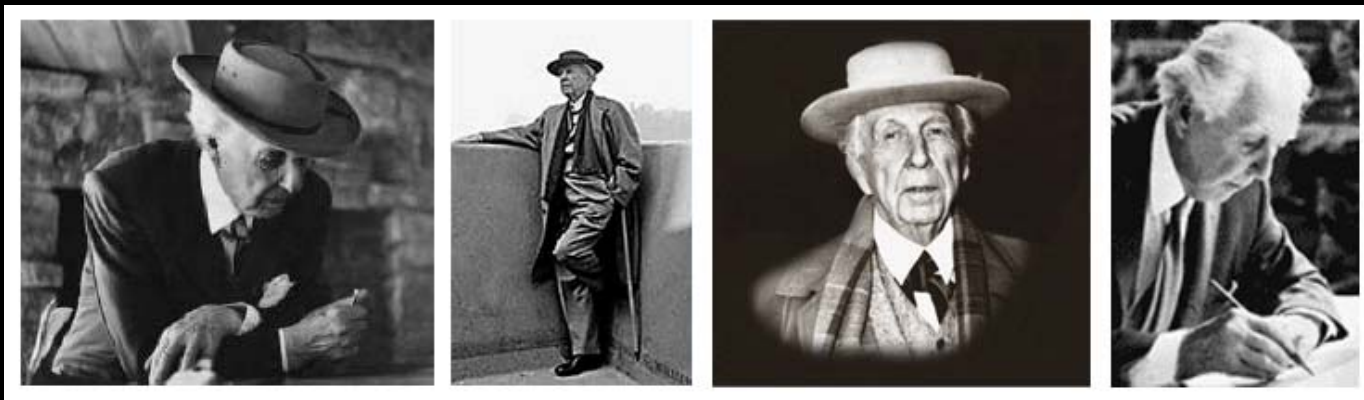


AN AMERICAN MASTERPIECE



"There in a beautiful forest was a solid, high rock ledge rising beside a waterfall, and the natural thing seemed to be to cantilever the house from that rock bank over the falling water. . . . Then came (of course) Mr. Kaufmann's love for the beautiful site. He loved the site where the house was built and liked to listen to the waterfall. So that was a prime motive in the design. I think that you can hear the waterfall when you look at the design. At least it is there, and he lives intimately with the thing he loves."

-Frank Lloyd Wright



FALLINGWATER

PAST-PRESENT-FUTURE

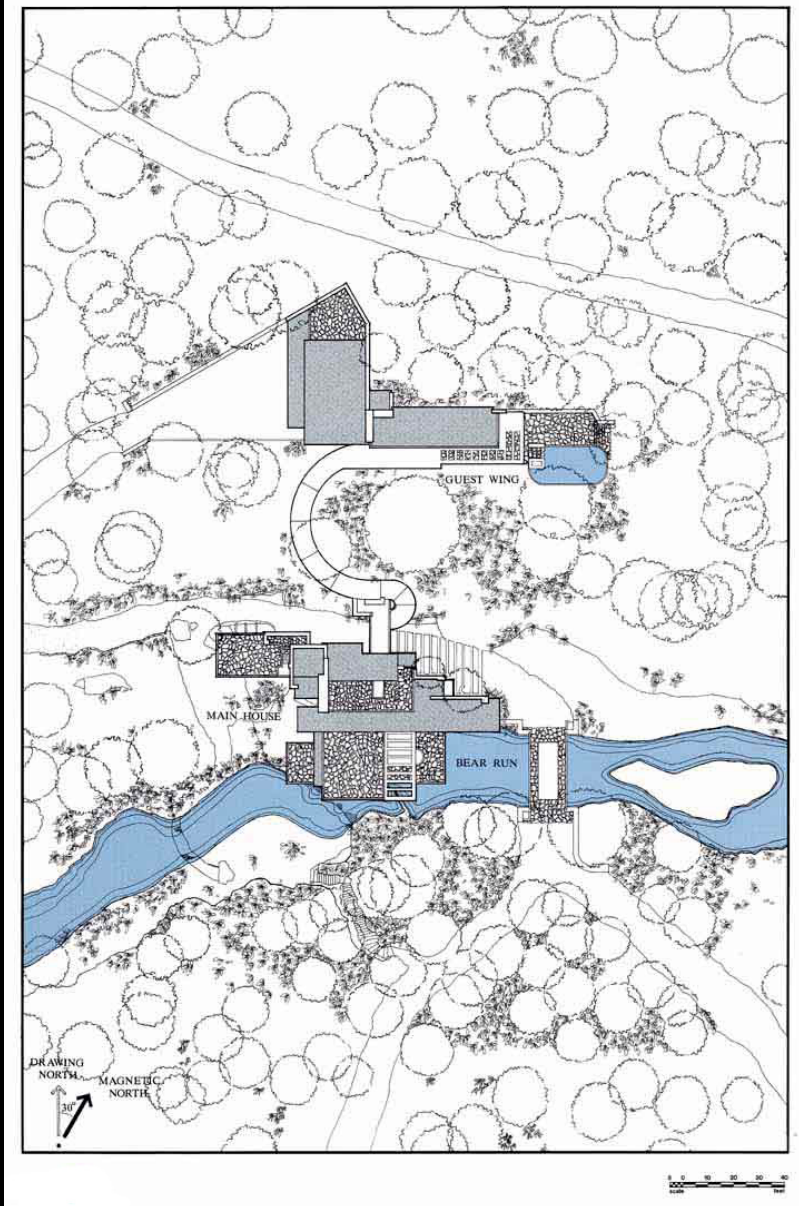
HISTORICAL BACKGROUND

- Designed by Frank Lloyd Wright in 1935 for the Kaufmann Family as a response to the family's love for the site where a waterfall was located in a rushing mountain stream.
- Great example of Wright's concept of organic architecture.



HISTORICAL BACKGROUND

- Located near Pittsburgh, Pennsylvania.
- It promotes harmony between man and nature through design.
- So well integrated with its site that the structure and surroundings are fused together in an interrelated composition.



STRUCTURAL ANALYSIS



"NATURE CANTILEVERED THOSE BOULDERS
OUT OVER THE FALL, I CAN CANTILEVER
THE HOUSE OVER THE BOULDERS."



STRUCTURAL DESIGN

- Mimics the natural pattern established by the stream's rocky ledges by placing the house over the falls in a series of cantilevered trays.
- Slabs were anchored to containment walls made out of the same sandstone found in the rock ledges.
- Structural design under the direction of the engineers: Mendel Glickman and William Peters.
- In 1963, Edgar Kaufmann, Jr. donated the house to the Western Pennsylvania Conservancy

STRUCTURAL DESIGN

- The house is anchored into the rocky mountain, its platforms jutting over the stream like rock ledges, counterweighted by the massing at the back.
- Slabs are anchored to containment walls made out of the same sandstone found in the rock ledges.



STRUCTURAL DESIGN

- Four massive concrete cantilever beams projecting 15 feet held cantilevered terraces where living room, master bedroom and adjoining terraces were located.

