# The New York Times Building

ARCH 631 Case Study



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### Overview

Location: Manhattan, New York

Owner: The New York Times Company

Architect:Renzo Piano Building

Workshop/FXFOWLE ARCHITECTS, P.C.

Structural Engineer:Thornton Tomasetti

Area: 1,700,000 sf

Completion Date: 2007





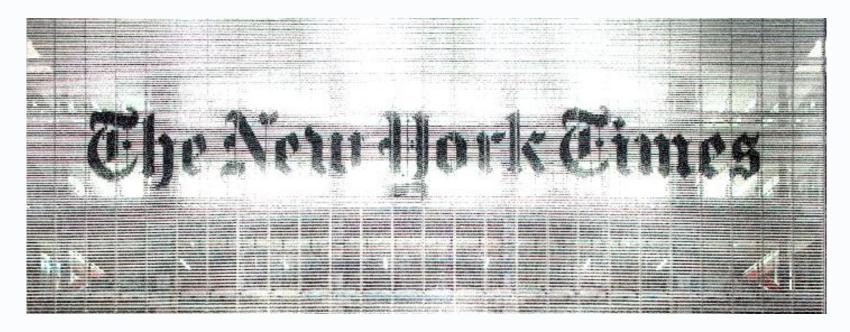
## Prospective

a signature building in the New York City skyline

a fitting home for a 21st-century media company

reinforcing the values of the Times Company and its culture of transparency and openess

a flagship building promoting sustainability, lightness, and transparency





## Design feature

The New York Times building is a 52-story high-rise office tower of lightness and transparency topped by a mast that disappears into the sky.

