Geisel Library



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Introduction

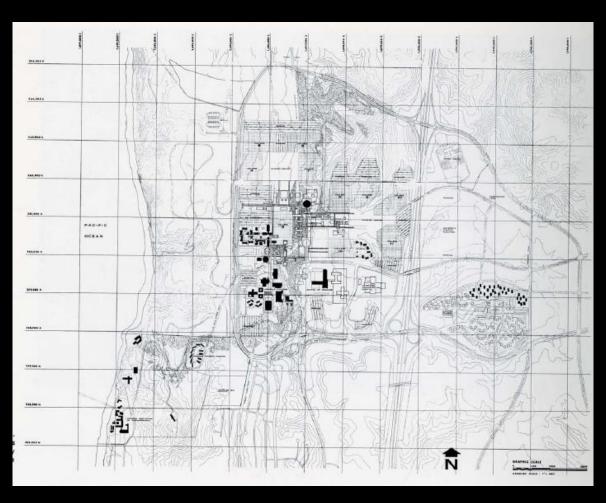
- & Architect: William L. Pereira

- № Dedicated to Audrey & Theodore Seuss Geisel (Dr. Seuss) in 1995



Background

Campus Master Plan



The Architect:

William L. Pereira

- Born: April 25, 1909; Chicago, Illinois
- Education: University of Illinois (1931)
- - Professor of architecture at University of Southern California

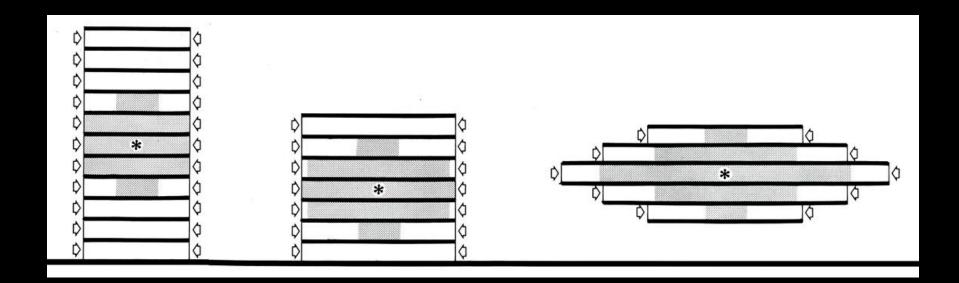
Style:

- g Functionalist

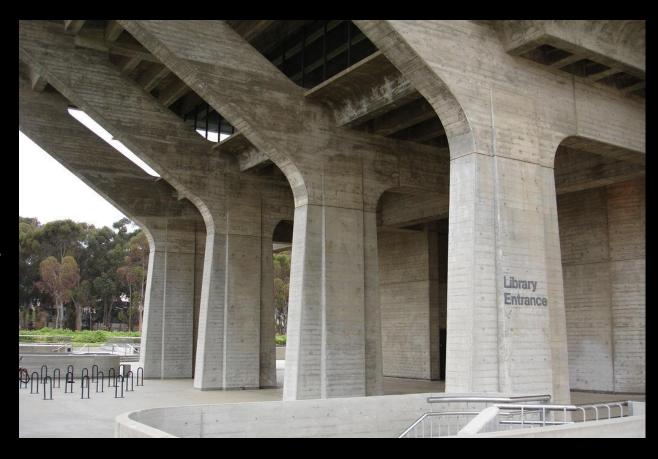


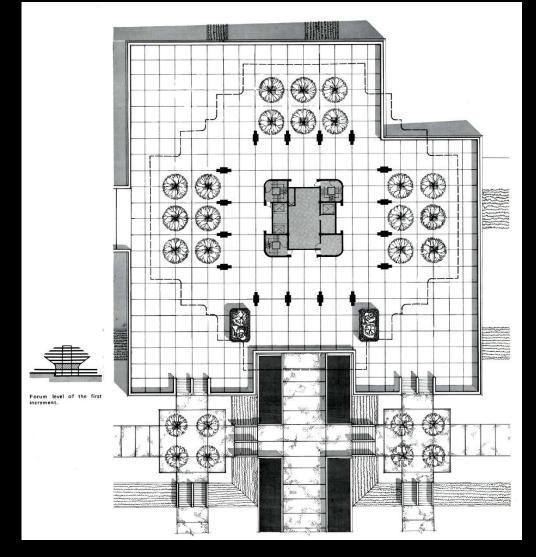
Program:

