APPLIED ARCHITECTURAL STRUCTURES:
STRUCTURAL ANALYSIS AND SYSTEMS

ARCH 631
DR. ANNE NICHOLS
FALL 2013

lecture
twenty three

Bright Football Complex
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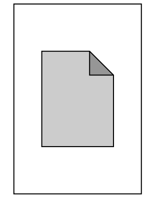
masonry construction

Learning Evaluation



Masonry

- columns
- beams
- arches
- walls
- footings

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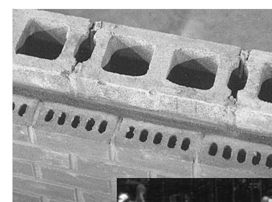
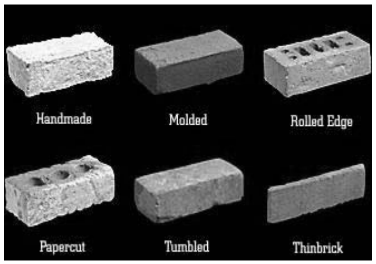
Masonry Construction

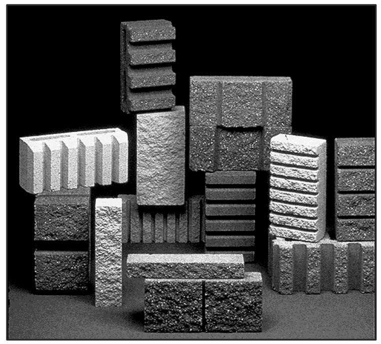
- solid, grouted, hollow
- unreinforced
- reinforced
- prestressing



Masonry Materials

- *brick*
- *concrete masonry units*



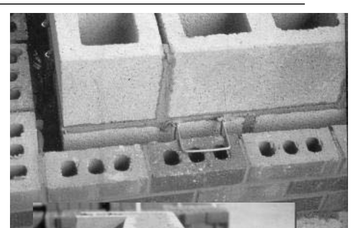


Masonry Materials

- *mortar*
  - *water, masonry cement, sand, lime*
  - *types:*
    - *M* *higher strength – 2500 psi (ave.)*
      A
    - *S* *medium high strength – 1800 psi*
      O
    - *N* *medium strength – 750 psi*
      W
    - *O* *medium low strength – 350 psi*
      R
    - *K* *low strength – 75 psi*

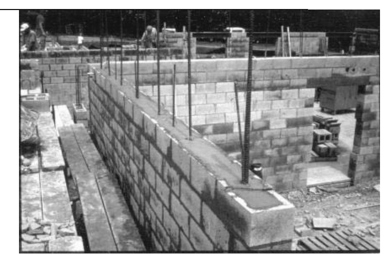
Masonry Materials

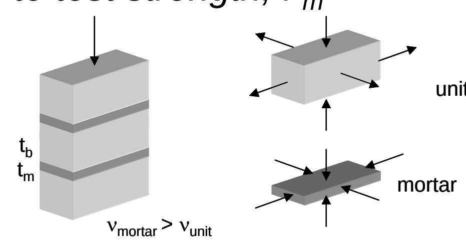
- *reinforcement*
  - *deformed bars*
  - *prestressing strand*
  - *development length*
  - *anchorage*
  - *splices*
  - *ties*
- *steel or composite*



Masonry Materials

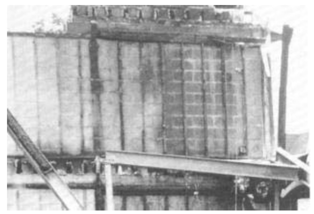
- *grout*
  - *high slump concrete*
  - *fills voids and fixes rebar*
- *prisms*
  - *used to test strength, f'_m*





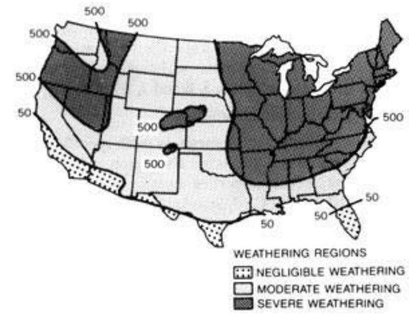
Masonry Materials

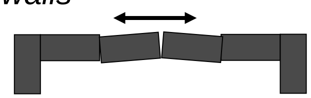
- *fire resistance*
  - – *fire-resistive structural material*
  - – *details important to prevent leaks or cracks*
  - – *retains strength if exposure not too long*
    - *mortar and cmu's dehydrate*
    - *loses 30-60% after that*
  - – *no toxic fumes*
  - – *cover necessary to protect steel*



