

APPLIED ARCHITECTURAL STRUCTURES:  
STRUCTURAL ANALYSIS AND SYSTEMS

ARCH 631

DR. ANNE NICHOLS

FALL 2012

lecture  
twenty three

masonry  
construction



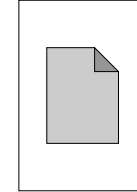
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Learning Evaluation



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Masonry

- columns
- beams
- arches
- walls
- footings



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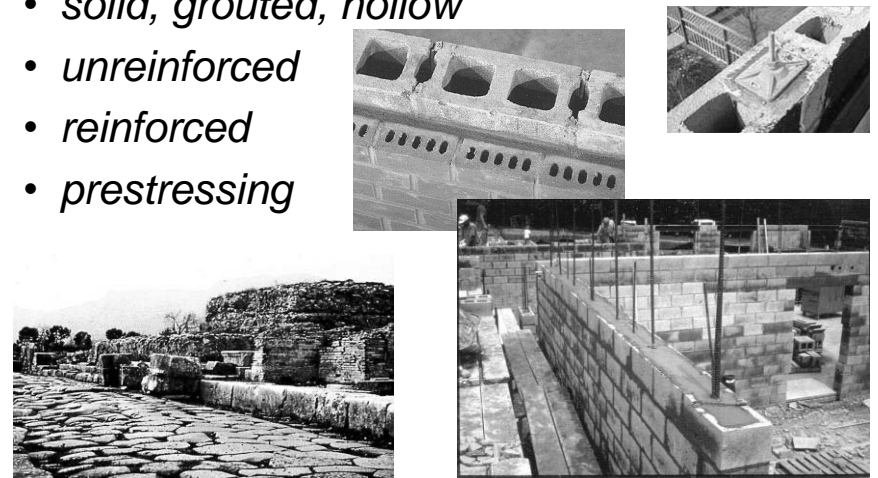
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Masonry Construction

- solid, grouted, hollow
- unreinforced
- reinforced
- prestressing



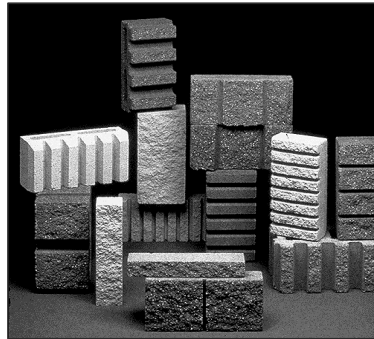
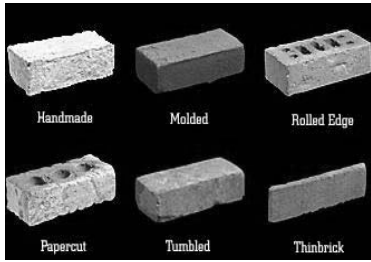
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# Masonry Materials

- brick
- concrete masonry units



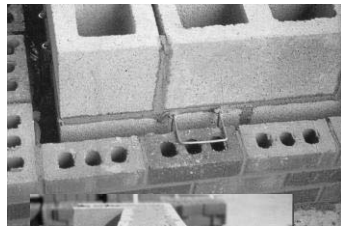
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# Masonry Materials

- reinforcement
  - deformed bars
  - prestressing strand
  - development length
  - anchorage
  - splices
  - ties
- steel or composite



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# Masonry Materials

- mortar
  - water, masonry cement, sand, lime
  - types:
    - M *higher strength – 2500 psi (ave.)*
    - S *medium high strength – 1800 psi*
    - N *medium strength – 750 psi*
    - O *medium low strength – 350 psi*
    - K *low strength – 75 psi*



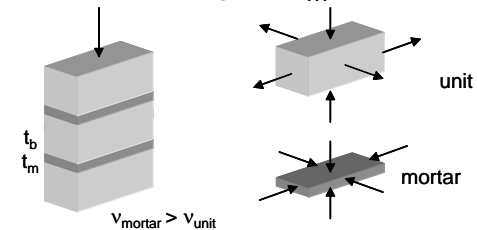
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# Masonry Materials