ø 2,500,000 books



Forum 8 16 columns

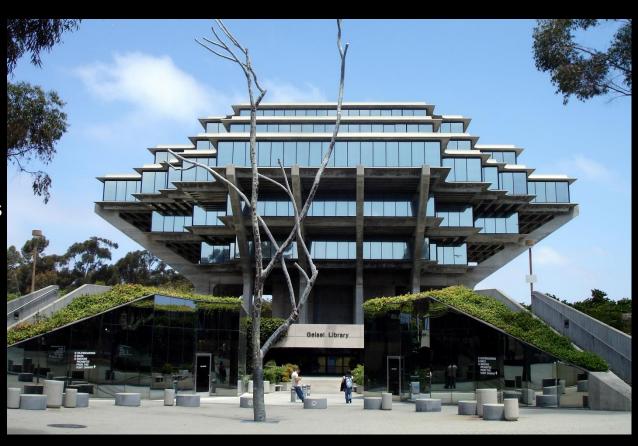




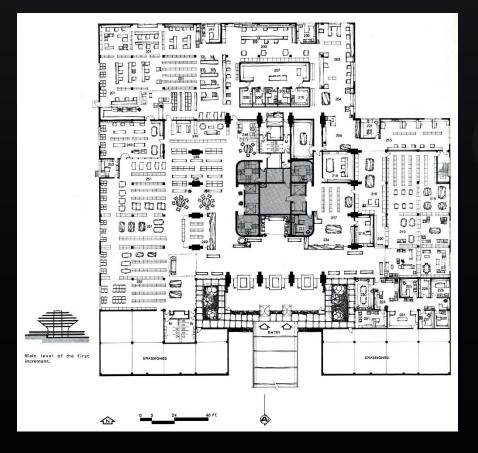
Forum

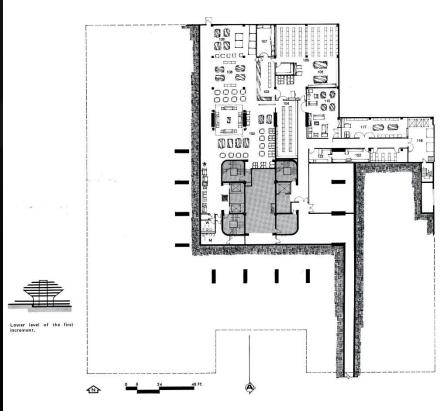
Below Forum

- g Public Floor
 - ষ Main entrance
 - ম Staff areas
 - ম Library Services
- g Basement
 - ম Staff areas
 - ম Mechanical



Below Forum





Above Forum

ষ Book collection

ষ Study areas

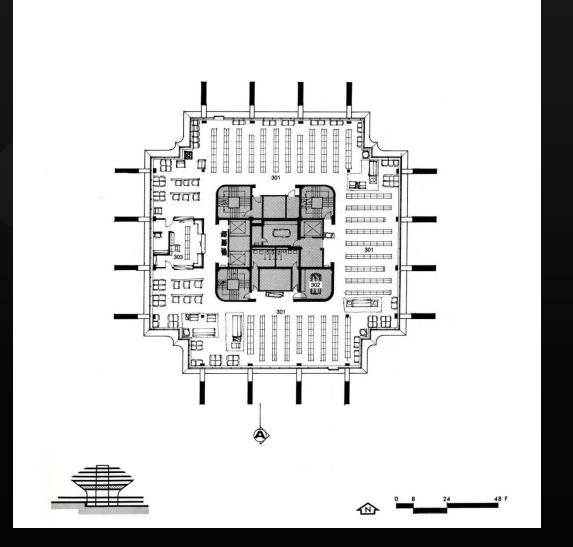
g Elliptical in Section

g "Circular" in Plan

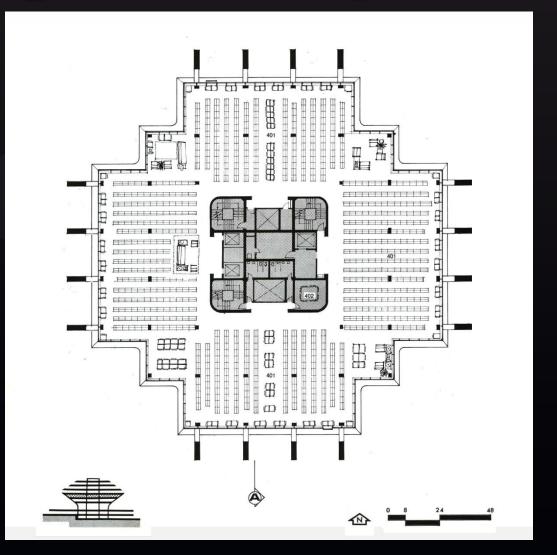


Above Forum:

1st level of
stacks



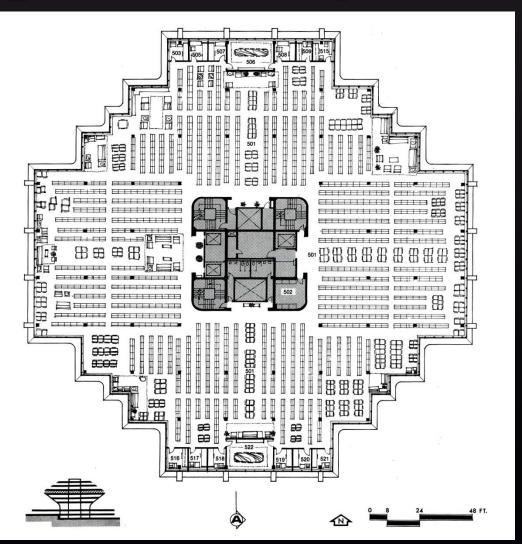
Above Forum: 2nd level of stacks



Above Forum:

