ELEMENTS OF ARCHITECTURAL STRUCTURES:

FORM, BEHAVIOR, AND DESIGN ARCH 614 DR. ANNE NICHOLS SPRING 2014

lecture SEVEN

shear & bending moment diagrams

V & M Diagrams 1 Lecture 7 Elements of Architectural Structures ARCH 614 S2009abn

Semigraphical Method

- by knowing
 - area under loading curve = change in V
 - area under shear curve = change in M
 - concentrated forces cause "jump" in V
 - concentrated moments cause "jump" in M

$$V_D - V_C = -\int_C^{X_D} w dx \qquad M_D - M_C = \int_C^{X_D} V dx$$

$$X_C$$

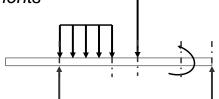
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Equilibrium Method

- important places
 - supports
 - concentrated loads
 - start and end of distributed loads

concentrated moments

- free ends
 - zero forces



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Semigraphical I

relationships

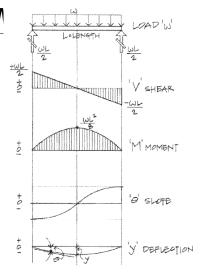
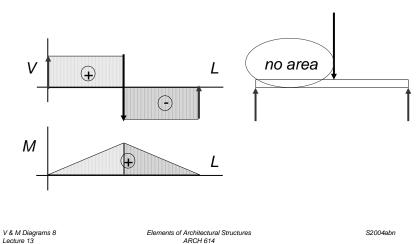


Figure 7.11 Relationship of load, shear, Elemer moment, slope, and deflection diagrams.

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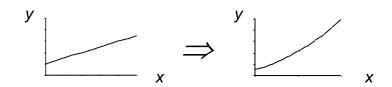
Semigraphical Method

• M_{max} occurs where V = 0 (calculus)



Curve Relationships

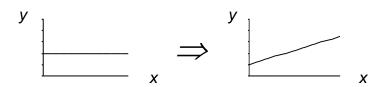
• line with slope, integrates to parabola



· ex: load to shear, shear to moment

Curve Relationships

- integration of functions
- line with 0 slope, integrates to sloped

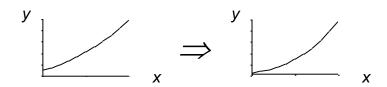


· ex: load to shear, shear to moment

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Curve Relationships

• parabola, integrates to 3rd order curve



• ex: load to shear, shear to moment

Basic Procedure

Find reaction forces & moments
 Plot axes, underneath beam load diagram

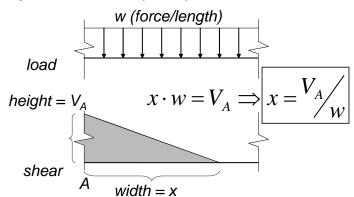
V.

- 2. Starting at left
- 3. Shear is 0 at free ends
- 4. Shear jumps with concentrated load
- 5. Shear changes with area under load

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Triangle Geometry

slope of V is w (-w:1)



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Basic Procedure

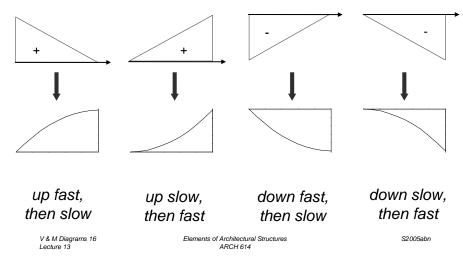
M:

- 6. Starting at left
- 7. Moment is 0 at free ends
- 8. Moment jumps with moment
- 9. Moment changes with area under V
- 10. Maximum moment is where shear = 0! (locate where V = 0)

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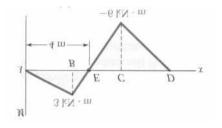
Parabolic Shapes

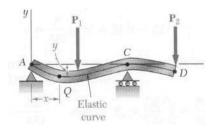
cases



Deflected Shape & M(x)

- -M(x) gives shape indication
- boundary conditions must be met





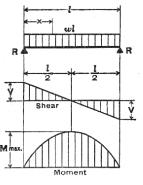
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S2004ahn

Tabulated Beam Formulas

- how to read charts
- 1. SIMPLE BEAM-UNIFORMLY DISTRIBUTED LOAD



Beam Deflection & Design 21

Total Equiv. Uniform Load . . . = wl

$$R = V$$
 $= \frac{wl}{2}$

$$V_X$$
 = $w\left(\frac{l}{2} - x^2\right)$

M max. (at center) =
$$\frac{wl^2}{8}$$

$$M_X$$
 $=\frac{wx}{2}(l-x)$

$$\triangle$$
max. (at center) . . . = $\frac{5 wl^4}{384 EI}$

$$\Delta_{X}$$
 = $\frac{w_{X}}{24EI}$ ($l^{3}-2l_{X}^{2}+x^{3}$)

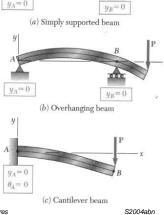
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Boundary Conditions

• at pins, rollers, fixed supports: y = 0



- at fixed supports: $\theta = 0$
- at inflection points from symmetry: $\theta = 0$
- y_{max} at $\frac{dy}{dx} = 0$



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