- grout
  - high slump concrete
  - fills voids and fixes rebar
- prisms
  - used to test strength,  $f'_m$



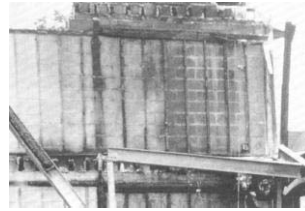
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# Masonry Materials

- **fire resistance**
  - fire-resistive structural material
  - details important to prevent leaks or cracks
  - retains strength if exposure not too long
    - mortar and cmu's dehydrate
    - loses 30-60% after that
  - no toxic fumes
  - cover necessary to protect steel

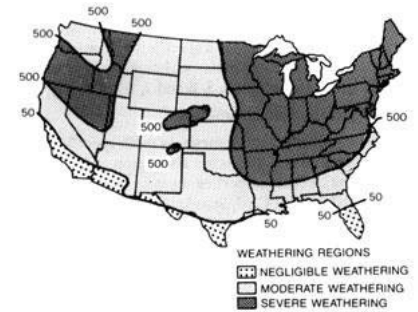


# Masonry Walls

- based on empirical requirements for minimum wall thickness and height
  - $h/t < 25$  (UBC 2105.2  $h/t < 35$ )
- wall thicknesses often increased by 4"/story
- bearing walls > 3-5 stories uneconomical, steel or concrete frames used
- strength design limit states:
  - serviceability: deflection
  - ultimate: compression & tension

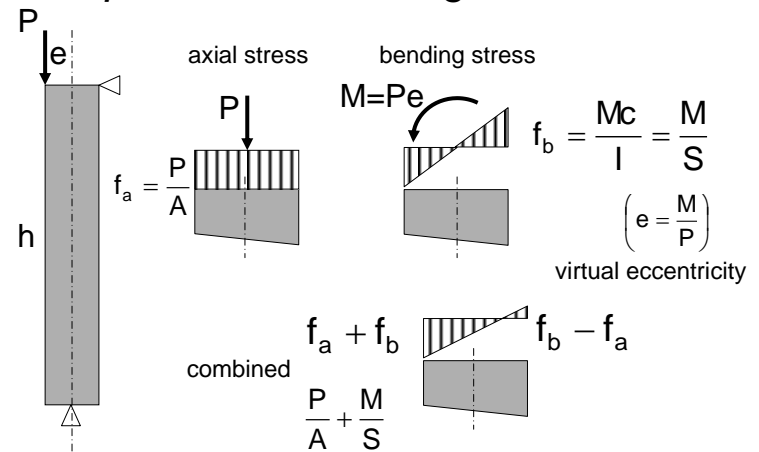
# Masonry Materials

- **moisture resistance**
  - weathering index for brick
  - bond and detailing
  - expansion or shrinking from water
    - provide control joints
    - parapets, corners, long walls