3rd level of

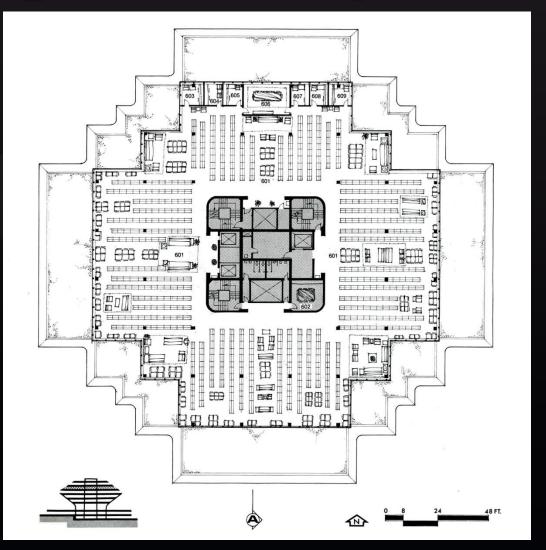
stacks



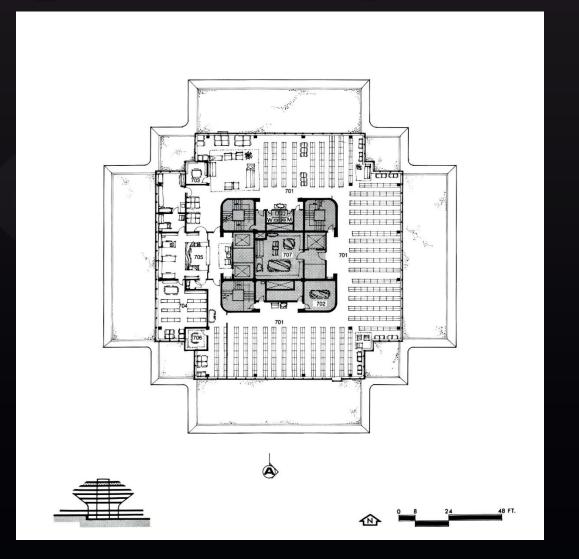
Above Forum:

4th level of

stacks



Above Forum: 5th level of stacks



Choice of structural material

Steel structure

with four large steel trusses supporting the third floor of spheroid, concealed in the second floor.



Hybrid Steelconcrete structure

With concrete up to steel trusses



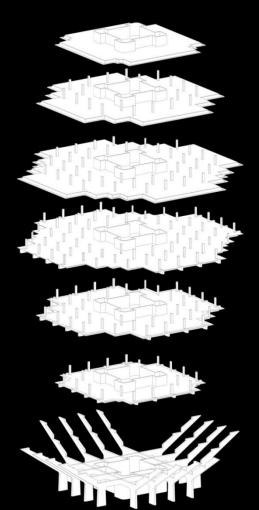
Reinforced Concrete structure

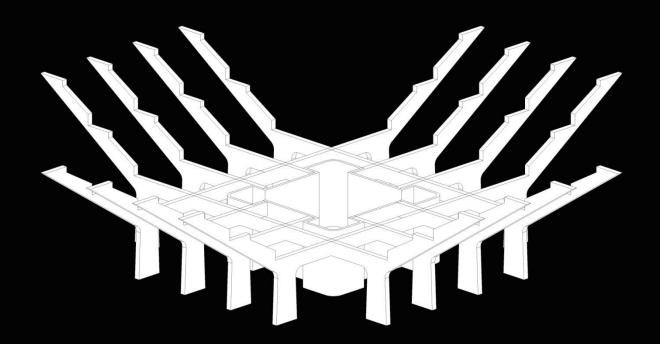
With external 16 sloped beam –column Laterally tied at lower three spheroid by posttensioned beams.

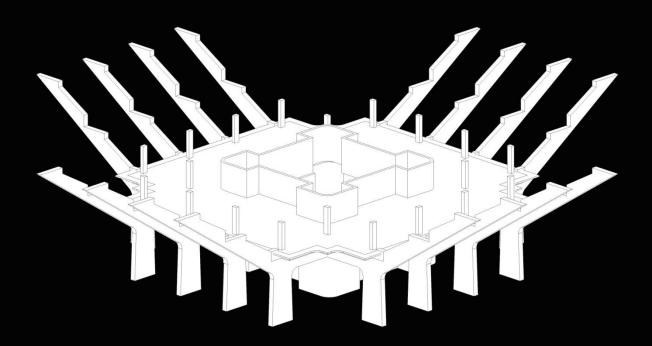
•Factors influencing the choice of Reinforced Concrete construction

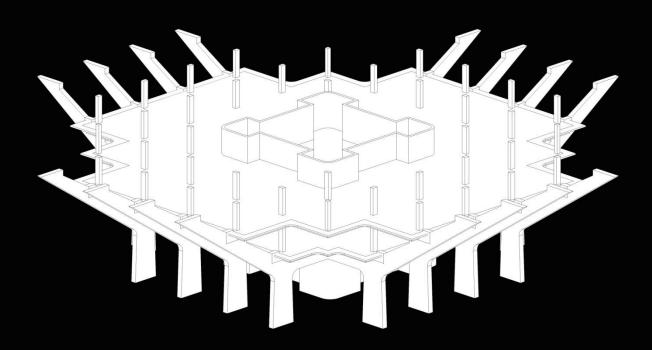
- Increasing rate of steel and extensive use of steel in the truss.
- Reduced flexibility of space at 2nd level of the spheroid.
- •Additional cost of fireproofing the steel.
- High cost of connection at points of intersection of materials and load transfer

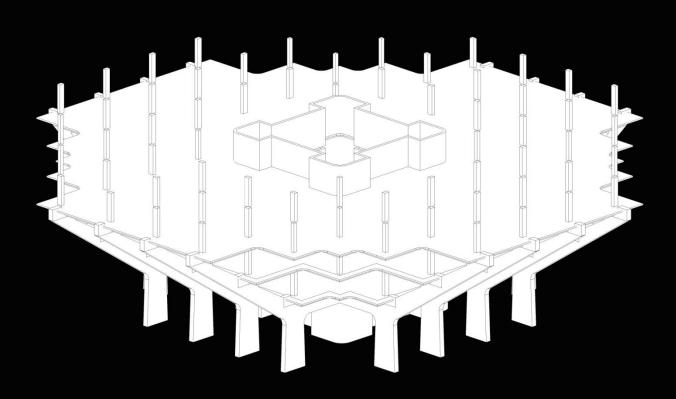
- •Main columns and sloped beamcolumns: 3000 psi R.C.C with exposed rough form board finish
- Floor system up to third level of spheroid: Standard concrete Pan-joists
- •Floor system above third floor: R.C.C slab and beam systems
- Stair and elevator core: Reinforced concrete stone aggregate
- Exterior walls: Poured in place R.C.C panels.
- •The peripheral beams tying the bent columns: Post tensioned concrete 300 post-tensioning rods of 1/4-inch diameter high-tensile steel.