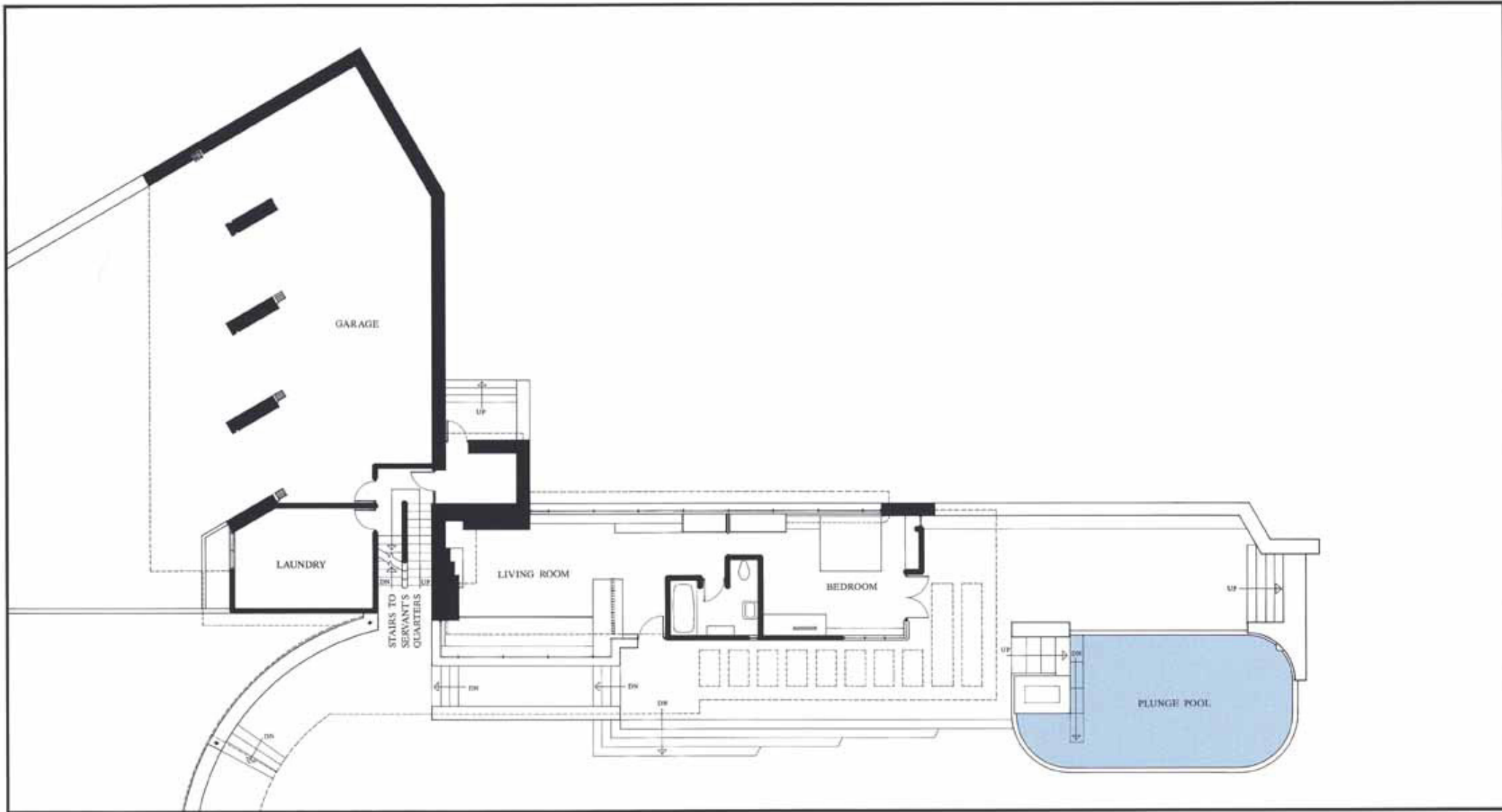


STRUCTURAL DESIGN

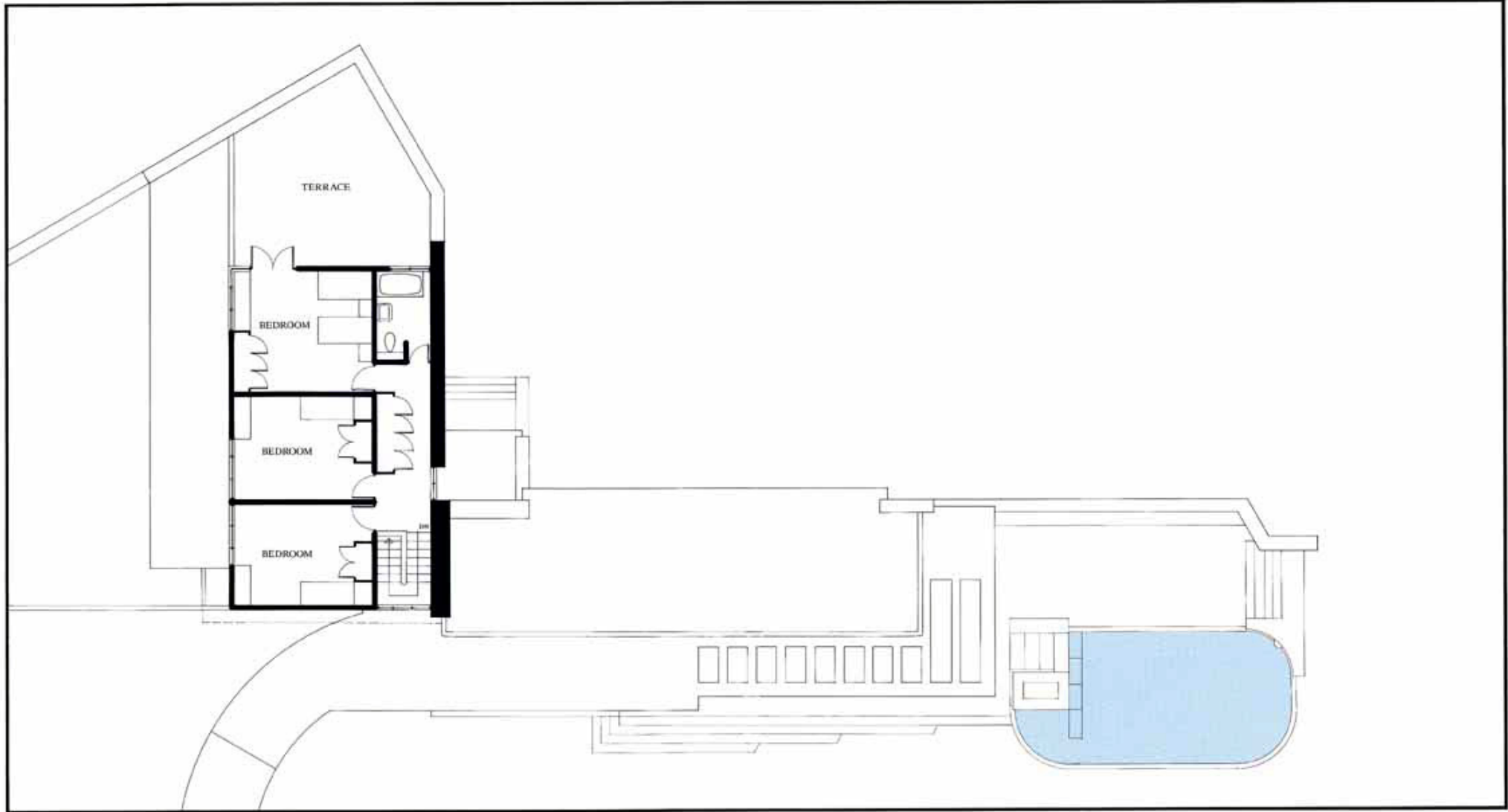
Window mullions which, at first were considered as non-structural elements were found to transfer loads from the upper terraces to the lower terrace.



