

lecture  
twenty seven

masonry construction:  
beams & columns



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Masonry Construction 1  
Lecture 27

Elements of Architectural Structures  
ARCH 614

S2009abn

Masonry Design

• Masonry Standards Joint Committee

- ACI, ASCE, TMS
- ASD (+empirical)
  - linear-elastic stresses
- LRFD added in 2002
- referenced by IBC
- unreinforced allows tension in flexure
- reinforced - all tension in steel
- walls are also in compression



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International Masonry Institute (Brian Trimble)

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Masonry Beam & Wall Design

- reinforcement increases capacity & ductility

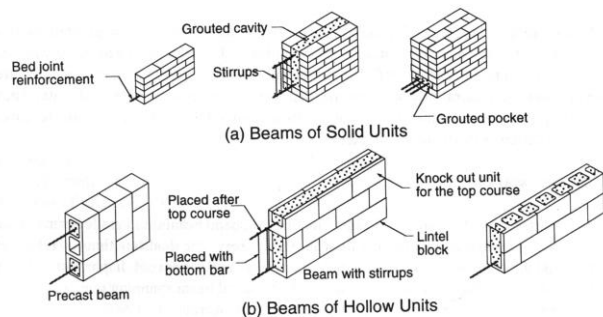


Figure 2.10 Reinforced masonry beams and lintels.

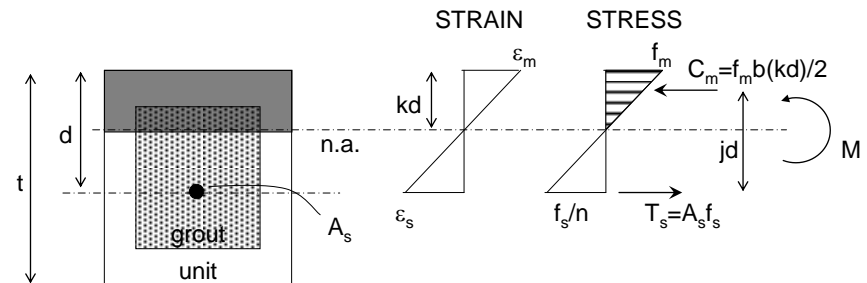
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Masonry Design

- $f_s$  is not the yield stress
- $f_m$  is the stress in the masonry



$$\rho = \frac{A_s}{bd}$$

BIA Teknote 17 series

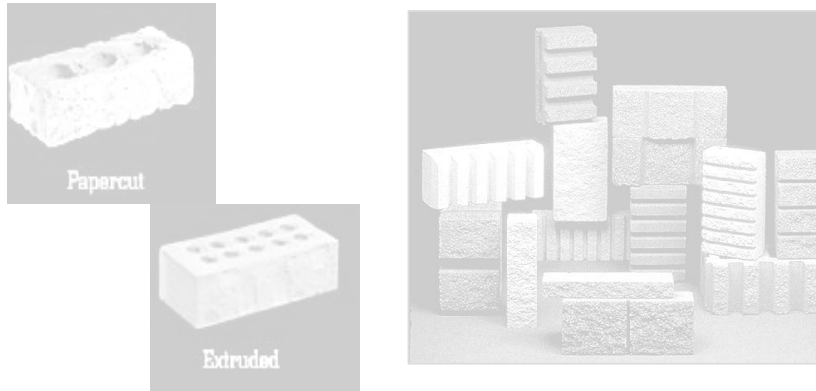
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# Masonry Materials

- *units*
  - stone, brick, concrete block, clay tile



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# Masonry Materials

- *rebar*
- *grout*
  - fills voids and fixes rebar
- *prisms*
  - used to test strength,  $f'_m$
- *fire resistant*



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# Masonry Materials

- *mortar*
  - water, masonry cement, sand, lime
  - types:
    - **M** higher strength – 2500 psi (ave.)
    - **S** medium high strength – 1800 psi
    - **N** medium strength – 750 psi
    - **O** medium low strength – 350 psi
    - **K** low strength – 75 psi