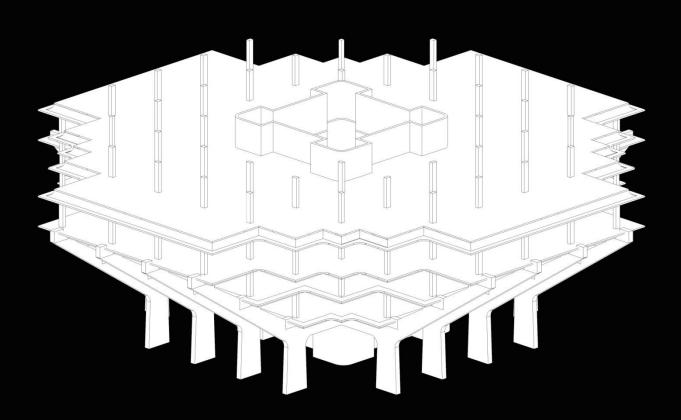


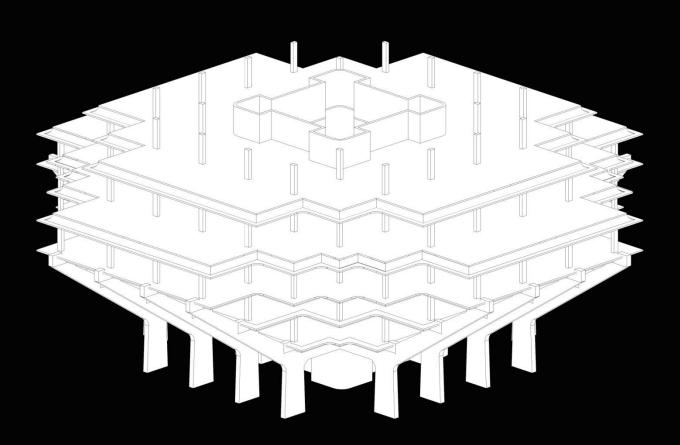


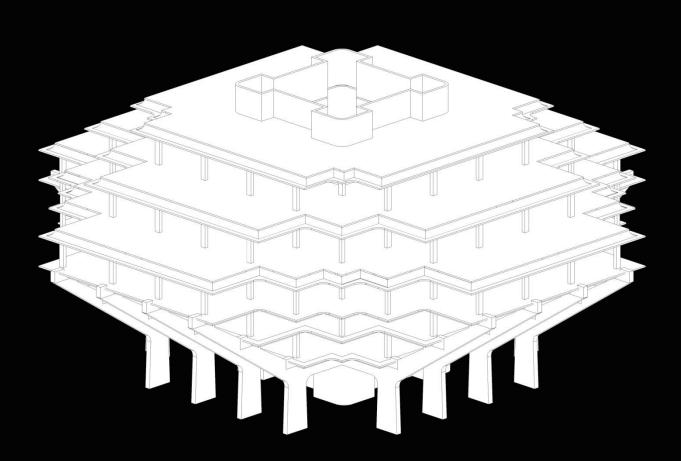


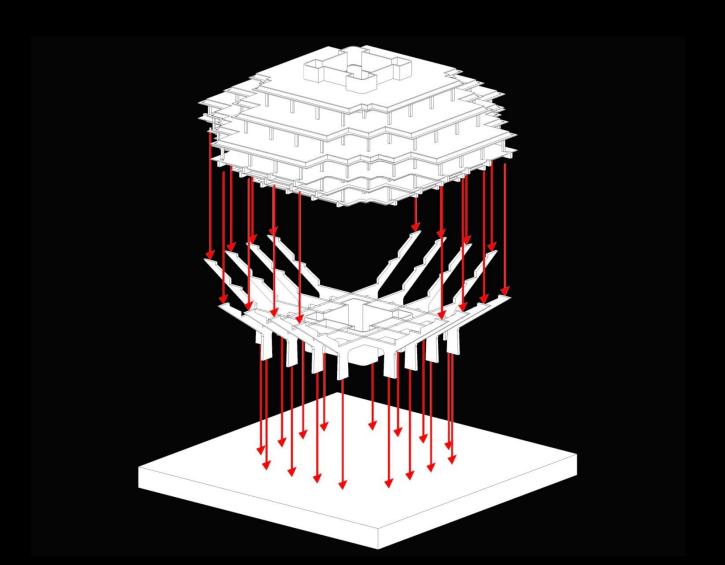




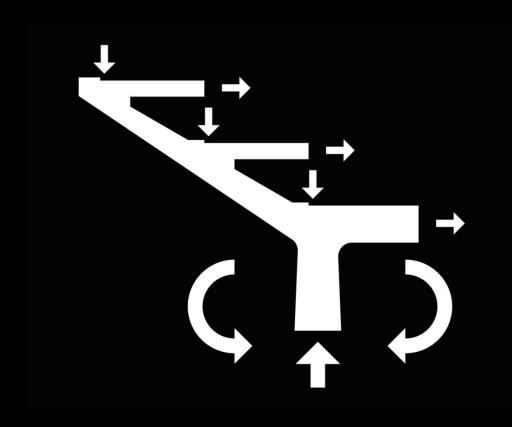




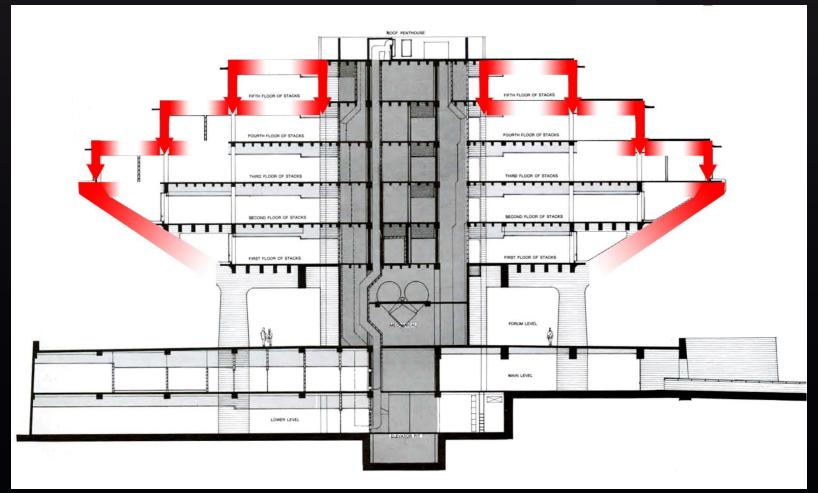


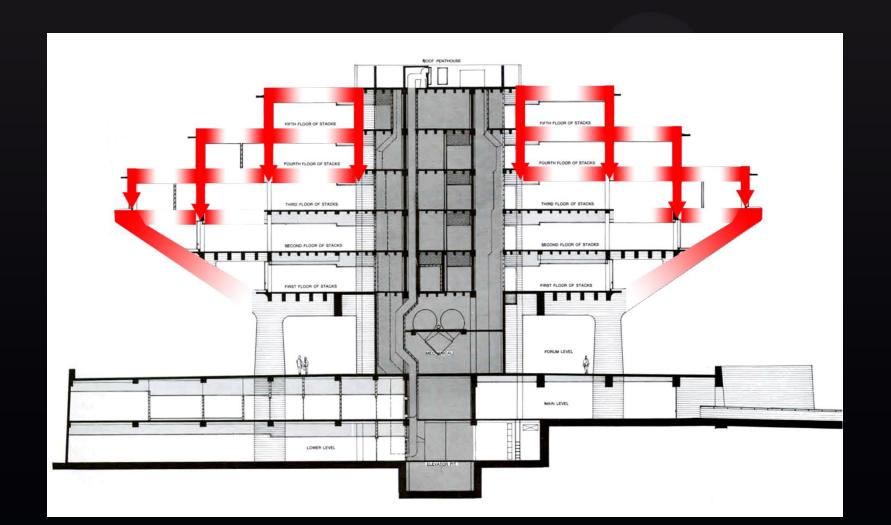