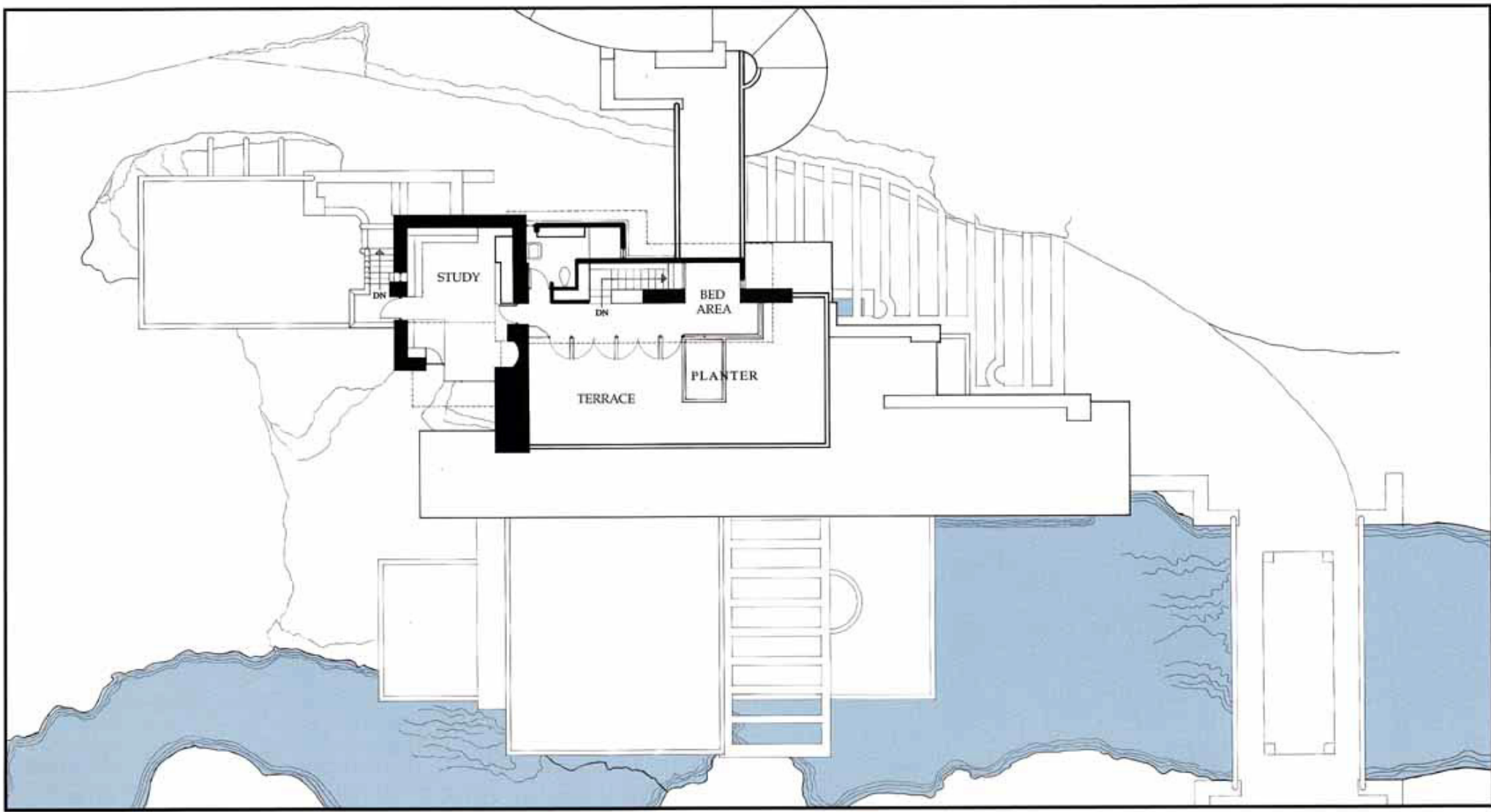
MAIN FLOOR



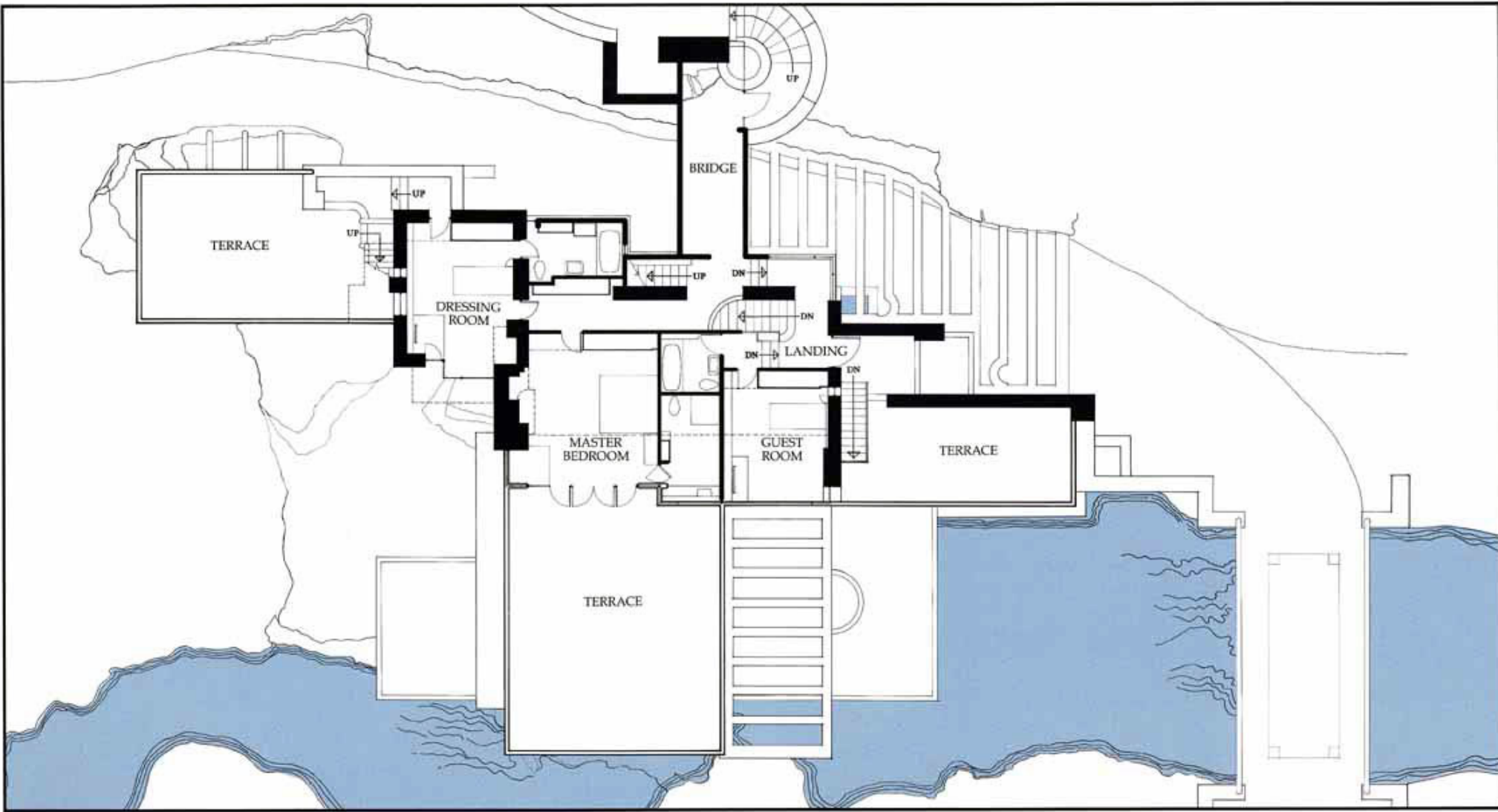
1ST FLOOR

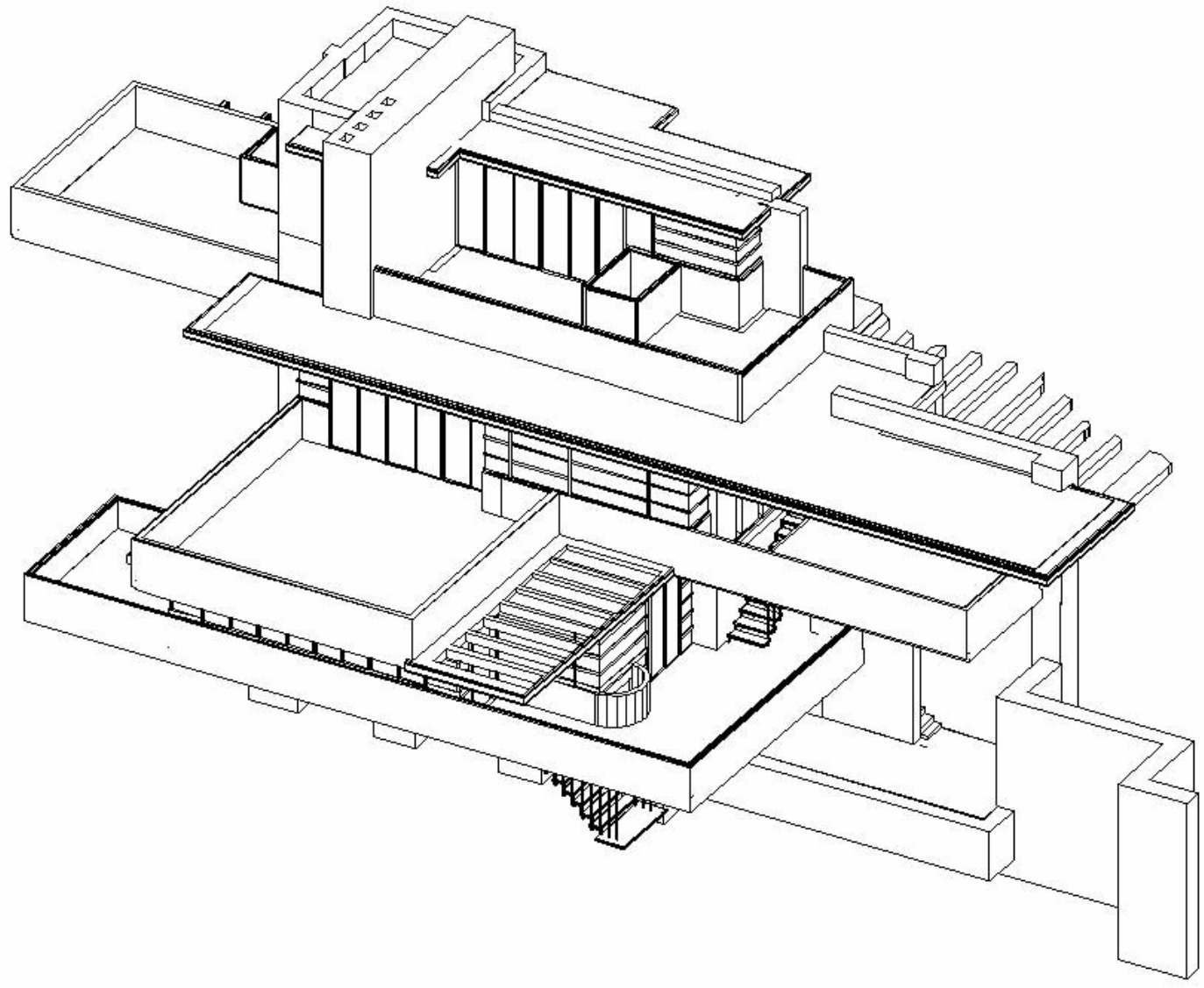


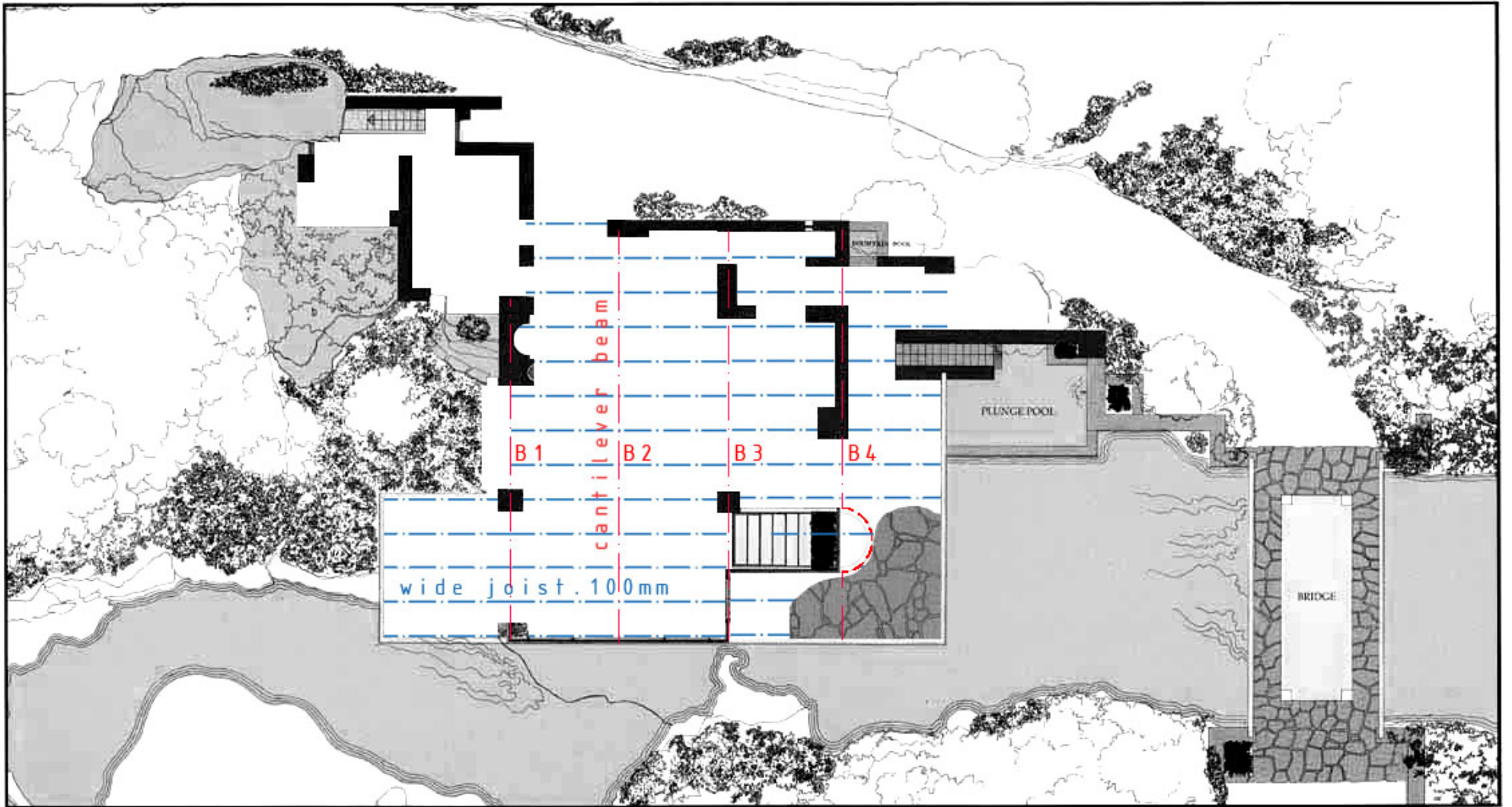
3RD FLOOR



2ND FLOOR







Structural Plan



Live Load

50 lb/ft²

Floor Slab

| | |
|--|-------------------------------|
| -Concrete Slab, stone aggregate (3 inches) | 36 lb/ft ² |
| -Stone Floor (1 in) | 12 lb/ft ² |
| -Sand | 8 lb/ft ² |
| -Plywood | 4 lb/ft ² |
| -Floor Slab | <u>+ 60 lb/ft²</u> |
| | 100 lb/ft ² |
| Total | 150 lb/ft² |

Main Beam

-3 ft x 2 ft = 6 ft²
-6 ft² x **150 lb/ft³** = 900 lb/ft

Dead Load

900 lb/ft

Parapet

-5 ft x 0.5 ft = 2.5 ft²
-2.5 ft² x **150 lb/ft³** = 375 lb/ft

Total **375 lb/ft**

Joists

4 in x 24 in = 96 in²/144 in²/ft² = 0.66 ft²
0.66 ft² x 1 ft x 150 lb/ft³ = 100 lb/ft