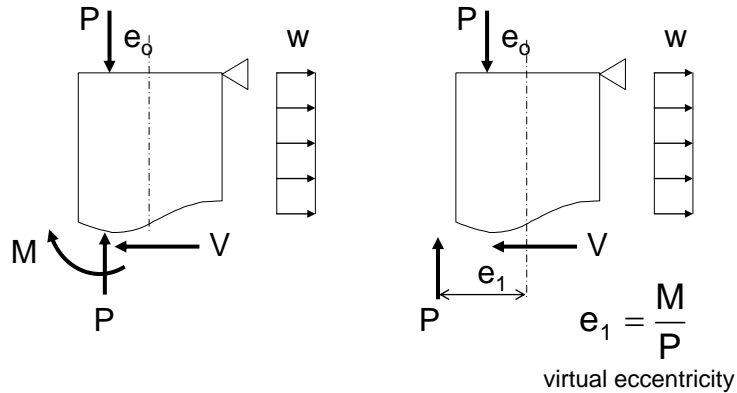
# Masonry Walls

- **compression + bending**



# Masonry Walls

- equivalent eccentricity with lateral load



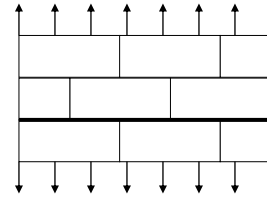
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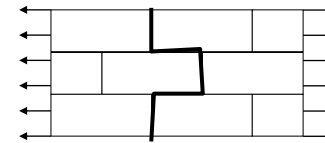
# Masonry Walls

tension normal to bed joints

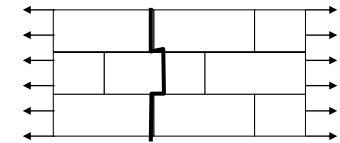


Not allowed in MSJC code

tension parallel to bed joints



strong units



weak units

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# Masonry Beam & Wall Design

- MSJC (ACI, ASCE, TMS)



- limit tensile stress in mortar
- working stress design (ASD)

- linear stresses in masonry
- no tension in masonry when reinforced
- elastic stress in steel <  $f_y$
- additional compression in walls

– masonry strength =  $f'_m$



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# Masonry Beam & Wall Design

- reinforcement increases capacity & ductility

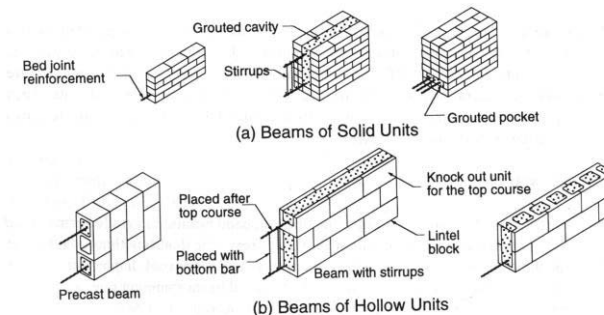


Figure 2.10 Reinforced masonry beams and lintels.

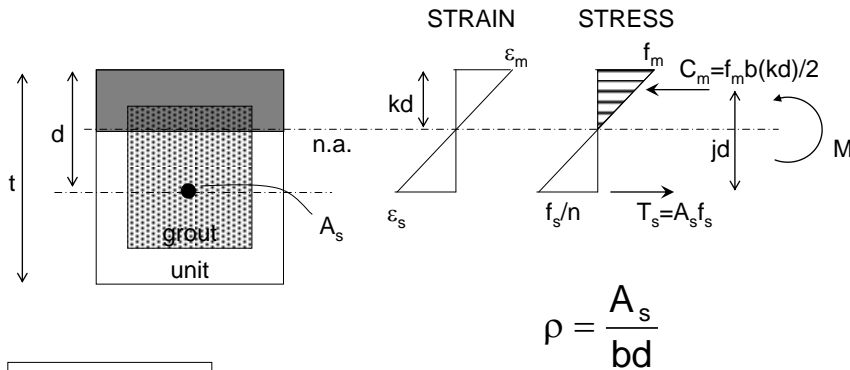
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# Masonry Design

- $f_s$  is not the yield stress
- $f_m$  is the stress in the masonry



BIA Teknote 17 series

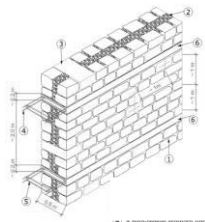
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# Allowable Masonry Stresses

- flexure
  - $F_b = 1/3 f'_m$  (unreinforced)
  - $F_b = 0.45 f'_m$  (reinforced)
- shear, unreinforced masonry
  - $F_v = 1.5 \sqrt{f'_m} \leq 120 \text{ psi}$
- shear, reinforced masonry
  - $M/Vd \leq 0.25: F_v = 3.0 \sqrt{f'_m}$
  - $M/Vd \geq 1.0: F_v = 2.0 \sqrt{f'_m}$