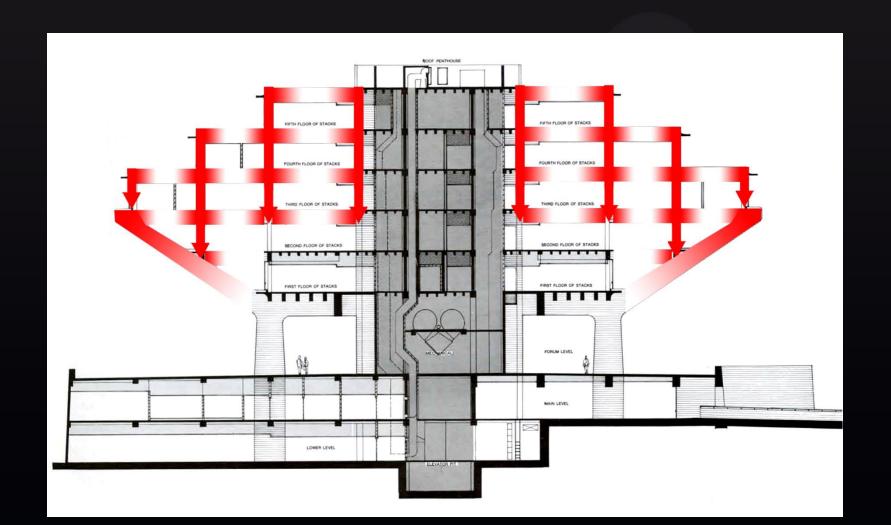
Structure: columns

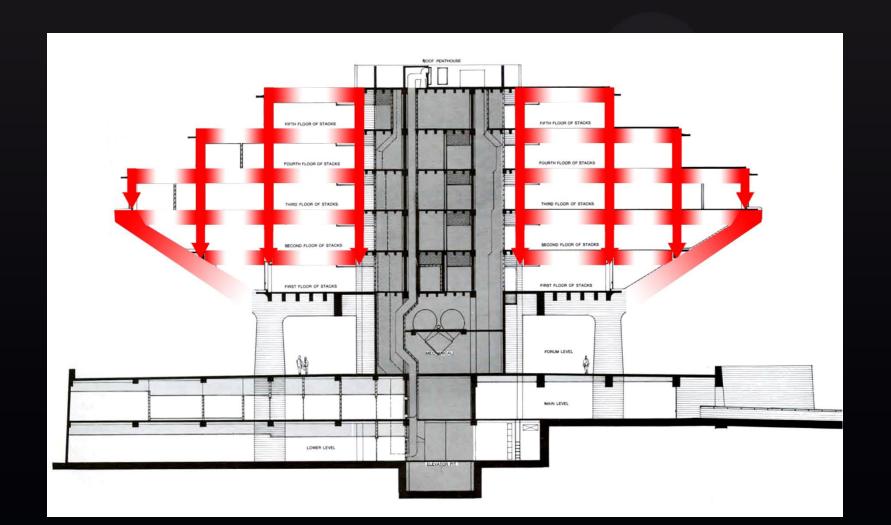


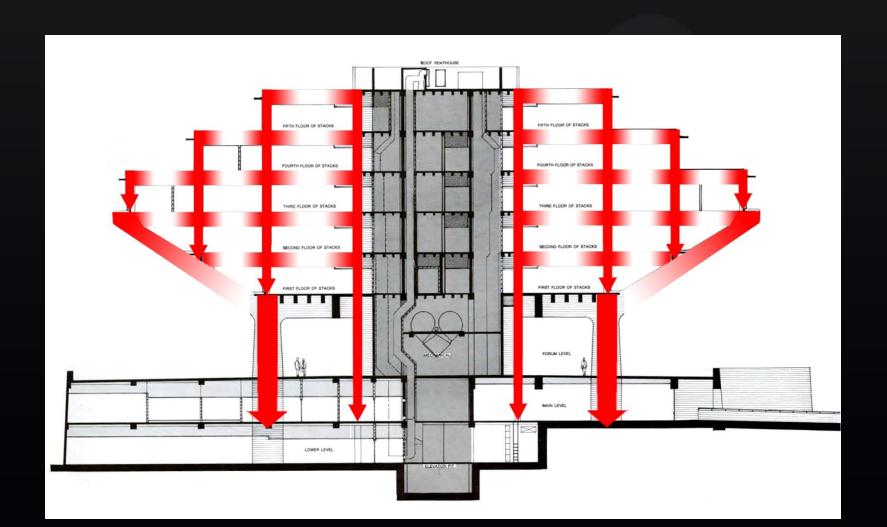
Exterior column loads in the tower 2400k-2475k
Interior column loads in tower 700k-800k
Column loads not in tower 150k-200k