Total **100 lb/ft**

Numbers

Parapet

$$375 \text{ lb/ft} \times 12 \text{ ft} = 4500 \text{ lb}$$

Total **4500lb/ft**

Live Load

$$50 \text{ lb/ft}^2 \times 12 \text{ ft} = 600 \text{ lb/ft}$$

Total **600lb/ft**

Floor Slab

$$60 \text{ lb/ft}^2 \times 12 \text{ ft} = 720 \text{ lb/ft}$$

Total **720lb/ft**

Joists

$$100 \text{ lb/ft} \times 12 \text{ ft} = 1200 \text{ lb}$$

$$(1200 \text{ lb/joist} \times 15 \text{ joist}) / 52 \text{ ft} = 346 \text{ lb/ft}$$

Total **346lb/ft**

Beam - Parapet

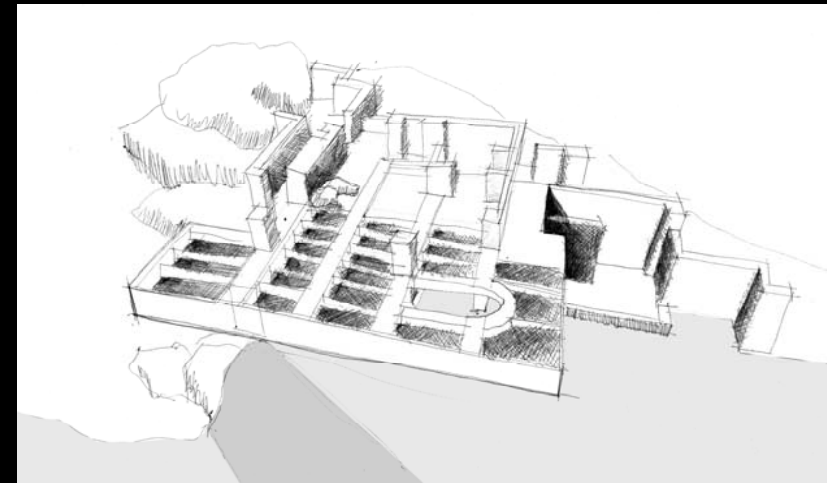
$$375 \text{ lb/ft} \times 2 = 750 \text{ lb/ft}$$

Total **750lb/ft**

Dead Load: **1816 lb/ft**

$$W_{\text{beam 1}} = \text{Live Load} + \text{Dead Load}$$

$$W_{\text{beam 1}} = 600 \text{ lb/ft} + 1816 \text{ lb/ft} = \underline{\underline{2416 \text{ lb/ft}}}$$



Parapet

$$375 \text{ lb/ft} \times 18 \text{ ft} = 6750 \text{ lb/ft}$$

Total **6750lb/ft**

Live load

$$50 \text{ lb/ft}^2 \times 18 \text{ ft} = 900 \text{ lb/ft}$$

Total **900lb/ft**

Floor Slab

$$60 \text{ lb/ft}^2 \times 18 \text{ ft} = 1080 \text{ lb/ft}$$

Total **1080lb/ft**

Joists

$$100 \text{ lb/ft} \times 18 \text{ ft} = 1800 \text{ lb}$$

$$(1800 \text{ lb/joist} \times 5 \text{ joist}) / 15 \text{ ft} = 600 \text{ lb/ft}$$

Total **600lb/ft**

Main Beam

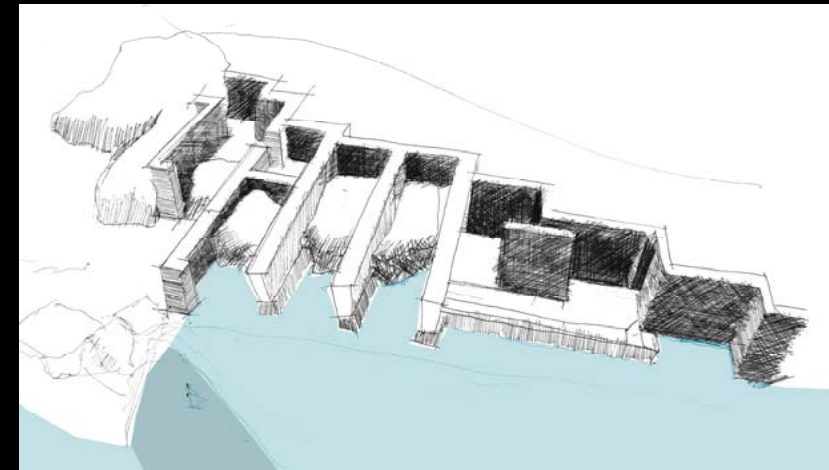
900 lb/ft

Dead Load

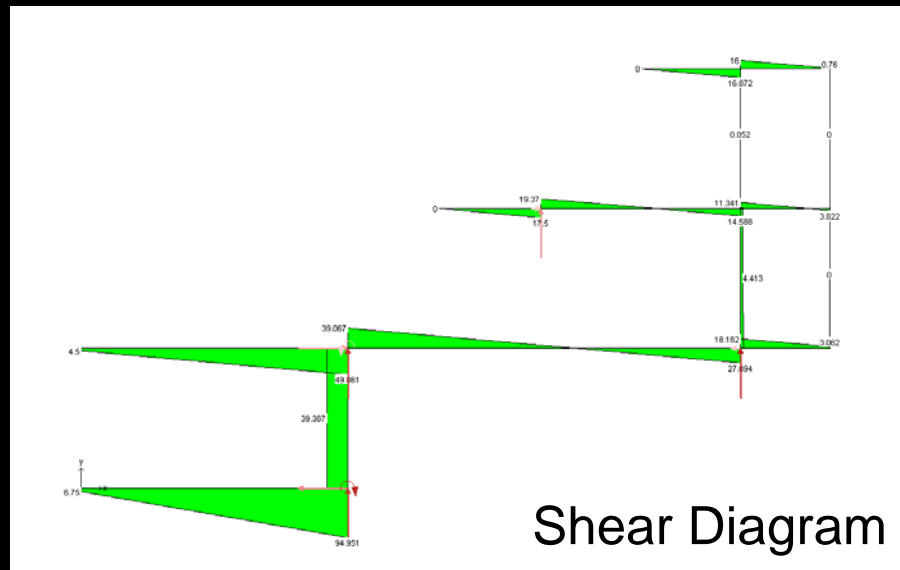
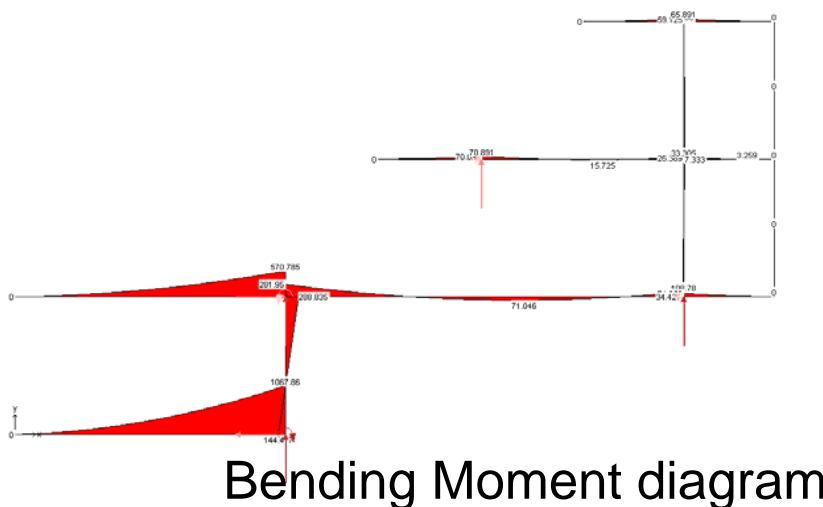
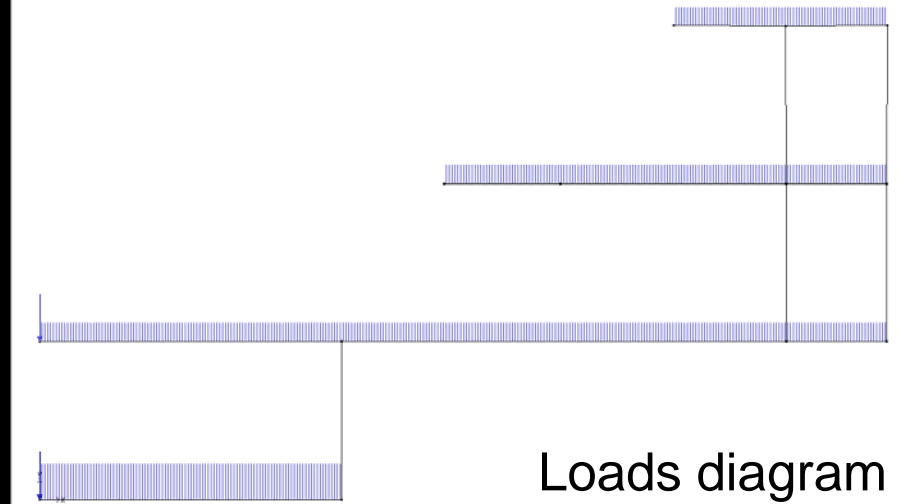
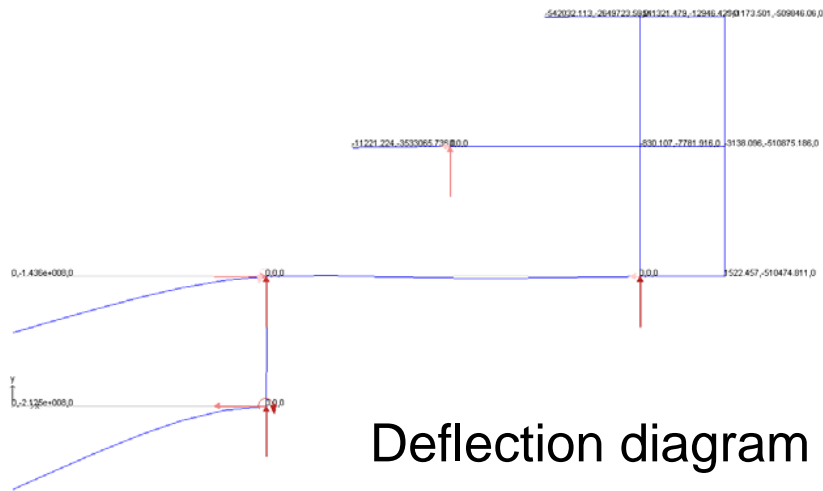
2580 lb/ft