### Concept

Exemplify transparency and lightness through every detail

#### **Elements**

elegant structural steel exoskeleton a glass and ceramic curtain wall



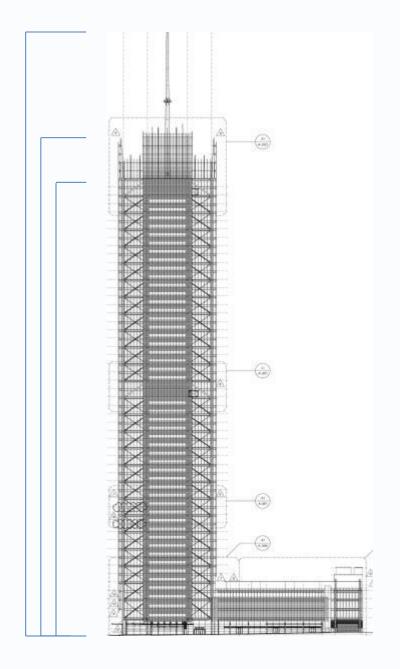


## Layout

mast tops off at 1,046 feet

curtain wall ends at 819 feet

52 stories 746 feet tall



**Mechanical 51 story** 

FCRC 29-52stories

**Mechanical 28 story** 

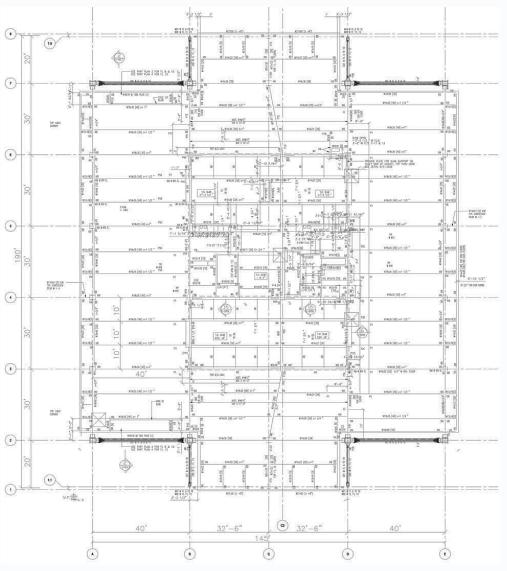
**NY Times 27 stories** 

**NY Times 5-story podium** 

lobby, retail space and a glassenclosed garden

Elevation © Thornton Tomasetti 2005

### Layout



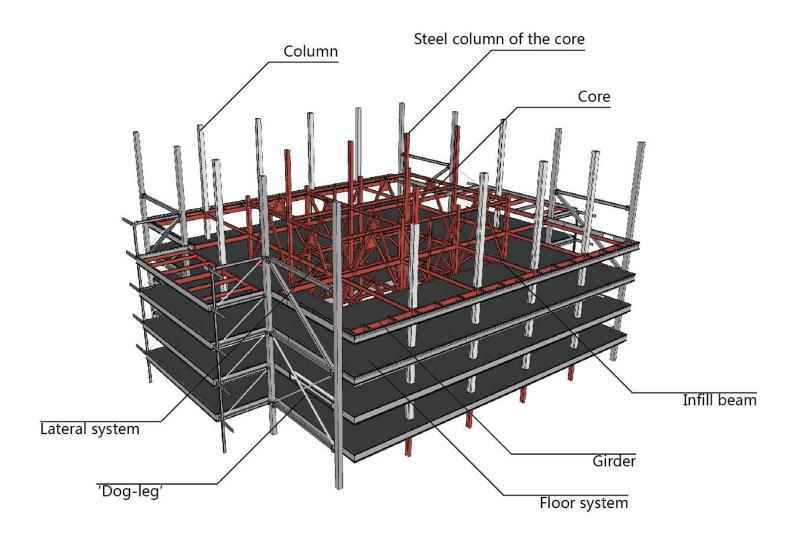
### **Floorplate**

Approximately 32,000 rentable square feet with 30-foot column spacing

**Typical Tower Framing Plan** 

(Benjamin and Erika & Andres, 2009)

## Main Structural System



### Foundation

The ground floor plan is consisted by a tower portion and a podium portion.

In structure studying, we will focus on the tower portion which is 1,046 ft high.

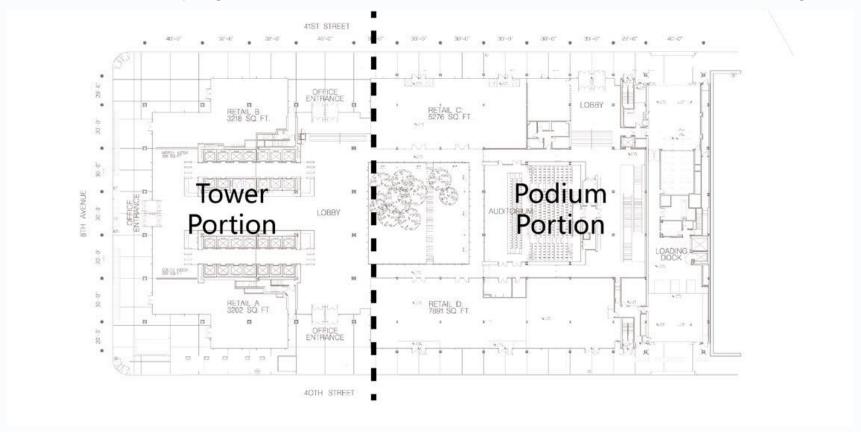
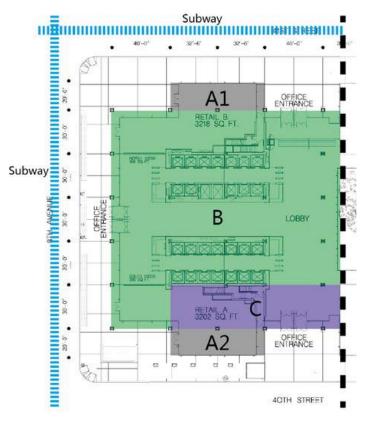


Figure 2 Ground plan portions

### Two different kinds of foundation constructions



The main portion showed on Figure 3-B Beared on stronger rocks
21 of the columns: spread footings

The portion showed on Figure 3-C 7 columns: concrete-filled steel **caisson** 

The portion A1 and A2 are cantilevered structure and do not need a foundation.