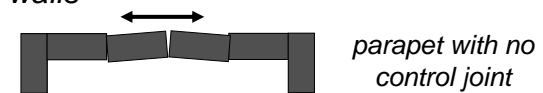
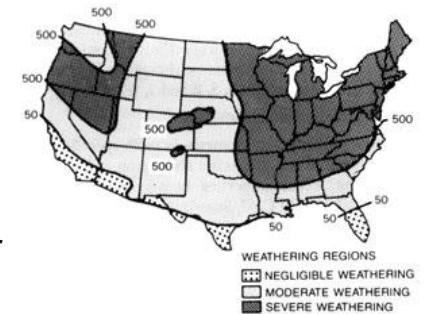
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# Masonry Materials

- *moisture resistance*
  - weathering index for brick
  - bond and detailing
  - expansion or shrinking from water
    - provide control joints
    - parapets, corners, long walls



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# Allowable Masonry Stresses

- tension - unreinforced only

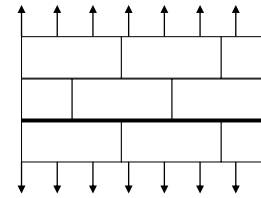
Table 2.2.3.2 — Allowable flexural tensile stresses for clay and concrete masonry, psi (kPa)

Direction of flexural tensile stress and masonry type	Mortar types			
	Portland cement/lime or mortar cement		Masonry cement or air entrained portland cement/lime	
	M or S	N	M or S	N
Normal to bed joints				
Solid units	53 (366)	40 (276)	32 (221)	20 (138)
Hollow units <sup>1</sup>				
UngROUTED	33 (228)	25 (172)	20 (138)	12 (83)
Fully grouted	86 (593)	84 (579)	81 (559)	77 (531)
Parallel to bed joints in running bond				
Solid units	106 (731)	80 (552)	64 (441)	40 (276)
Hollow units				
UngROUTED and partially grouted	66 (455)	50 (345)	40 (276)	25 (172)
Fully grouted	106 (731)	80 (552)	64 (441)	40 (276)
Parallel to bed joints in masonry not laid in running bond				
Continuous grout section parallel to bed joints	133 (917)	133 (917)	133 (917)	133 (917)
Other	0 (0)	0 (0)	0 (0)	0 (0)

<sup>1</sup> For partially grouted masonry, allowable stresses shall be determined on the basis of linear interpolation between fully grouted hollow units and ungrouted hollow units based on amount (percentage) of grouting.

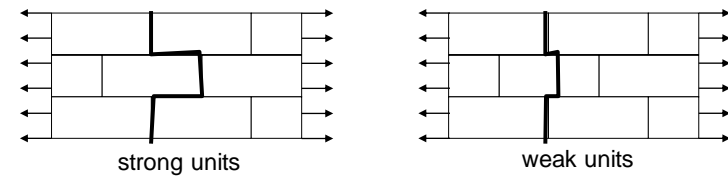
# Masonry Walls

tension normal to bed joints



Not allowed in MSJC codes

tension parallel to bed joints



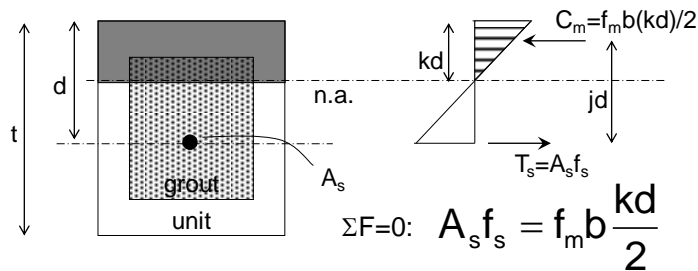
# Allowable Masonry Stresses

- flexure
  - $F_b = 1/3 f'_m$  (unreinforced)
  - $F_b = 0.45 f'_m$  (reinforced)
- shear, unreinforced masonry
  - $F_v = 1.5\sqrt{f'_m} < 120 \text{ psi}$
- shear, reinforced masonry
  - $M/Vd \leq 0.25: F_v = 3.0\sqrt{f'_m}$
  - $M/Vd \leq 0.25: F_v = 2.0\sqrt{f'_m}$

# Allowable Reinforcement Stress

- tension
  - Grade 40 or 50  $F_s = 20 \text{ ksi}$
  - Grade 60  $F_s = 32 \text{ ksi}$
  - Wire joint  $F_s = 30 \text{ ksi}$
- \*no allowed increase by 1/3 for combinations with wind & earthquake
  - did before 2011 MSJC code