Masonry Materials

- *moisture resistance*
  - – *weathering index for brick*
  - – *bond and detailing*
  - – *expansion or shrinking from water*
    - *provide control joints*
    - *parapets, corners, long walls*





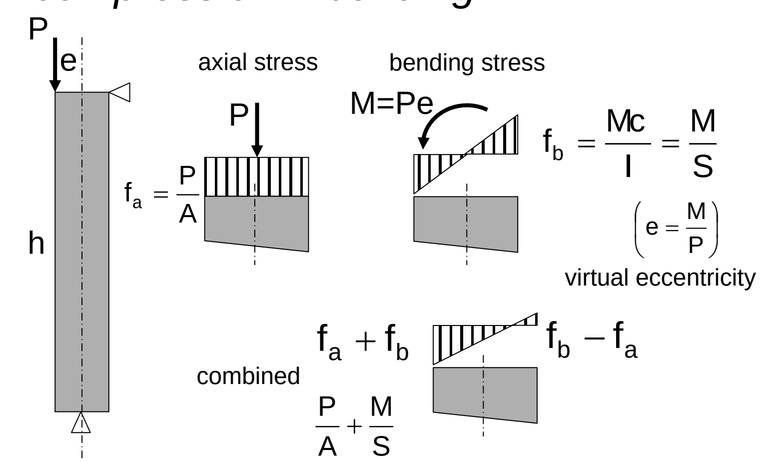
parapet with no control joint

Masonry Walls

- – *based on empirical requirements for minimum wall thickness and height*
  - *h/t < 25 (UBC 2105.2 h/t<35)*
- – *wall thicknesses often increased by 4"/story*
- – *bearing walls > 3-5 stories uneconomical, steel or concrete frames used*
- – *strength design limit states:*
  - *serviceability: deflection*
  - *ultimate: compression & tension*

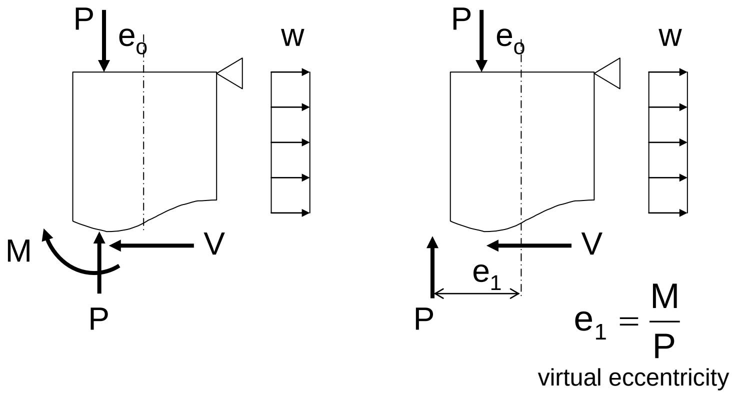
Masonry Walls

- *compression + bending*

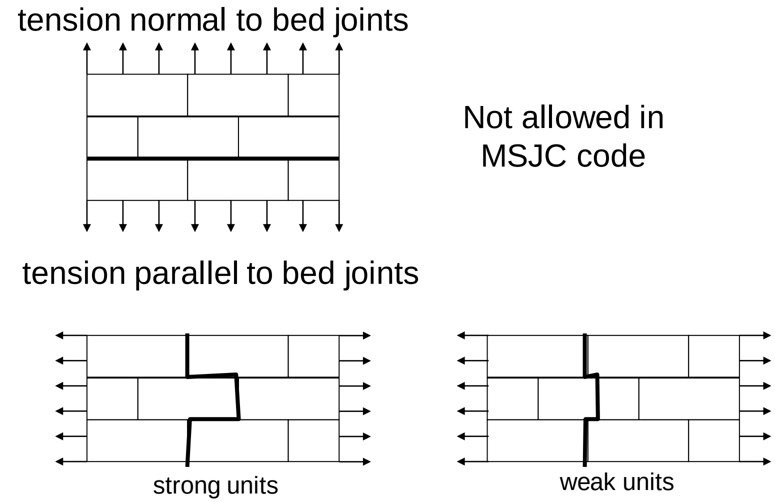


Masonry Walls

- *equivalent eccentricity with lateral load*



Masonry Walls



Masonry Beam & Wall Design

- *MSJC (ACI, ASCE, TMS)*

  - *limit tensile stress in mortar*
  - *working stress design (ASD)*
    - *linear stresses in masonry*
    - *no tension in masonry when reinforced*
    - *elastic stress in steel < f_y*
    - *additional compression in walls*
  - *masonry strength = f'_m*