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# Allowable Masonry Stresses

- tension - unreinforced only

Table 2.2.3.2 — Allowable flexural tensile stresses for clay and concrete masonry, psi (kPa)

Direction of flexural tensile stress and masonry type	Mortar types			
	Portland cement/lime or mortar cement		Masonry cement or air entrained portland cement/lime	
	M or S	N	M or S	N
Normal to bed joints				
Solid units	53 (366)	40 (276)	32 (221)	20 (138)
Hollow units <sup>1</sup>				
UngROUTED	33 (228)	25 (172)	20 (138)	12 (83)
Fully grouted	86 (593)	84 (579)	81 (559)	77 (531)
Parallel to bed joints in running bond				
Solid units	106 (731)	80 (552)	64 (441)	40 (276)
Hollow units				
UngROUTED and partially grouted	66 (455)	50 (345)	40 (276)	25 (172)
Fully grouted	106 (731)	80 (552)	64 (441)	40 (276)
Parallel to bed joints in masonry not laid in running bond				
Continuous grout section parallel to bed joints	133 (917)	133 (917)	133 (917)	133 (917)
Other	0 (0)	0 (0)	0 (0)	0 (0)

<sup>1</sup> For partially grouted masonry, allowable stresses shall be determined on the basis of linear interpolation between fully grouted hollow units and ungrouted hollow units based on amount (percentage) of grouting.

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# Allowable Reinforcement Stress

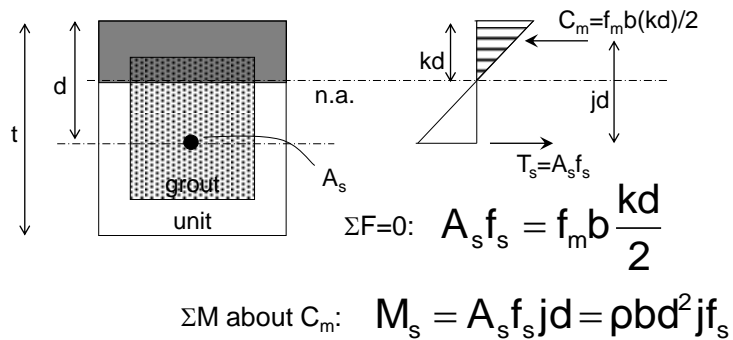
- tension
  - a) Grade 40 or 50  $F_s = 20 \text{ ksi}$
  - b) Grade 60  $F_s = 24 \text{ ksi}$
  - c) Wire joint  $F_s = 30 \text{ ksi}$
- \*no allowed increase by 1/3 for combinations with wind & earthquake
  - did before 2011 MSJC

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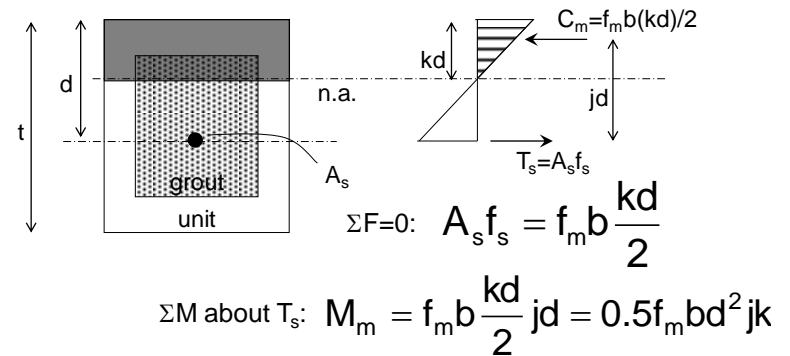
## Reinforcement, $M_s$