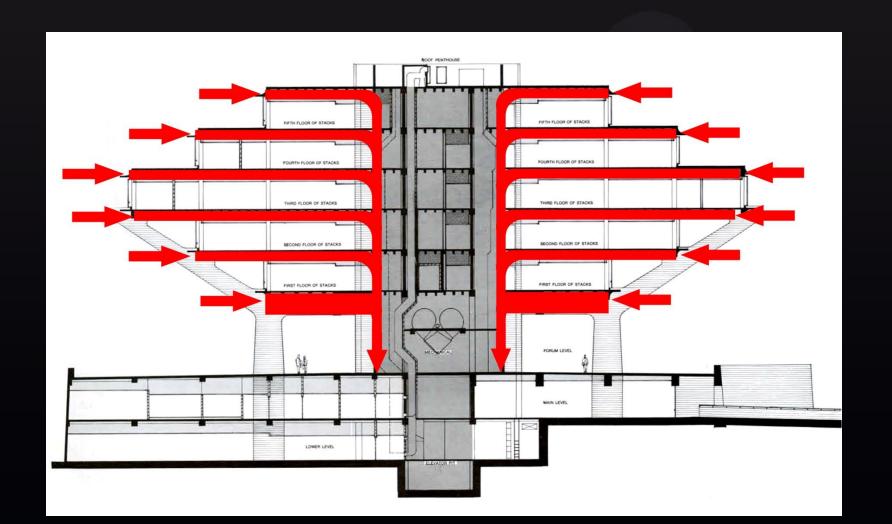








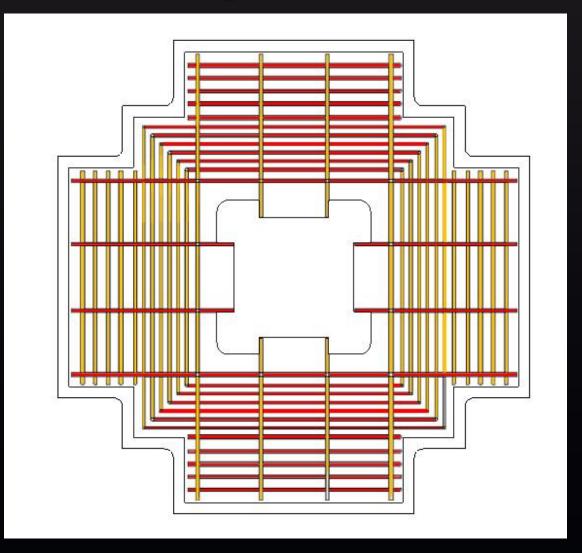




Structure: Roof System

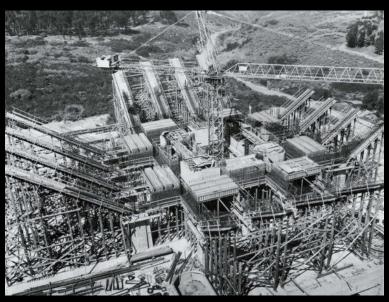
Two way floor system

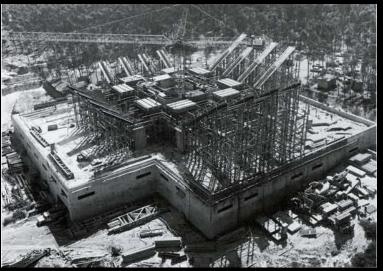
Diaphragm – cast in place



Construction

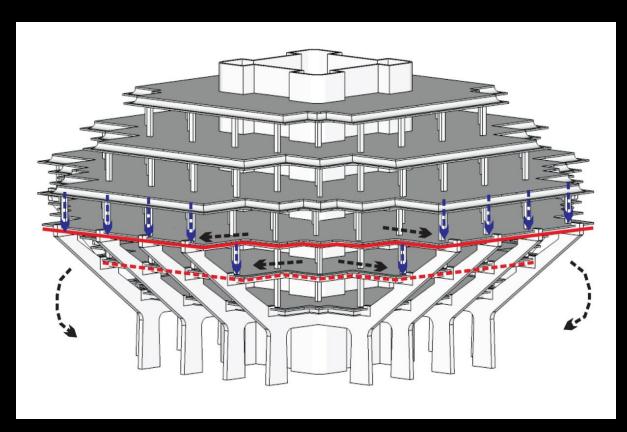
- ·Construction began in 1968.
- •The center of the core was left open for placement of construction crane
- •The lower two levels were built first to act as a base for the scaffolding for formwork of the floors above.
- •The scaffolding was not removed until top floor was finished till 1969.
- •This was done since posttensioning in the lower floors and peripheral beams at lower floors of the spheroid could not be executed correctly until the upper floors were constructed.
- In 1970, Building was in full operation.





Post-tensioning

- •The nature of the loads causes the sloped beam columns to overturn outside. Hence, the peripheral beams tying these columns are post tensioned.