Masonry Beam & Wall Design

- *reinforcement increases capacity & ductility*

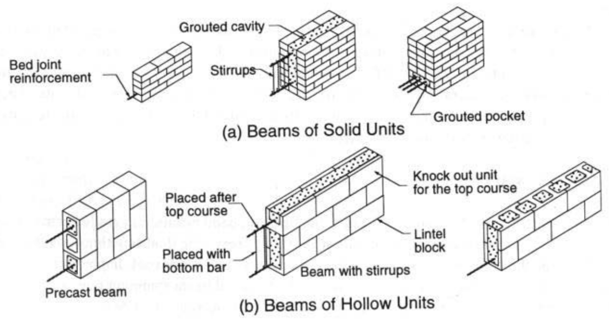
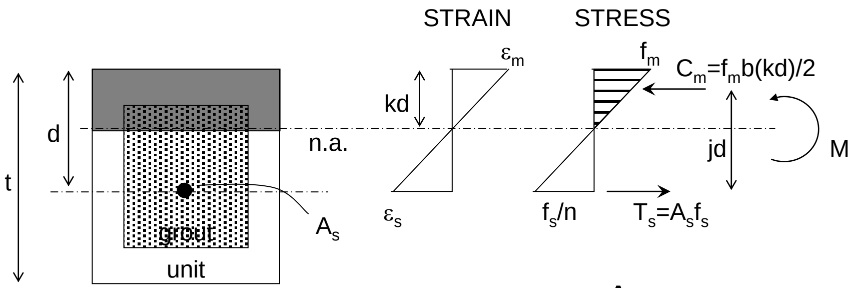


Figure 2.10 Reinforced masonry beams and lintels.

Masonry Design

- f_s is <u>not</u> the yield stress
- f_m is the stress in the masonry



$$\rho = \frac{A_s}{bd}$$

BIA Teknote 17 series

Allowable Masonry Stresses

- tension - <u>unreinforced</u> only

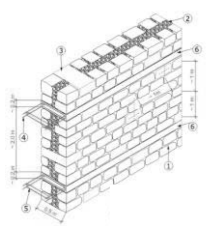
Table 2.2.3.2 — Allowable flexural tensile stresses for clay and concrete masonry, psi (kPa)

Direction of flexural tensile stress and masonry type	Mortar types			
	Portland cement/lime or mortar cement		Masonry cement or air entrained portland cement/lime	
	M or S	N	M or S	N
Normal to bed joints				
Solid units	53 (366)	40 (276)	32 (221)	20 (138)
Hollow units[1]				
Ungrouted	33 (228)	25 (172)	20 (138)	12 (83)
Fully grouted	86 (593)	84 (579)	81 (559)	77 (531)
Parallel to bed joints in running bond				
Solid units	106 (731)	80 (552)	64 (441)	40 (276)
Hollow units				
Ungrouted and partially grouted	66 (455)	50 (345)	40 (276)	25 (172)
Fully grouted	106 (731)	80 (552)	64 (441)	40 (276)
Parallel to bed joints in masonry not laid in running bond				
Continuous grout section parallel to bed joints	133 (917)	133 (917)	133 (917)	133 (917)
Other	0 (0)	0 (0)	0 (0)	0 (0)

1 For partially grouted masonry, allowable stresses shall be determined on the basis of linear interpolation between fully grouted hollow units and ungrouted hollow units based on amount (percentage) of grouting.

Allowable Masonry Stresses

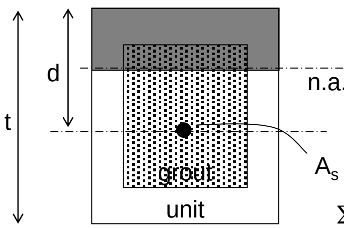
- flexure
  - $F_b = 1/3\ f'_m$ (unreinforced)
  - $F_b = 0.45\ f'_m$ (reinforced)
- shear, unreinforced masonry
  - $F_v = 1.5\sqrt{f'_m} \le 120$ psi
- shear, reinforced masonry
  - $M/Vd \le 0.25$: $F_v = 3.0\sqrt{f'_m}$
  - $M/Vd \ge 1.0$: $F_v = 2.0\sqrt{f'_m}$

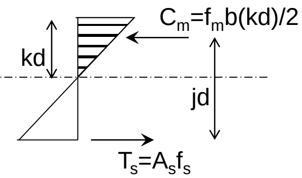


Allowable Reinforcement Stress

- tension
  - a) Grade 40 or 50 F_s = 20 ksi
  - b) Grade 60 F_s = 24 ksi
  - c) Wire joint F_s = 30 ksi
- *no allowed increase by 1/3 for combinations with wind & earthquake
  - did before 2011 MSJC