if  $f_s = F_s$  (allowable) the moment capacity is limited by the steel

MSJC:  $F_s = 20 \text{ ksi}, 24 \text{ ksi}$  or  $30 \text{ ksi}$  by type

## Reinforcement, $M_m$

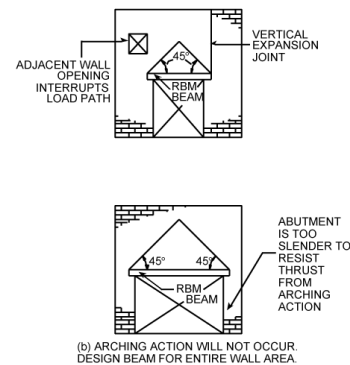
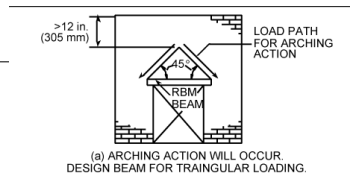
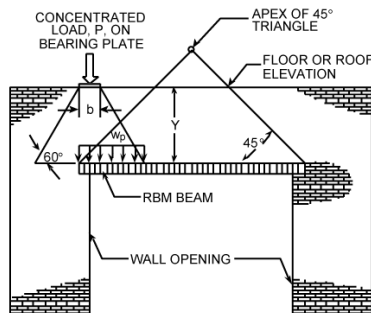


if  $f_s = F_s$  (allowable) the moment capacity is limited by the steel

MSJC  $F_b = 0.33 f'_m$

## Masonry Lintels

- distributed load
  - triangular or trapezoidal

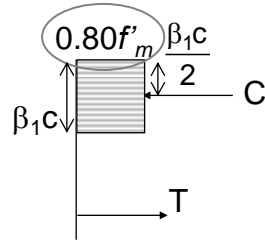


## Strategy for RM Flexural Design

- to size section and find reinforcement
  - find  $\rho_b$  knowing  $f'_m$  and  $f_y$
  - size section for some  $\rho < \rho_b$ 
    - get  $k, j$
    - $bd^2 = \frac{M}{\rho j F_s}$  } needs to be sized for shear also
    - get  $b$  &  $d$  in nice units
  - size reinforcement (bar size & #):  $A_s = \frac{M}{F_s jd}$
  - check design:  $M_s = A_s F_s jd > M$
  - $f_b = \frac{M}{0.5 b d^2 j k} < F_b$

## Ultimate Strength Design

- LRFD
- like reinforced concrete
- useful when beam shear is high
- improved inelastic model
  - ex. earthquake loads

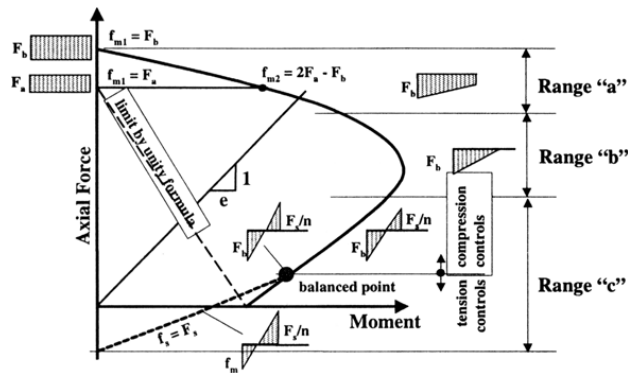


## Masonry Walls

- one-way or two-way bending
- usually use hollow units (< 75% solid)
- reinforcement grouted
  - into cells if hollow units
  - between wythes if solid
- reinforcement usually at center
- reinforcement in compression ineffective
- avoid stirrups
- desirable in seismic zones