## Reinforcement, $M_s$

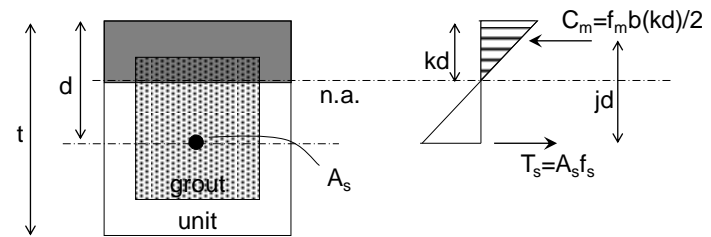


$$\Sigma M \text{ about } C_m: M_s = A_s f_s j d = \rho b d^2 j f_s$$

if  $f_s = F_s$  (allowable) the moment capacity is limited by the steel

MSJC:  $F_s = 20 \text{ ksi}, 32 \text{ ksi}$  or  $30 \text{ ksi}$  by type

## Reinforcement, $M_m$



for equilibrium:

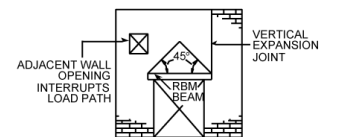
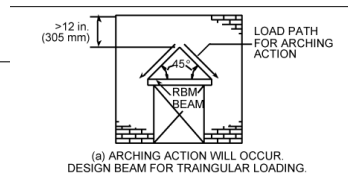
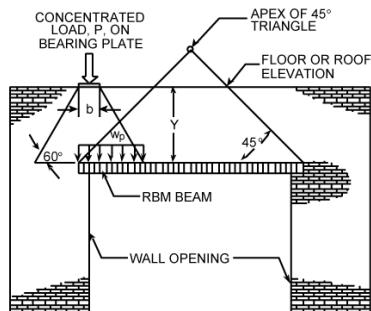
$$\Sigma M = 0 \text{ about } F_s \quad M_m = f_m b \frac{kd}{2} j d = 0.5 f_m b d^2 j k$$

if  $f_m = F_b$  (allowable) the moment capacity is limited by the masonry

$$\text{MSJC } F_b = 0.33 f'_m$$

## Masonry Lintels

- distributed load
  - triangular or trapezoidal

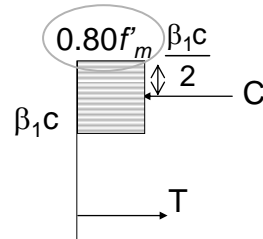


## Strategy for RM Flexural Design

- to size section and find reinforcement
    - find  $\rho_b$  knowing  $f'_m$  and  $f_y$
    - size section for some  $\rho < \rho_b$ 
      - get  $k, j$
      - $bd^2 = \frac{M}{\rho j F_s}$
      - get  $b$  &  $d$  in nice units
    - size reinforcement (bar size & #):  $A_s = \frac{M}{F_s j d}$
    - check design:  $M_s = A_s F_s j d > M$
    - $f_b = \frac{M}{0.5 b d^2 j k} < F_b$
- } needs to be sized for shear also

# Ultimate Strength Design

- LRFD
- like reinforced concrete
- useful when beam shear is high
- improved inelastic model
  - ex. earthquake loads



# Masonry Columns and Pilasters

- considered a column when
  - $b/t < 3$  and  $h/t > 4$ 
    - $b$  is width of “wall”
    - $t$  is thickness of “wall”
- slender is
  - 8” one side
  - $h/t \leq 25$
- needs ties
- eccentricity may be required



# Masonry Columns and Pilasters

- must be reinforced

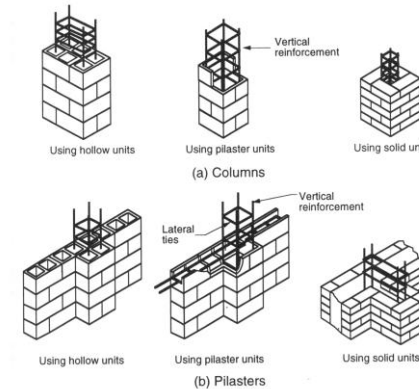


Figure 9.2 Columns and pilaster details.