Reinforcement, M_s





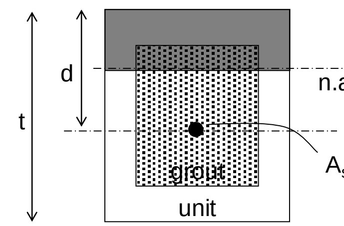
ΣF=0: $$A_s f_s = f_m b \frac{kd}{2}$$

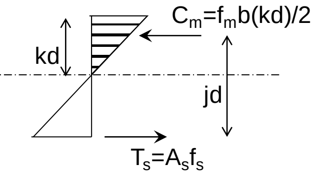
ΣM about C_m: $$M_s = A_s f_s jd = \rho bd^2 jf_s$$

if f_s=F_s (allowable) the moment capacity is limited by the steel

MSJC: F_s = 20 ksi, 24 ksi or 30 ksi by type

Reinforcement, M_m





ΣF=0: $$A_s f_s = f_m b \frac{kd}{2}$$

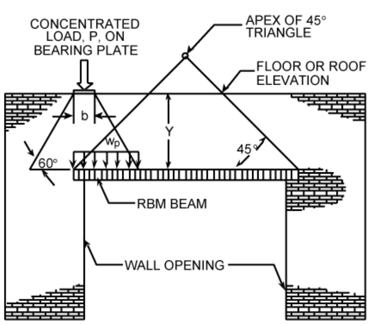
ΣM about T_s: $$M_m = f_m b \frac{kd}{2} jd = 0.5 f_m bd^2 jk$$

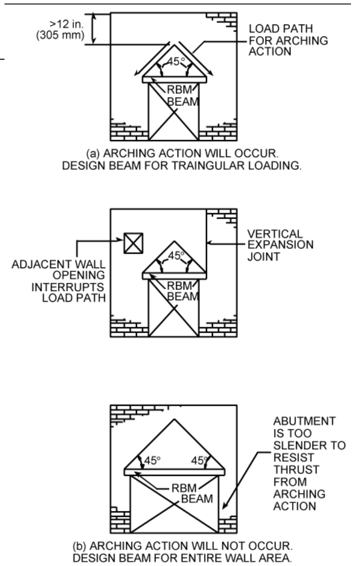
if f_s=F_s (allowable) the moment capacity is limited by the steel

MSJC F_b =0.33f'$_m$

Masonry Lintels

- *distributed load*
  - *triangular or trapezoidal*





Strategy for RM Flexural Design

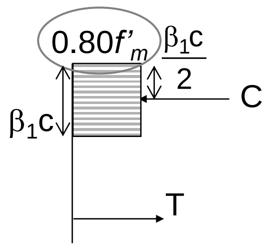
- *to size section and find reinforcement*
  - *find ρ_b knowing f'$_m$ and f_y*
  - *size section for some $\rho < \rho_b$*
    - *get k, j*
    - $bd^2 = \dfrac{M}{\rho j F_s}$
    - *get b & d in nice units*

    } *needs to be sized for shear also*
  - *size reinforcement (bar size & #):* $A_s = \dfrac{M}{F_s jd}$
  - *check design:* $M_s = A_s F_s jd > M$

$$f_b = \frac{M}{0.5bd^2 jk} < F_b$$

Ultimate Strength Design

- LRFD
- like reinforced concrete
- useful when beam shear is high
- improved inelastic model
  - ex. earthquake loads



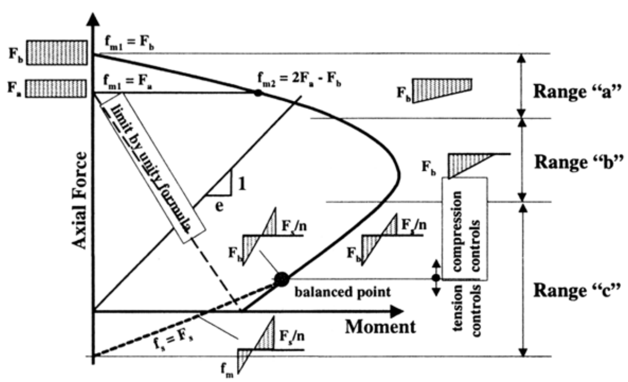
Masonry Walls

- one-way or two-way bending
- usually use hollow units (< 75% solid)
- reinforcement grouted
  - into cells if hollow units
  - between wythes if solid
- reinforcement usually at center
- reinforcement in compression ineffective
- avoid stirrups
- desirable in seismic zones