Figure 3 Foundation location diagram

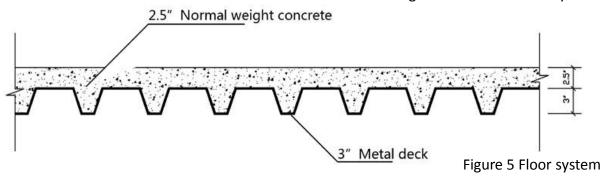


The floor system includes aurrounding bays to a centeral core.
The bay sizes are varied

Core 30" x30" box columns Cantilevere portion W33 girders W18x40 girders Floor system W18x40 girders W12x19 to W18x35 beams Cantilevere portion

concrete and metal decking

Figure 4 Floor structure system



### 'Dog-leg' connection



Figure 6 Exposed connection

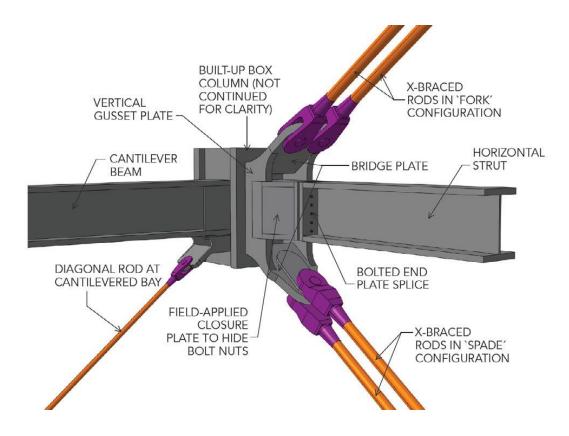


Figure 7 Connection(Jeffrey ,Kyle, and Thomas, 2009)

## Lateral System

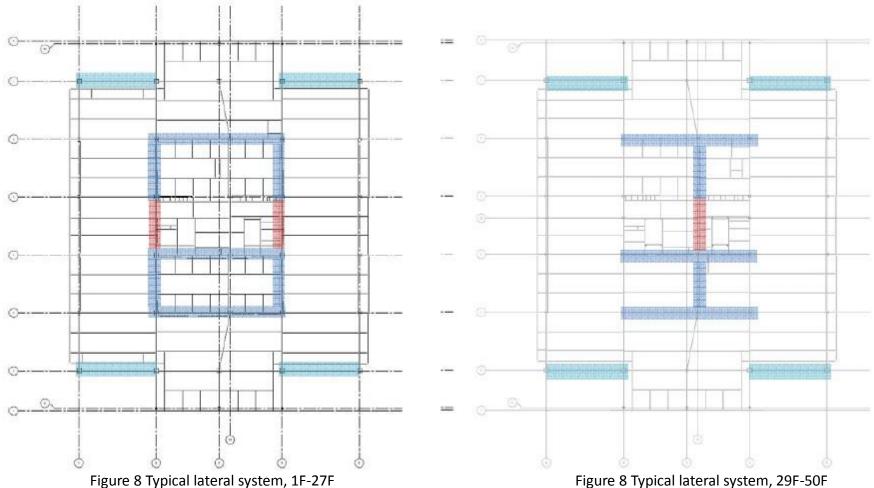


Figure 8 Typical lateral system, 1F-27F (Benjamin and Erika & Andres, 2009)

Figure 8 Typical lateral system, 29F-50I (Benjamin and Erika & Andres, 2009)



### **Load Types**

**Gravity loads**: dead loads, live loads and snow loads

Lateral loads: wind loads, seismic loads

### **A.Gravity loads:**

#### 1' Dead loads

It contain the typical tower floor dead load 93psf,typical tower mechanical floor dead loads 110psf, the exterior tower wall system dead load 25psf, total mechanical area roof dead load 100psf and nomal roof dead load 100psf (all above when for seismic should be larger)

#### 2' Live loads

Because we cannot know the weight of the mechanical equipment on the mechanical roof and the mechanical floor ,and no minimum live load is provided in ASCE7-05 and the Building Code of the City of New York, the equipments' self weight was assumed to be equivalent to light manufacturing therefore we can get a minimum live load should be 125 psf.

