi_b M_n = 0.9F_y Z$$

M_u - maximum moment
ϕ_b - resistance factor for bending = 0.9
M_n - nominal moment (ultimate capacity)
F_y - yield strength of the steel
Z - plastic section modulus*

Internal Moments - at yield

- material hasn't failed

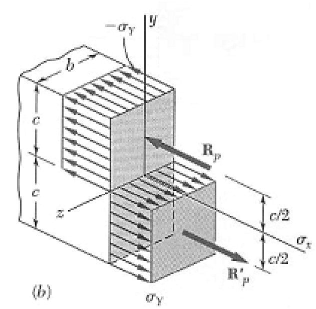


$$M_y = \frac{I}{c} f_y = \frac{bh^2}{6} f_y$$

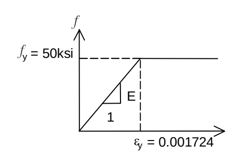
$$= \frac{b(2c)^2}{6} f_y = \frac{2bc^2}{3} f_y$$

Internal Moments - ALL at yield

- all parts reach yield
- plastic hinge forms
- ultimate moment
- $A_{tension} = A_{compression}$

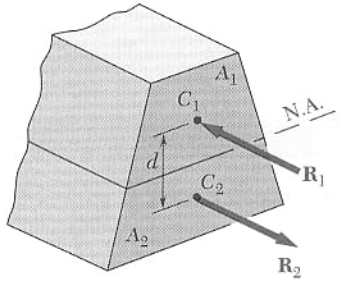


$$M_p = bc^2 f_y = \frac{3}{2} M_y$$



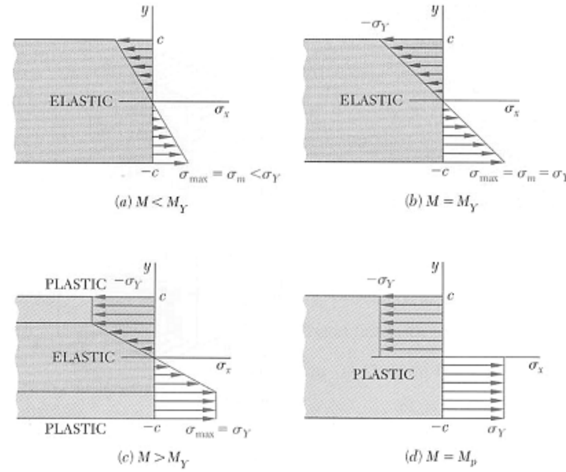
n.a. of Section at Plastic Hinge

- cannot guarantee at centroid
- $f_y A_1 = f_y A_2$
- moment found from yield stress times <u>moment area</u>



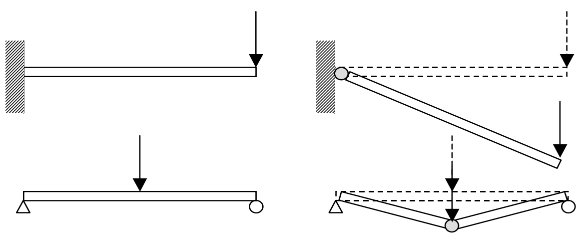
$$M_p = f_y A_1 d = f_y \sum_{n.a} A_i d_i$$

Plastic Hinge Development



Plastic Hinge Examples

- stability can be effected



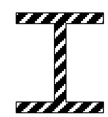
Plastic Section Modulus

- shape factor, k

$$k = M_p / M_y$$

= 3/2 for a rectangle

$\approx$ 1.1 for an I



$$k = Z / S$$

- plastic modulus, Z

$$Z = \frac{M_p}{f_y}$$

Shear

$$\Sigma\gamma_i R_i = V_u \le \phi_v V_n = 0.9(0.6F_{yw}A_w)$$

V_u - maximum shear
ϕ_v - resistance factor for shear = 0.9
V_n - nominal shear
F_{yw} - yield strength of the steel in the web
A_w - area of the web = $t_w d$

Flexure Design

- *limit states for beam failure*
  1. *yielding*
  2. *lateral-torsional buckling**
  3. *flange local buckling*
  4. *web local buckling*
- *minimum M_n governs*

$$\Sigma\gamma_i R_i = M_u \le \phi_b M_n$$

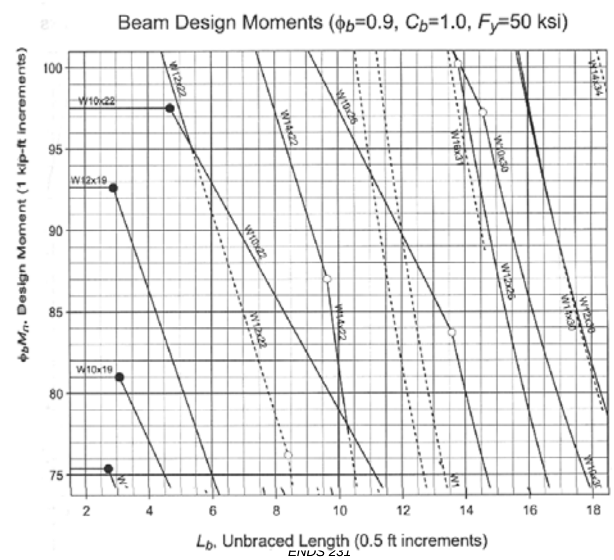
Lateral Torsional Buckling

$$M_n = C_b\left[\begin{matrix}\text{moment based on}\\ \text{lateral buckling}\end{matrix}\right] \le M_p$$

$$C_b = \frac{12.5M_{\max}}{2.5M_{\max} + 2M_A + 4M_B + 3M_C}$$

C_b - modification factor
M_{max} - |max moment|, unbraced segment
M_A - |moment|, 1/4 point
M_B - |moment|, center point
M_C - |moment|, 3/4 point

Beam Design Charts



Charts & Deflections

- *beam charts*
  - *solid line is most economical*
  - *dashed indicates there is another more economical section*
  - *self weight is NOT included in M_n*
- *deflections*
  - *no factors are applied to the loads*
  - *often governs the design*