## Masonry Walls

- axial force-moment interaction diagram
 
$$\frac{f_a}{F_a} + \frac{f_b}{F_b} \leq 1$$



## Masonry Shear Walls

- bearing, bending, and shear
  - compression increases resistance

$$f_v = \frac{VQ}{I_n b} \quad \text{or} \quad \frac{V}{A_{nv}} \leq F_v$$

unreinforced  reinforced

- unreinforced stress limit  $1.5\sqrt{f'_m} \leq 120 \text{ psi}$

## Masonry Shear Walls

- (and beams)
  - reinforcement strength included:

$$F_v = F_{vm} + F_{vs}$$

$$F_{vm} = \frac{1}{2} \left[ \left( 4.0 - 1.75 \left( \frac{M}{Vd} \right) \right) \sqrt{f'_m} \right] + 0.25 \frac{P}{A_n}$$

$$F_{vs} = 0.5 \left( \frac{A_v F_s d}{A_{nv} s} \right)$$

- stress limit depends on ratio of bending moment to overturning moment:  $M/Vd$

- spacing limits

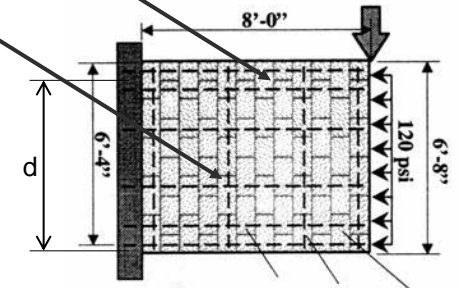
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## Masonry Shear Walls

- model as deep cantilever beam
  - flexure reinforcement
  - shear stirrups



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## Masonry Columns and Pilasters

- must be reinforced

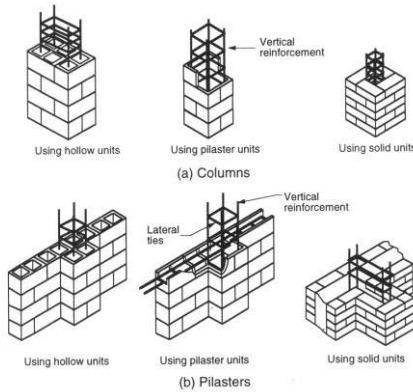


Figure 9.2 Columns and pilaster details.

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## Masonry Columns and Pilasters

- considered a column when  $b/t < 3$  and  $h/t > 4$
- slender is
  - 8" one side
  - $h/t \leq 25$
- needs ties
- eccentricity
  - 10% of side dimension required
  - interaction diagrams like  $r/c$



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## Masonry Columns

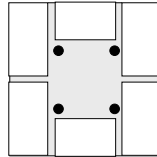
- allowable axial load

$$P_a = \left[ 0.25 f'_m A_n + 0.65 A_{st} F_s \right] \left[ 1 - \left( \frac{h}{140r} \right)^2 \right]$$

$h/r \leq 99$  (unreinforced  $A_{st} = 0$ )

$$P_a = \left[ 0.25 f'_m A_n + 0.65 A_{st} F_s \right] \left( \frac{70r}{h} \right)^2$$

$h/r > 99$



$h$  = effective length

$r$  = radius of gyration

$A_n$  = effective area of masonry

$A_{st}$  = effective area of column reinforcement

$F_s$  = allowable compressive stress in column reinforcement

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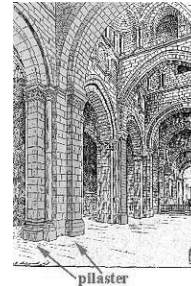
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## Masonry Pilasters, Arches

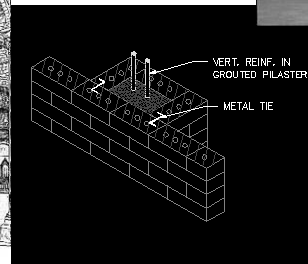
- column in wall
  - increase bearing area and stiffness



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## Construction Supervision

- proper placement of all reinforcement
- prism construction
  - masonry
  - mortar
- hot/cold weather protection



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