$$W \text{ beam } 1 = \text{Live Load} + \text{Dead Load}$$

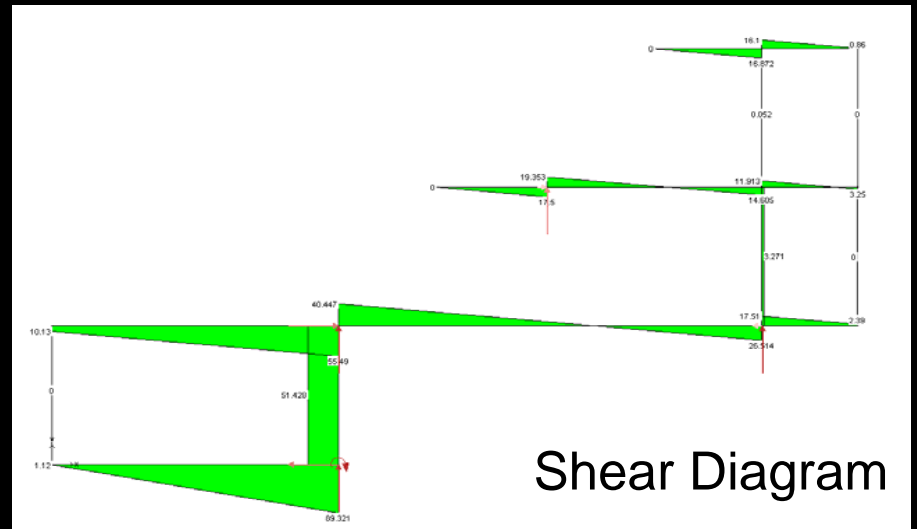
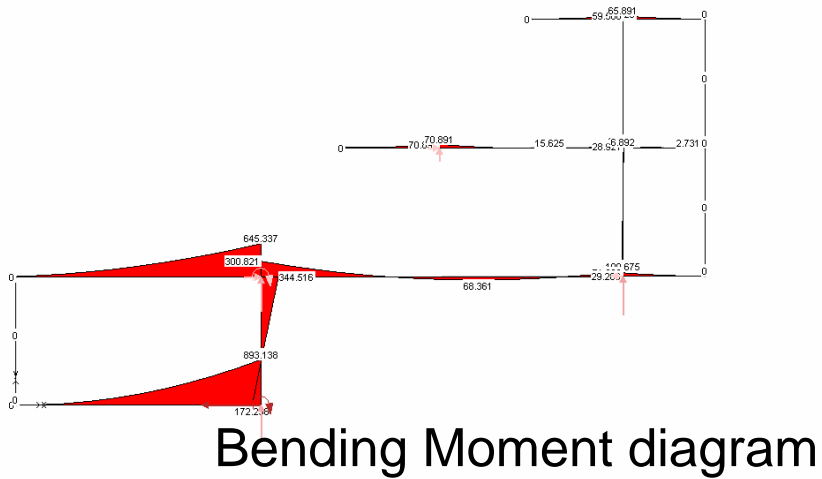
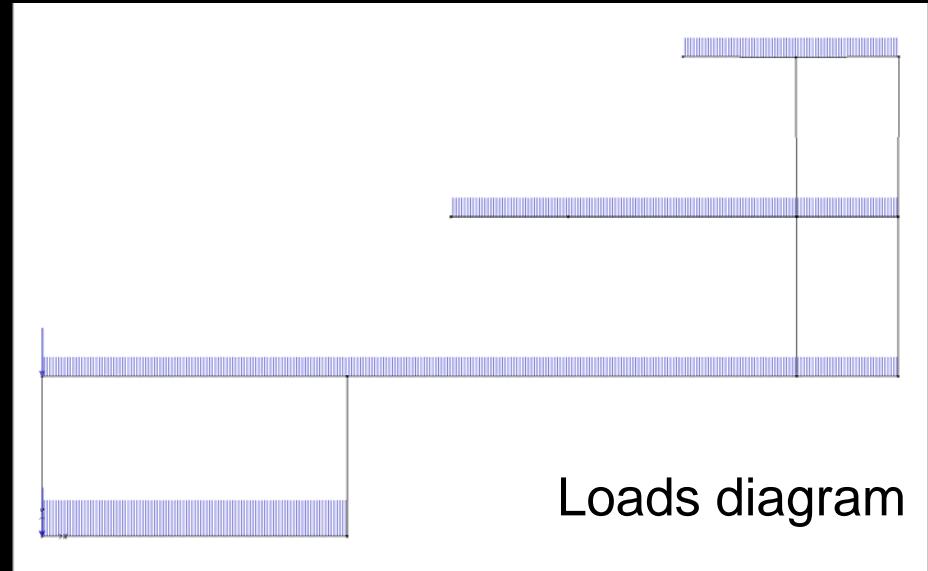
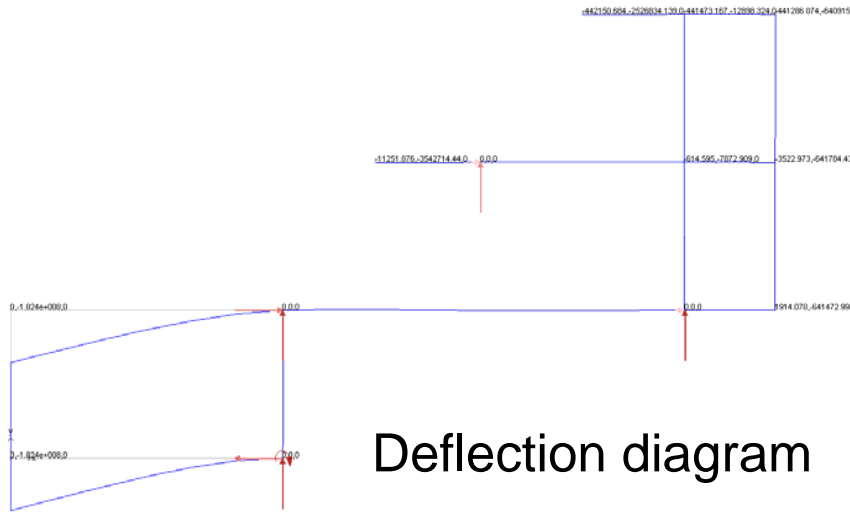
$$W \text{ beam } 1 = 900 \text{ lb/ft} + 2580 \text{ lb/ft} = \underline{\underline{3480 \text{ lb/ft}}}$$



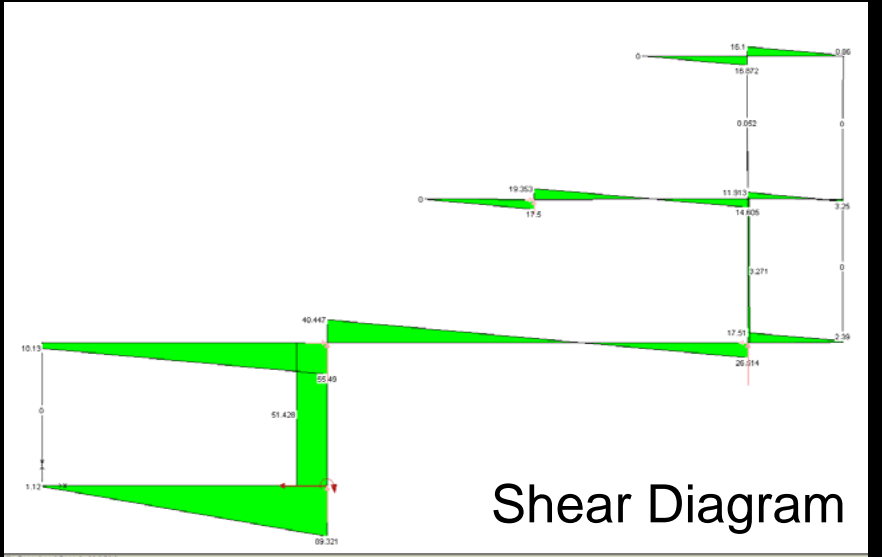
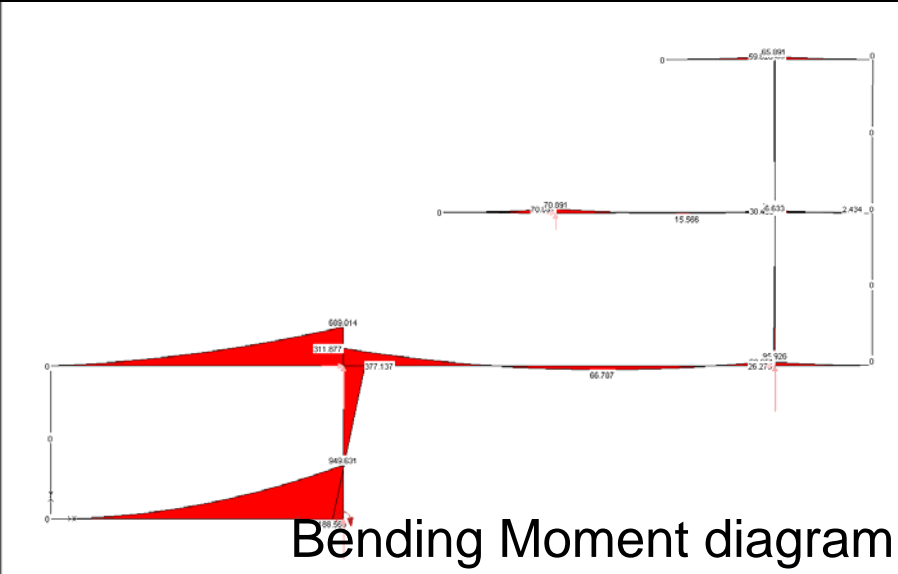
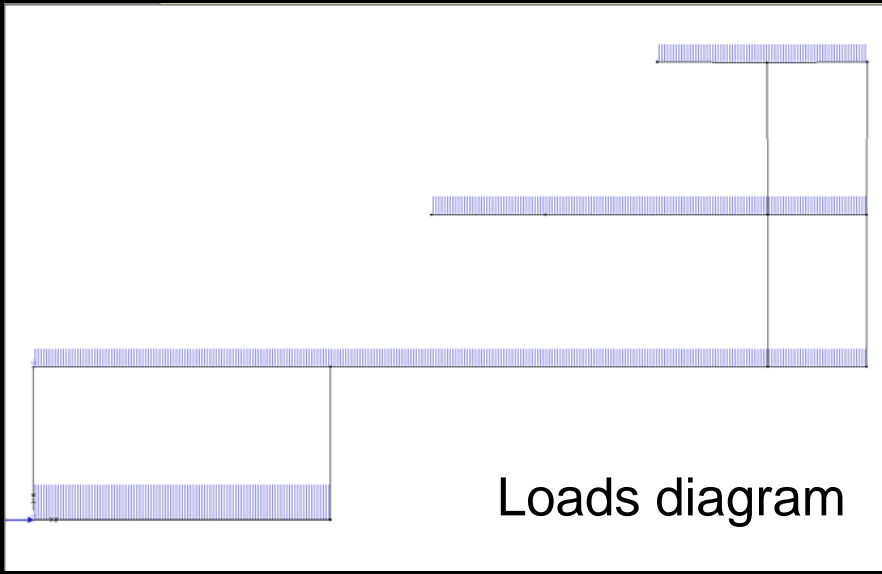
Multi-Frame :1 case



