ARCH 631. Assignment #2

Date: 8/29/13, *due* 9/12/13

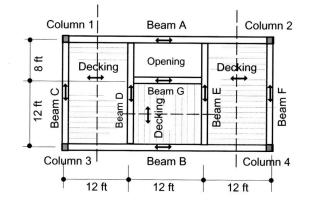
Worth 25 pts.

Problems:

- 1. Complete text problem 3.6 on page 120.
 - **3.6** Determine the reactions to Beam D in Figure 3.22 Assume that the average dead and live load is 60 lbs/ft^2 .

Answers: 4896 lb., 4464 lb.

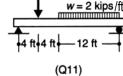
Figure 3.22



2. What is the maximum bending moment in Beam D of Figure 3.22 for the load given in Problem 1. Answer: 27,648 lb-ft

3. Complete text problem 2.20 on page 85.

2.20 Draw shear and moment diagrams for the beam analyzed in Question 2.11 [Figure 2.59 (Q11)]. Answer: $V_{\text{max}} = 17.8$ k and $M_{\text{max}} = 79.2$ ft-k.



4. Complete text problem 6.7 on page 273. (Note: The answer for deflection should be 0.27 in.)

6.7 A simply supported beam 12 ft long carries a uniformly distributed load of 100 lb/ft. Assume that the beam is $1^{1}/_{2}$ in. $\times 9^{1}/_{2}$ in. in cross section and is laterally braced. Assume also that the beam is made of timber that has an allowable stress in bending of 1200 lb/in.² and in shear of 150 lb/in.² Is the beam safe with respect to bending and shear stress considerations? What is the maximum deflection of the beam? Assume that $E = 1.6 \times 10^{6}$ lb/in.² Is this deflection acceptable?

Answer: $(f_b = 959) < (F_b = 1200)$, \therefore safe in bending; $(f_v = 63.1) < (F_v = 150)$, \therefore safe in shear; and (0.29) < (L/240 = 0.6), \therefore deflections are okay. (0.27) 5. Complete text problem 6.8 on page 273 for ASD only. (See Table A.17.1 provided.)

6.8. A simply supported steel beam will be used to span 30 ft and to support a uniformly distributed live load of 400 lb/ft. Assume that the yield stress in bending is 50,000 lb/in.², and that allowable bending stress is 33,000 lb/in.² Use both ASD and LRFD methods to determine the most efficient wide-flange shape to be used, based on a bending-stress analysis. Assume a factor of 1.6 for live loads when using LRFD methods, and use one of the shapes listed in Appendix 17. Ignore dead loads.

Partial Answer: $S_{req'd} \ge 16.4 \text{ in}^3$

- **6.** What is the SEI/ASCE 7-10 letter designation for roof live load? Provide the factored load combinations expressions for *strength design* that it is used in.
- 7. What is the minimum required live load for occupancy or use of a bowling alley, a cell block in a penal institution, and a operating room of a hospital? How much area dead load from weight would you design the cell block walls for if they are to be 8 in. concrete hollow block with light aggregate?

| | | | operties AXIS X-X | | | | | | | | AXIS X-X |
|-------------------|-------------|--------------------------|---------------------------|---------------------------|----------------------|---------------------------|------------------------------------|------------------------------------|----------------------|------------------------------------|------------------------|
| | | | | | | AXIS Y-Y | | | | AXIS X-X | |
| Shape | Area (in.²) | Web Thick- ness (in.) | I_x (in. ⁴) | S_x (in. ³) | r _x (in.) | Z_x (in. ³) | I _x (in. ⁴) | S _x (in. ³) | r _x (in.) | Z _x (in. ³) | $S_x (mm^3 \times 10)$ |
| W 36 $	imes$ 282 | 82.9 | 0.885 | 19,600 | 1050 | 15.4 | 1,190 | 1200 | 144 | 3.8 | 223 | 17190 |
| W 33 $	imes$ 201 | 59.2 | 0.715 | 11,600 | 686 | 14 | 773 | 749 | 95.2 | 3.56 | 147 | 11231 |
| W 30 $	imes$ 99 | 29.1 | 0.520 | 3990 | 269 | 11.7 | 312 | 128 | 24.5 | 2.1 | 38.6 | 4404 |
| W 27 \times 102 | 30 | 0.515 | 3620 | 267 | 11 | 305 | 139 | 27.8 | 2.15 | 27.8 | 4371 |
| W 14 $	imes$ 90 | 26.5 | 0.440 | 999 | 143 | 6.14 | 157 | 362 | 49.9 | 3.7 | 75.6 | 2341 |
| W 21 $	imes$ 68 | 20 | 0.430 | 1480 | 140 | 8.6 | 160 | 64.7 | 15.7 | 1.8 | 24.4 | 2292 |
| W 14 $	imes$ 82 | 24 | 0.510 | 881 | 123 | 6.05 | 139 | 148 | 29.3 | 2.48 | 44.8 | 2014 |
| W 14 $	imes$ 74 | 21.8 | 0.450 | 795 | 112 | 6.04 | 126 | 134 | 26.6 | 2.48 | 40.5 | 1834 |
| W 18 $	imes$ 60 | 17.6 | 0.415 | 984 | 108 | 7.47 | 123 | 50.1 | 13.3 | 1.69 | 20.6 | 1768 |
| W 16 $	imes$ 50 | 14.7 | 0.380 | 659 | 81 | 6.68 | 92 | 37.2 | 10.5 | 1.59 | 16.3 | 1326 |
| W 12 $	imes$ 26 | 7.65 | 0.230 | 204 | 33.4 | 5.17 | 37.2 | 17.3 | 5.34 | 1.51 | 8.17 | 547 |
| W 8 \times 31 | 9.12 | 0.285 | 110 | 27.5 | 3.47 | 30.4 | 37.1 | 9.27 | 2.02 | 14.1 | 450 |
| W 10 $	imes$ 22 | 6.49 | 0.240 | 118 | 23.2 | 4.27 | 26 | 11.4 | 3.97 | 1.33 | 6.1 | 380 |
| W 8 \times 24 | 7.08 | 0.245 | 82.7 | 20.9 | 3.42 | 23.1 | 18.3 | 5.63 | 1.61 | 8.57 | 342 |
| W 8 $	imes$ 18 | 5.26 | 0.230 | 61.9 | 15.2 | 3.43 | 17 | 7.97 | 3.04 | 1.23 | 4.66 | 249 |
| $C 9 \times 15$ | 4.41 | 0.285 | 51 | 11.3 | 3.4 | 13.6 | 1.91 | 1.01 | 0.661 | 2.04 | 185 |
| $C 6 \times 13$ | 3.81 | 0.437 | 17.3 | 5.8 | 2.13 | 7.29 | 1.05 | 0.642 | 0.525 | 1.35 | 95 |

The beams in the table are arranged according to their relative S values in descending order of magnitude. The first entry in a group represents a light member and one with a relatively large section modulus. It is thus an efficient and often preferred member. W: wide-flange shape; C: channel shape; MC: miscellaneous shape; WT: structural tees cut from W shapes. Typical designation: W 21×68

weight per linear foot, lb