# Masonry Columns

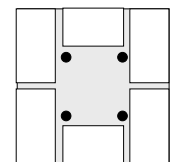
– allowable axial load

$$P_a = \left[ 0.25 f'_m A_n + 0.65 A_{st} F_s \right] \left[ 1 - \left( \frac{h}{140r} \right)^2 \right]$$

$h/r \leq 99$

$$P_a = \left[ 0.25 f'_m A_n + 0.65 A_{st} F_s \right] \left( \frac{70r}{h} \right)^2$$

$h/r > 99$



$h$  = effective length

$A_n$  = effective area of masonry

$A_{st}$  = effective area of column reinforcement

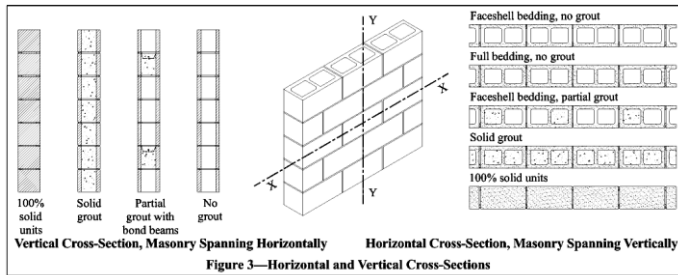
$F_s$  = allowable compressive stress in column reinforcement  
(lesser of  $0.4f_y$  or 24 ksi)

# Masonry Walls (unreinforced)

– allowable axial stresses

$$F_a = 0.25 f'_m \left[ 1 - \left( \frac{h}{140r} \right)^2 \right] \quad \text{for } h/r \leq 99$$

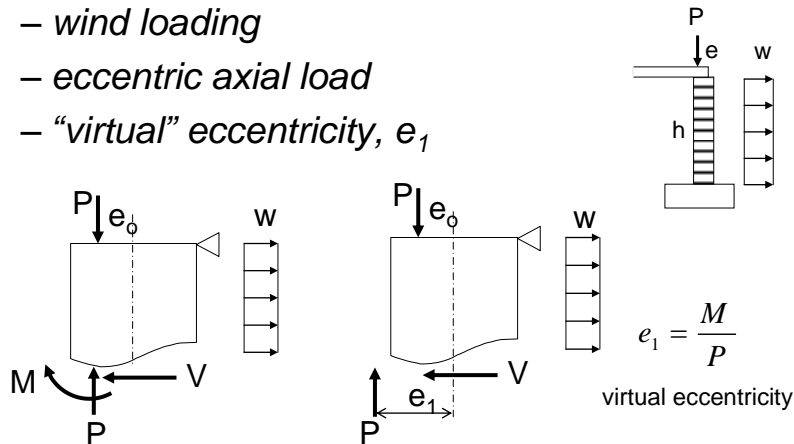
$$F_a = 0.25 f'_m \left( \frac{70r}{h} \right)^2 \quad \text{for } h/r > 99$$



# Design

• masonry columns and walls - loading

- wind loading
- eccentric axial load
- “virtual” eccentricity,  $e_1$



# Design

• masonry columns and walls (unreinforced)

$$\frac{f_a}{F_a} + \frac{f_b}{F_b} \leq 1.0 \quad \text{and} \quad f_b - f_a \leq F_t$$

$$\text{– } h/r < 99 \quad F_a = 0.25 f'_m \left[ 1 - \left( \frac{h}{140r} \right)^2 \right]$$

$$\text{– } h/r > 99 \quad F_a = 0.25 f'_m \left( \frac{70r}{h} \right)^2$$

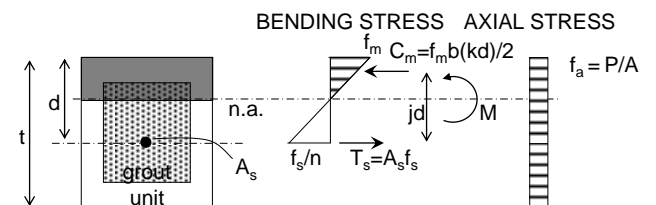
$$F_b = 0.33 f'_m$$

# Design

• masonry columns and walls – with rebar

- wall reinforcement usually at center and ineffective in compression

$$f_a + f_b \leq F_b \quad \text{provided} \quad f_a \leq F_a$$



$$\text{for equilibrium: } \sum F = P = C_m - T_s$$

## Design Steps Knowing Loads

### 1. assume limiting stress

- buckling, axial stress, combined stress

### 2. solve for $r$ , $A$ or $S$

### 3. pick trial section

### 4. analyze stresses

### 5. section ok?

### 6. stop when section is ok