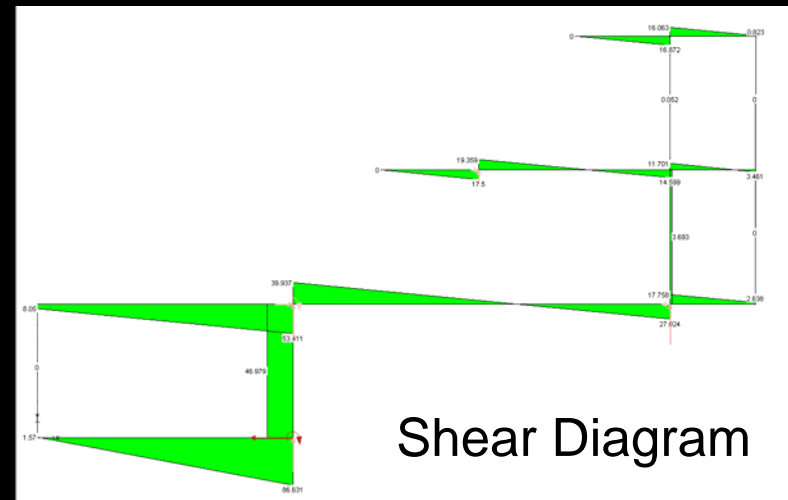
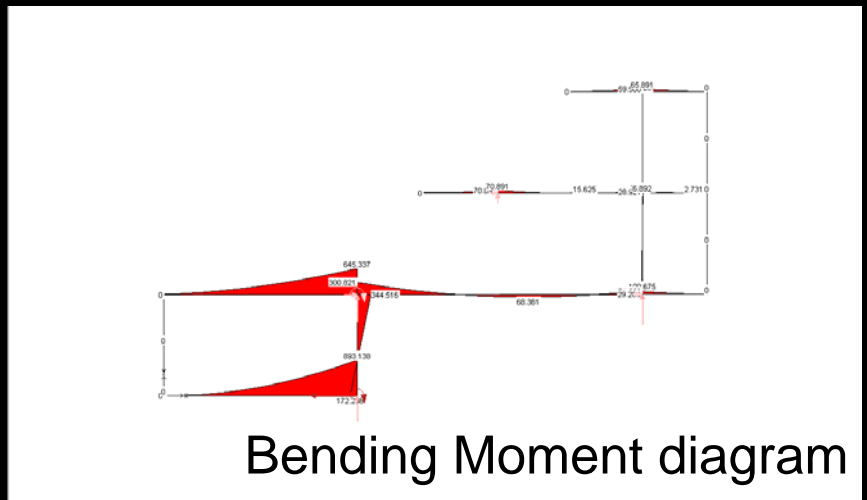
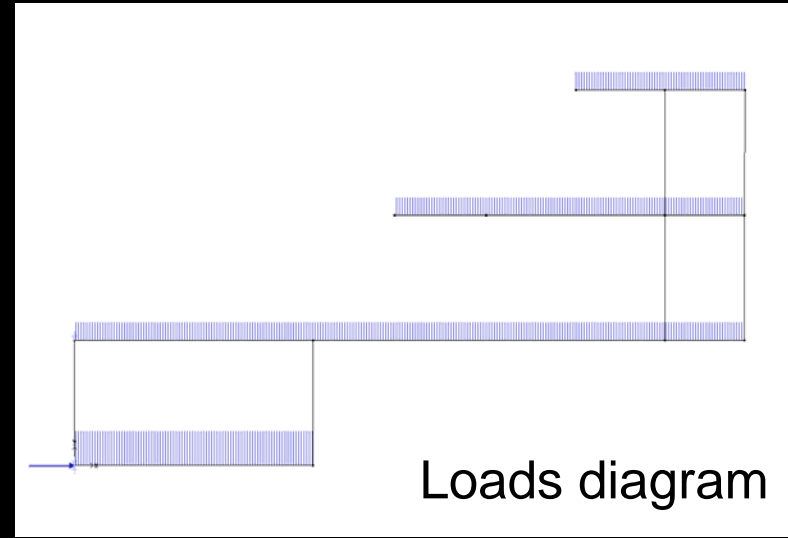
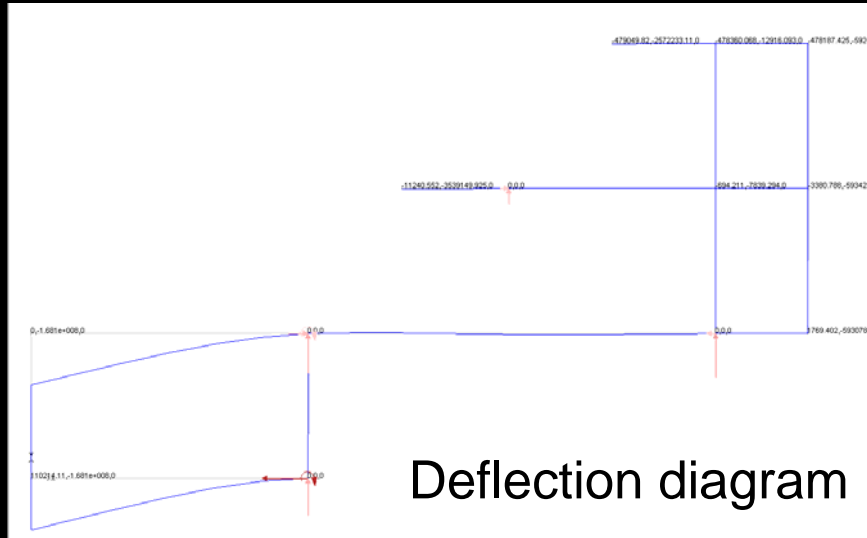
Multi-Frame: 2 case