- •Post-tensioning helps in designing an larger effective section in order to resist deflections. The ultimate strength of a post-tensioned beam is not overly superior to that of a similarly proportioned plain R.C.C beam. In a post-tensioned beam no tensile stresses are developed and hence no cracks develop.

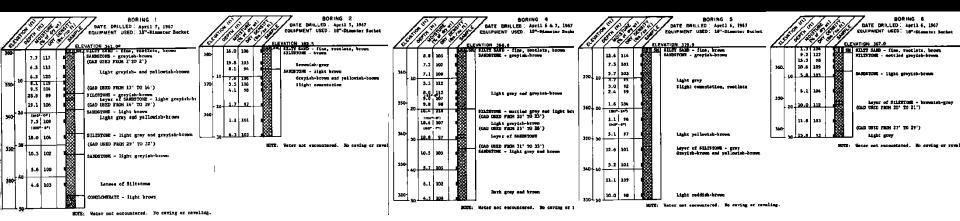


Structure: soil & foundation

Shallow layer of sand/clay/silt over siltstone and sandstone

Concrete spread footings and caissons established in bedrock

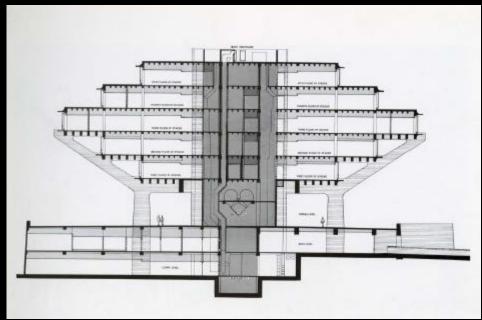
Boring is much deeper where site is backfilled and where there is an existing fault line



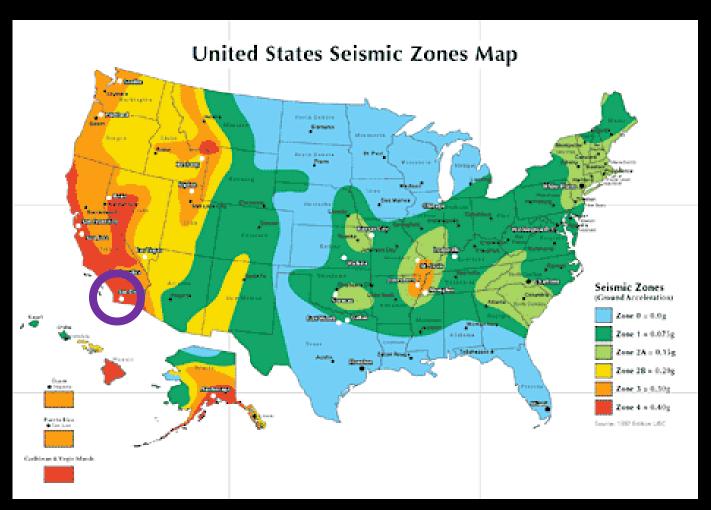
Structure: soil & foundation

Variations in soil and boring depth cause differential settling

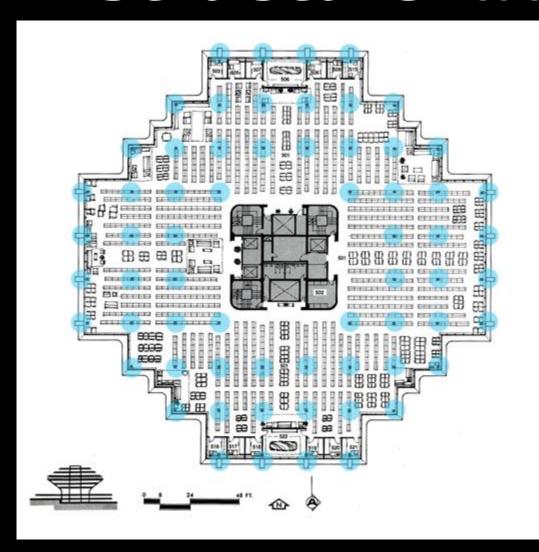
The structural tower was built before the non-tower portion was completed and prior to rigid connections to allow for presettling