#### 3' Snow loads

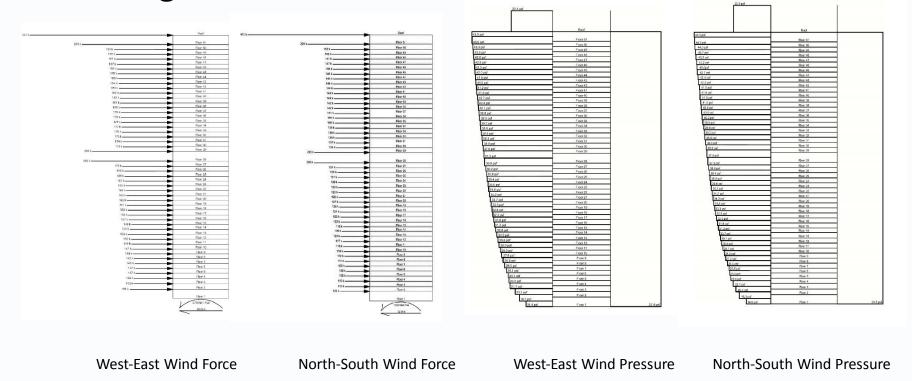
From the ASCE7-05, since the weight of snow on the roof and snow drift is almost 2 times smaller of the controlling roof live load and mechanical area roof live load, we can get a total snow loads 35.28 psf.

### **B.** Lateral Loads

#### 1' Wind load

During the time of the building's design, this code permitted the use of a simplified approach for calculating the wind loads of all buildings not more that 300 ft within the Borough of Manhattan. ASCE7-05 was used to analyse the wind load as a method. To simplify the caculation the tower was analyzed with a rectangular foot print instead of a cruciform shape the screens around each face of the roof top allow air flow through them. After the windward pressure was calculated on this "solid face", a multiplier of 0.5 was implemented to account for the permeability of the screen.

Method Wind Load Design Variables Summary					
Variable	Value	Unit	Reference		
V =	110	miles/hr	ASCE 7-05 6.5.4		
$K_a =$	0.85		ASCE 7-05 6.5.4.4		
Occupancy Category =	III		IBC Table 1604.5		
Importance factor =	1.15		ASCE 7-05 6.5.5		
Surface Roughness Category =	В		ASCE 7-05 6.5.2		
Exposure Category =	В		ASCE 7-05 6.5.6		
$K_{zt} =$	1		ASCE 7-05 6.5.7		
B =	194	Feet			
L =	157	Feet			
	1.032		West-East Direction		
$G_{\mathbf{f}} =$	1.048		North-South Direction		



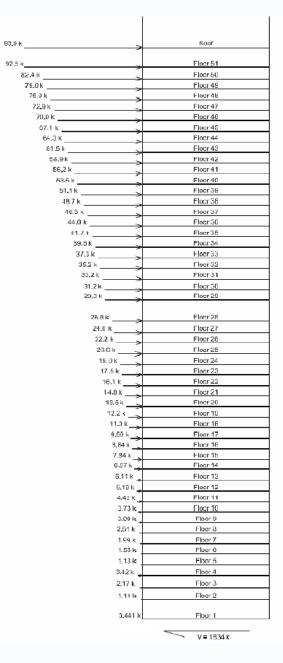
### **Conclusion Of wind loads**

The analysis shows that the controlling wind loads are in the East/West direction with a base shear of 9336 kips and overturning moment of 3.7 million ft-kips. This direction was expected to control due to its wider façade face. Please note that the base shears and overturning moments calculated in this report only consider the direct loading from windward and leeward pressures.