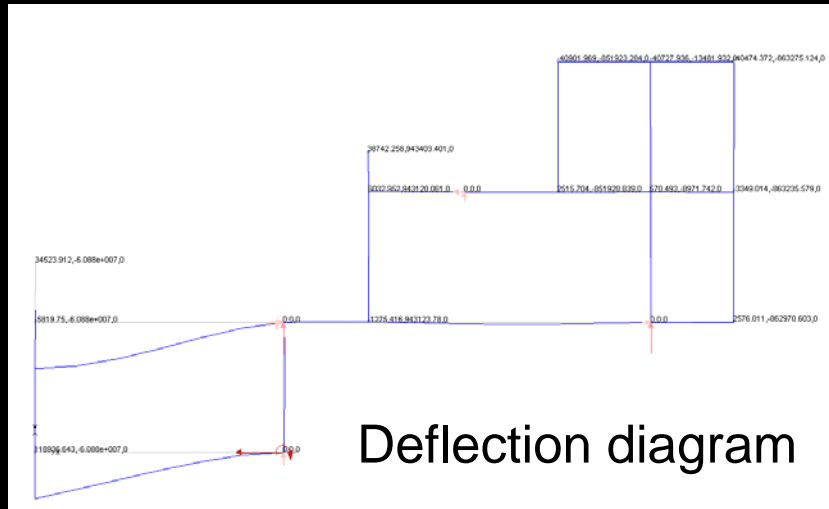
Multi-Frame: 3 case



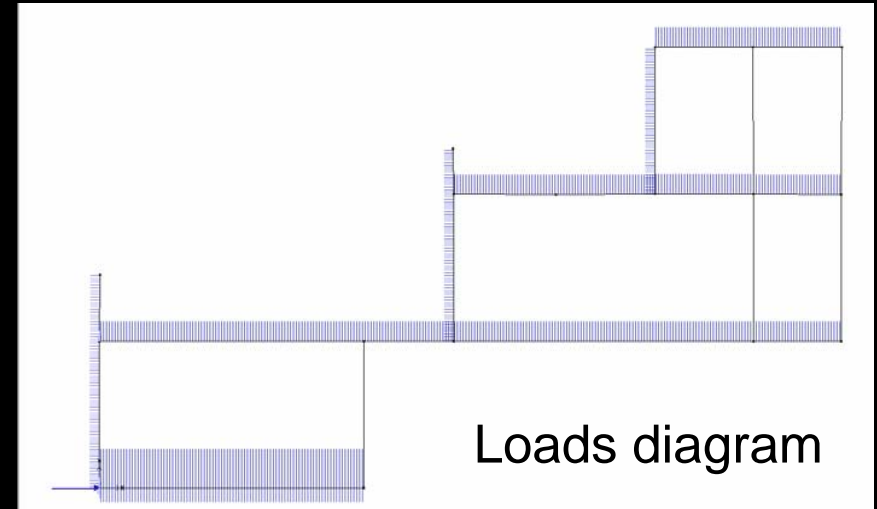
Multi-Frame: 4 case



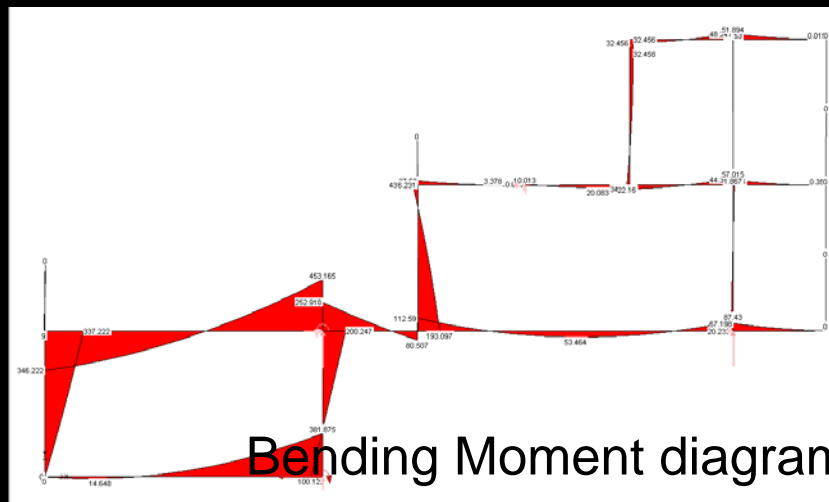
Multi-Frame: 5 case



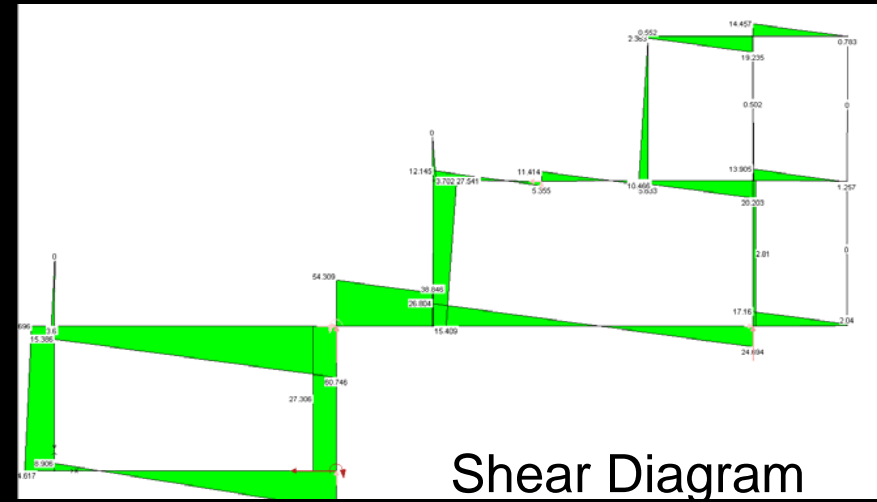
Deflection diagram



Loads diagram



Bending Moment diagram



Shear Diagram

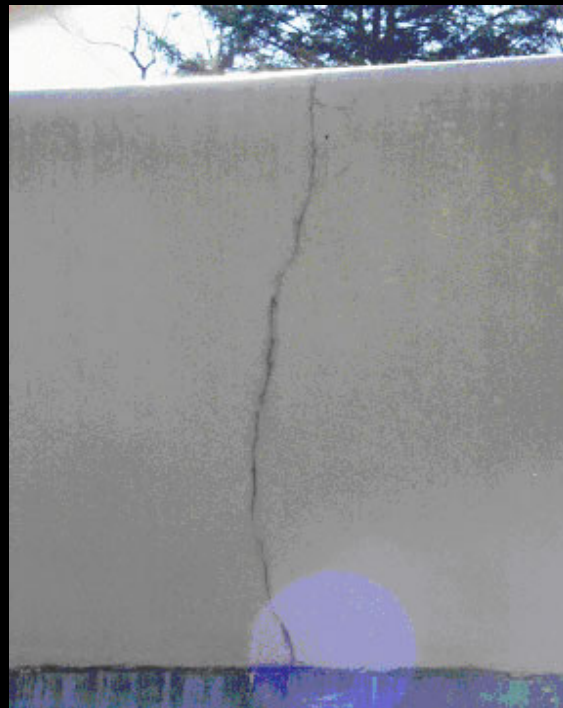
STRUCTURAL FAILURE



PROBLEMS

The initial deflection occurred during construction when the forms were not designed to carry the wet weight of the concrete.

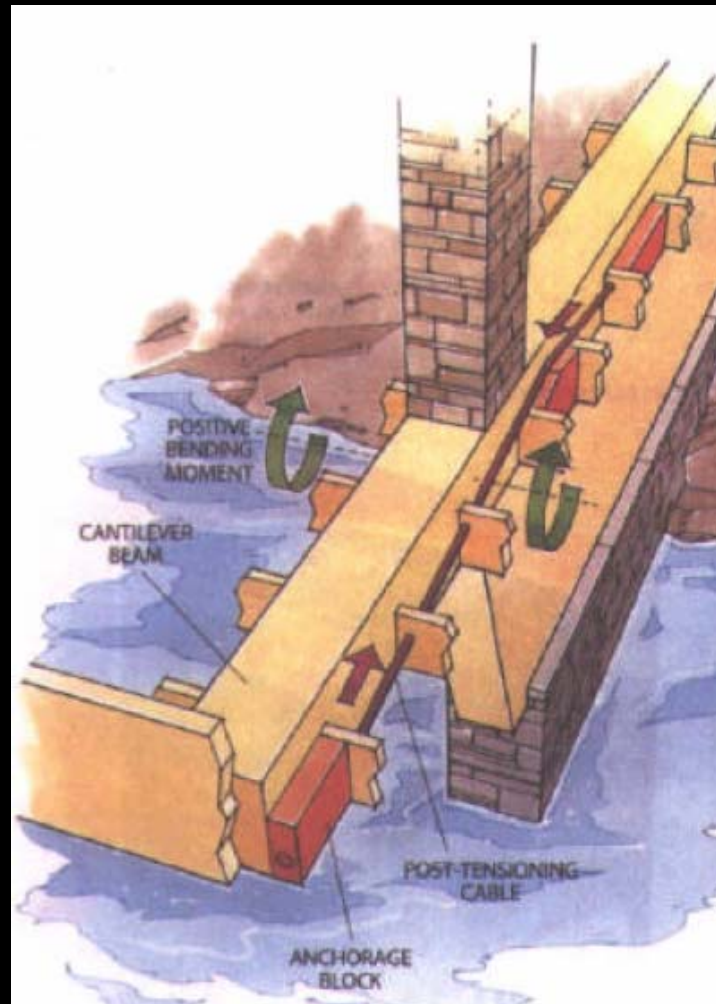
The secondary problem was the lack of tensile reinforcement in the beams that carried the first and second story terrace cantilevers. The amount of steel was doubled during construction to sixteen, one inch, steel bars. This still was not enough to carry the load.



The major problem occurs in the lowest terrace with a fifteen foot cantilever. Because the tensile reinforcement was not adequate for the second story terrace, the living room window mullions transfer the weight downward to the first level terrace. The first level terrace was also insufficiently reinforced and could not support its own load plus the additional weight of the second story terrace.



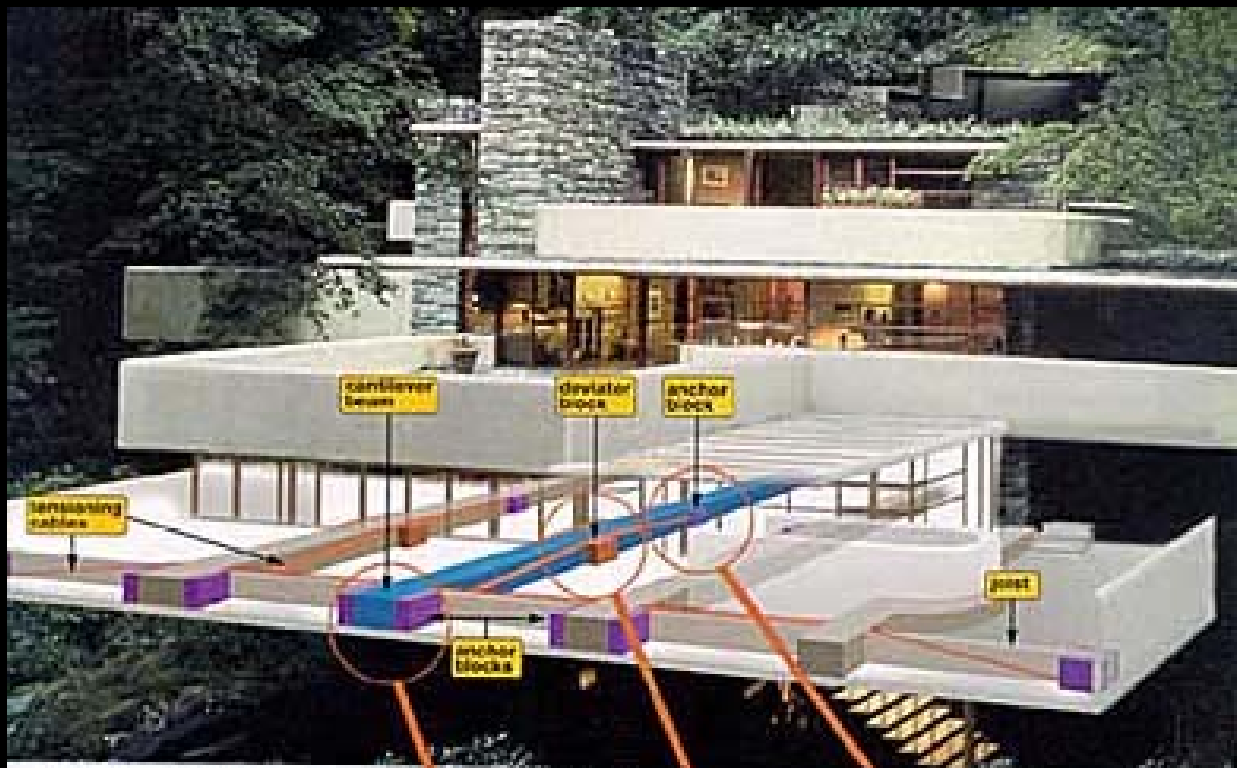
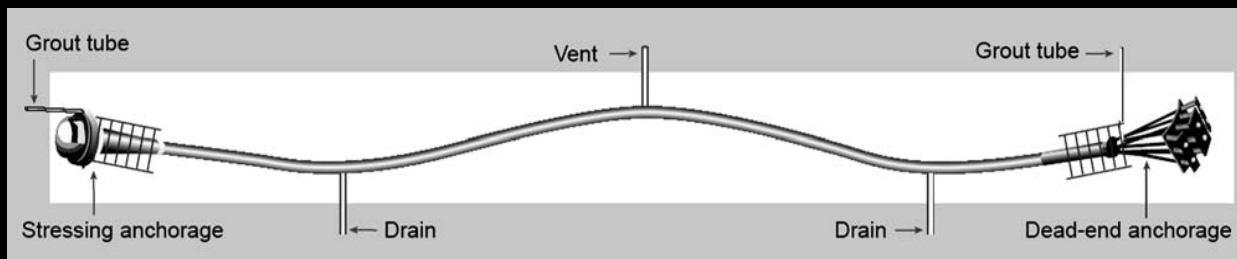
SOLUTION



In 1999 plans were approved for the structural strengthening of the living room terrace with post-tensioning and waterproofing.