Masonry Walls

- axial force-moment interaction diagram

$$\frac{f_a}{F_a} + \frac{f_b}{F_b} \leq 1$$



Masonry Shear Walls

- bearing, bending, and shear
  - compression increases resistance

$$f_v = \frac{VQ}{I_n b} \quad \text{or} \quad \frac{V}{A_{nv}} \leq F_v$$

unreinforced reinforced

- unreinforced stress limit $1.5\sqrt{f'_m} \leq 120$ psi

Masonry Shear Walls

- *(and beams)*
  - *reinforcement strength included:*

$$F_v = F_{vm} + F_{vs}$$

  - *where*

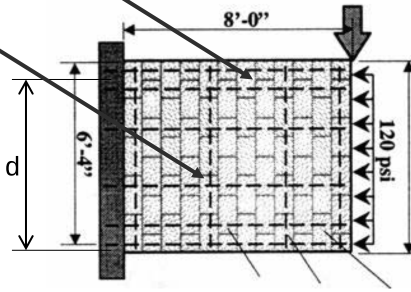
$$F_{vm} = \frac{1}{2}\left[\left(4.0 - 1.75\left(\frac{M}{Vd}\right)\right)\sqrt{f'_m}\right] + 0.25\frac{P}{A_n}$$

$$F_{vs} = 0.5\left(\frac{A_v F_s d}{A_{nv} s}\right)$$

  - *stress limit depends on ratio of bending moment to overturning moment: M/Vd*
  - *spacing limits*

Masonry Shear Walls

- *model as deep cantilever beam*
  - *flexure reinforcement*
  - *shear stirrups*



Masonry Columns and Pilasters

- *must be reinforced*

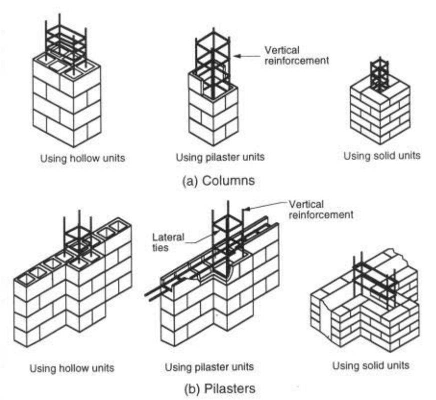


Figure 9.2 Columns and pilaster details.

Masonry Columns and Pilasters

- *considered a column when b/t<3 and h/t>4*
- *slender is*
  - *8" one side*
  - *h/t* $\leq$ *25*
- *needs ties*
- *eccentricity*
  - *10% of side dimension required*
  - *interaction diagrams like r/c*

Masonry Columns

– allowable axial load

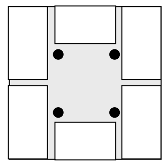
$$P_a = \left[0.25 f'_m A_n + 0.65 A_{st} F_s\right]\left[1 - \left(\frac{h}{140r}\right)^2\right]$$

$h/r \le 99$

(unreinforced $A_{st} = 0$)

$$P_a = \left[0.25 f'_m A_n + 0.65 A_{st} F_s\right]\left(\frac{70r}{h}\right)^2$$

$h/r > 99$



h = effective length

r = radius of gyration

A_n = effective area of masonry

A_{st} = effective area of column reinforcement

F_s = allowable compressive stress in column reinforcement

Masonry Pilasters, Arches

- column in wall
  – increase bearing area and stiffness

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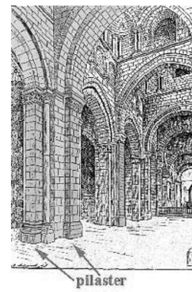

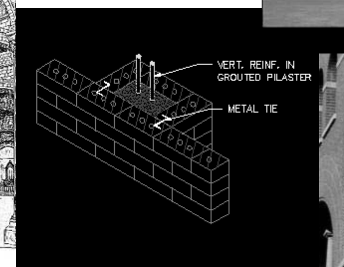

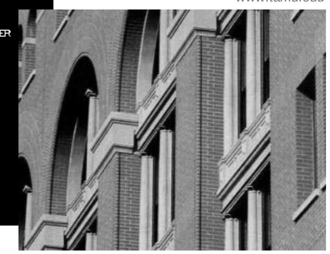

Construction Supervision

- proper placement of all reinforcement
- prism construction
  – masonry
  – mortar
- hot/cold weather protection