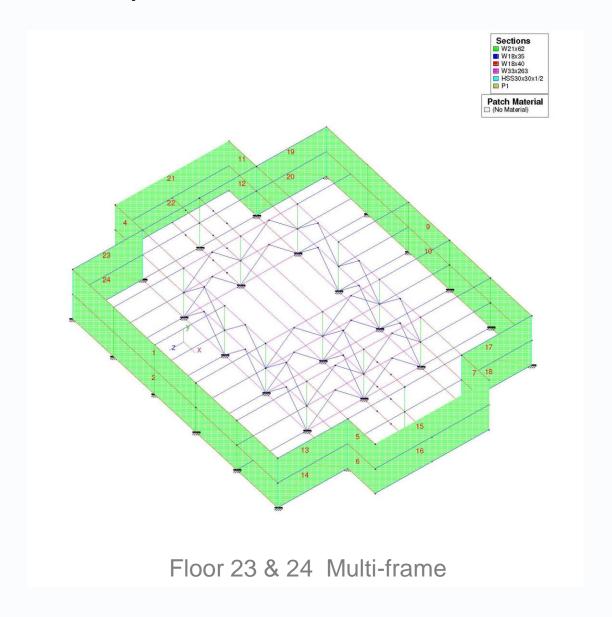
### 2' Seismic load

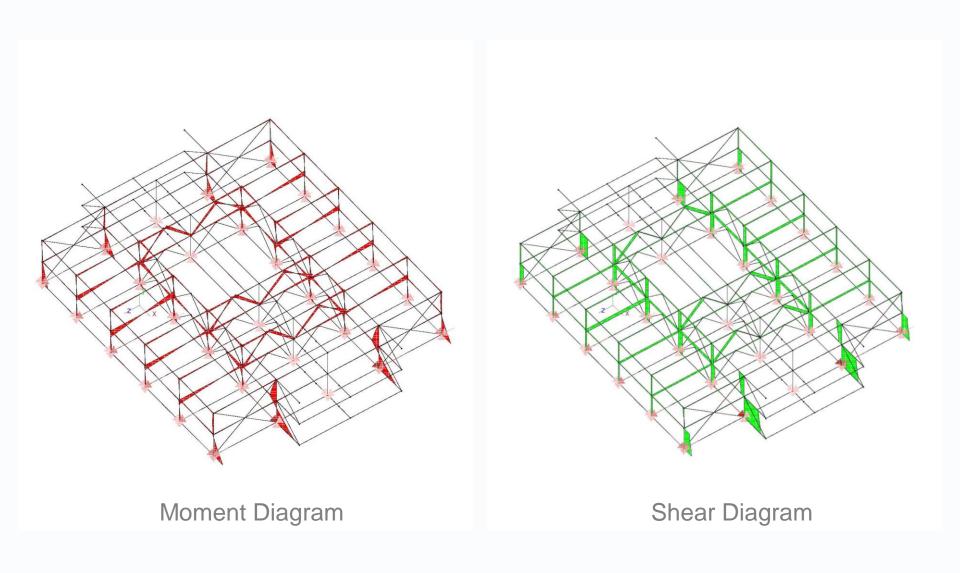
To caculate the seismic loads condition we need to use the New York City Building Code as a basis for calculation. The base shear was determined to be 1834 kips, calculated from the effective seismic weight, including the assumed dead loads and partition loads. The lateral seismic forces at each level increase with elevation, and range from 1.1 kips to 94 kips. Due to the height and location of the New York Times building, it was expected that the lateral loading due to wind pressure would control over seismic loadings.

Seismic Factors Summary			
Site Class	=	C	
Occupancy Category	=	III	
Importance Factor, I	=	1.25	
Latitude	=	40.756	
Longitude	=	-73.990	
$\mathbf{F_a}$	=	1.20	
$\overline{\mathbf{F}}_{\mathbf{v}}$	=	1.70	
$S_s$	=	0.363g	
$S_{\mathbf{i}}$	Ξ	0.070g	
Seismic Design Cat.	=	В	



## Muliframe Analysis





# **THANK YOU!**

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#### Reference List:

The New York Times Company, 2006. "Ground Level Plan", http://newyorktimesbuilding.com/

Benjamin R. Barben, Erika L. Bonfanti, & Anders R. Perez, 2009. The New York Times Building Technical Report

Jeffrey A. Callow, P.E., Kyle E. Krall, P.E., and Thomas Z. Scarangello, P.E., 2009, "Inside and out", *Modern Steel Construction*, Jan 2009