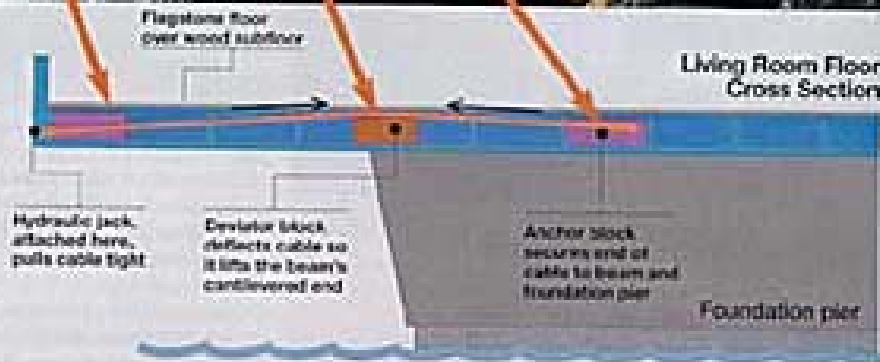
The goal was to prevent further deflection and not to raise the terrace so further structural damage was not caused to the building.





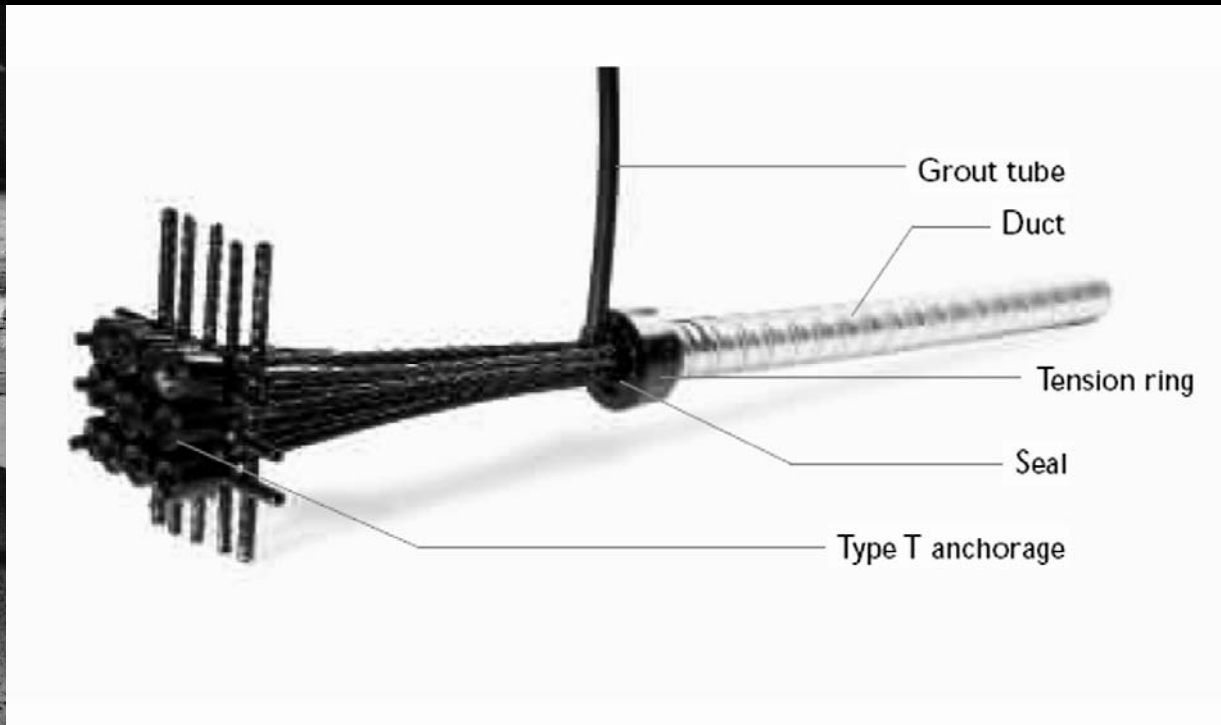
How They Did It

Beneath the living room floor at Fallingwater, as shown in the illustrations above and at right, a network of steel cables was installed to stabilize the sagging cantilever beams and joints. The cross section shows how a cable is run alongside the central beam, anchored at each end, and positioned over a deviator block in the middle. When tension was applied with a hydraulic jack (photo, bottom right), the cable tightened, lifting the end of the cantilever beam. The photo below shows the work in progress.



High strength cables will be placed through the main three beams of the lower terrace in a bent path so the cables will be placed at the top and bottom of the beam where they are most needed.

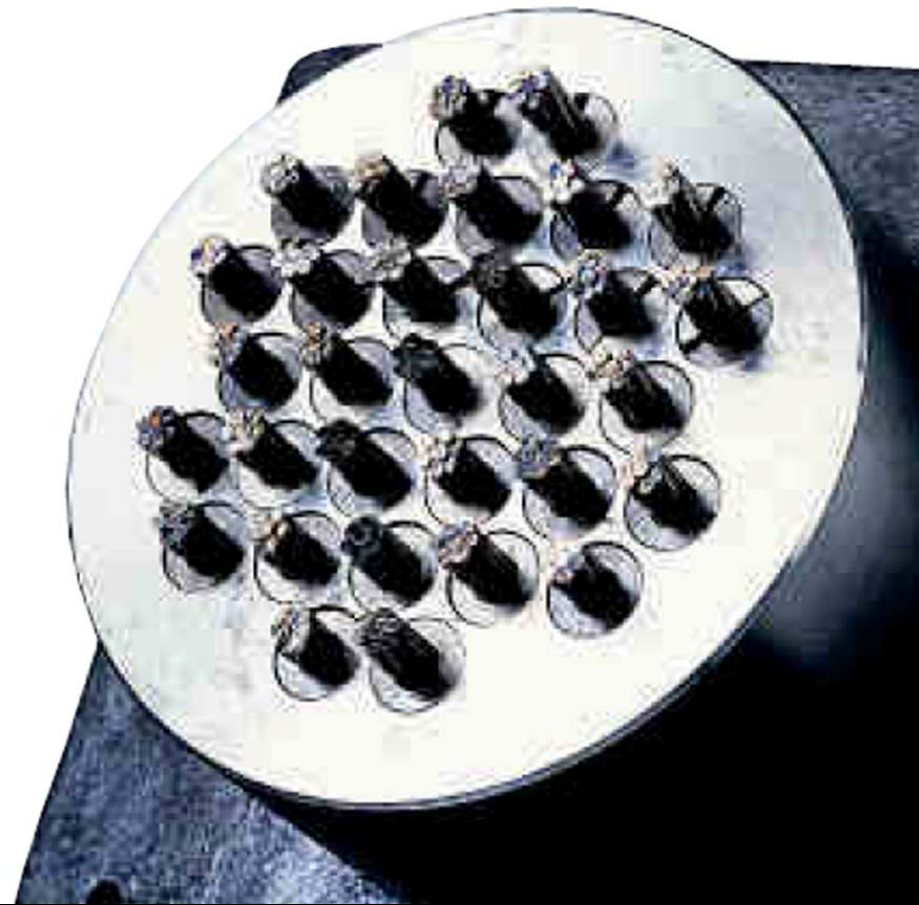
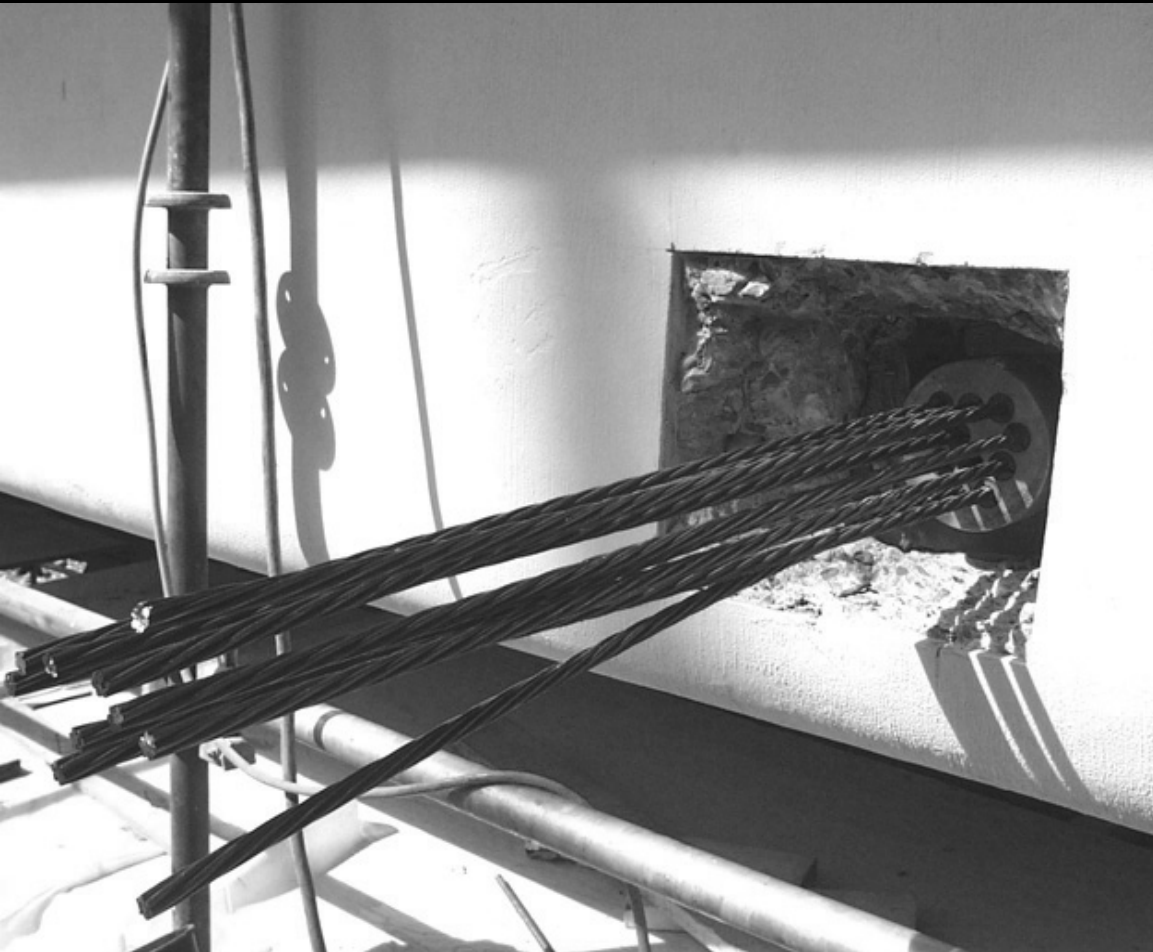
An anchoring block will be installed at one end of the beam to anchor the tensioning of the cable.



Post-tensioning jacks will be placed on the opposite end to tension the cable to 100 tons of force.



This will add 200 tons of tension to each beam.



Steel channels will be placed on both sides of the concrete joist directly above the living room window mullions to help transfer the load from the second story terrace to the living room terrace.