Structure: seismic loads



Structure: seismic loads

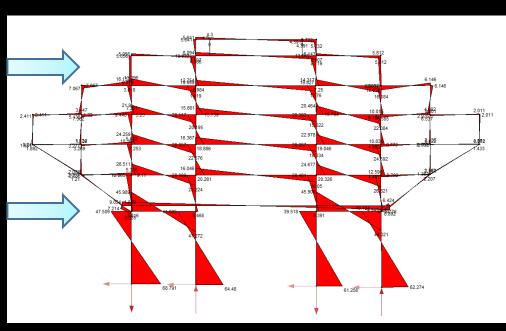


Pereira planned for symmetrical dead and live loads to decrease the chance for destructive torsion

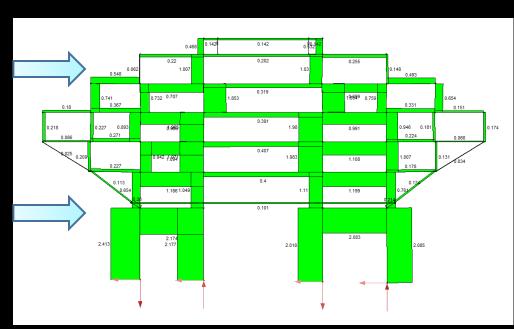
Expansion joints allow independent movement

Post-tensioned concrete tower and beam-columns designed to withstand 1/3 extra load

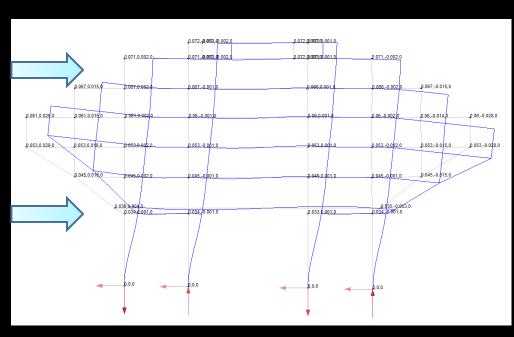
1000psf DL + LL + 3333psf wind & seismic 4333psf



Lateral loads from wind are worth noting, but not as significant as the gravity loads from a concrete structure. Because the mass is so large, Seismic loads are a large concern.

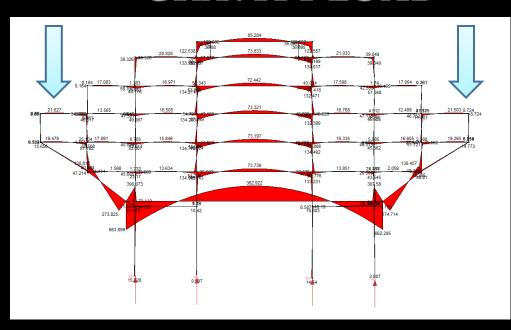


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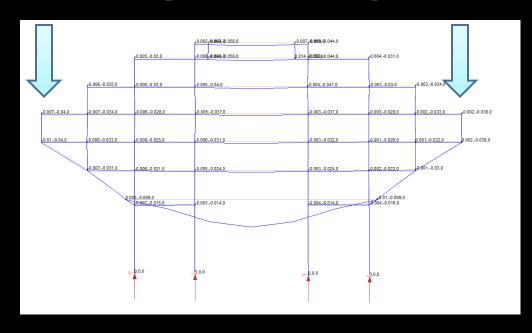
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GRAVITY LOAD



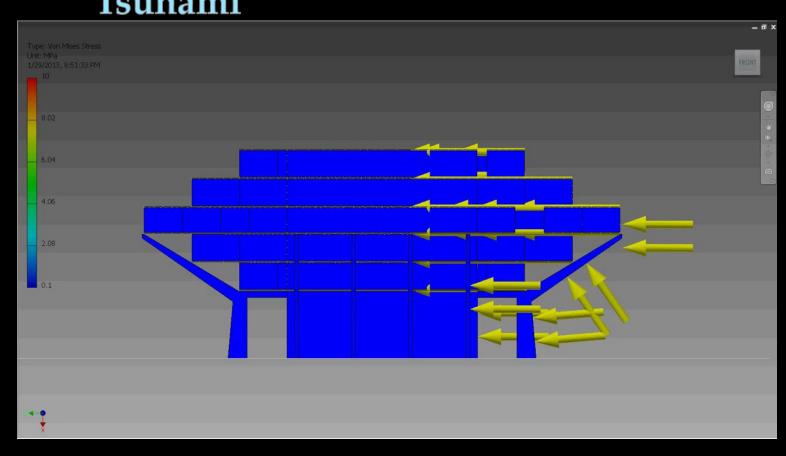
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GRAVITY LOAD



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Lateral Load from Tsunami



conclusion

References

William L. Pereira & Associates, Planning &

Architecture. *Central Library - University of California at San Diego*. Project Proposal. Corona Del Mar, CA: n.p., 1969. *Digital Archives*. University of California at San Diego Libraries. Web. 3 Nov. 2013.

http://libraries.ucsd.edu/speccoll/DigitalArchives/

z679_2u54w7_1969/z679_2_u54-w7-1969.pdf>.