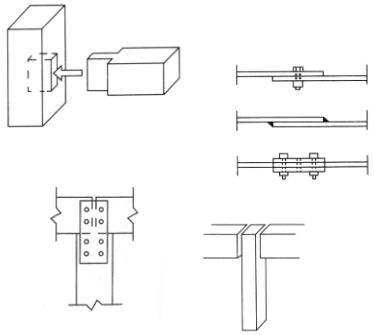
ARCHITECTURAL STRUCTURES:
FORM, BEHAVIOR, AND DESIGN
ARCH 331
DR. ANNE NICHOLS
SUMMER 2014

lecture
fourteen

wood construction: connections

Connectors

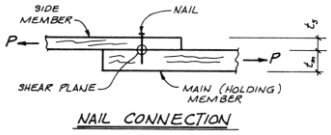
- joining
  - lapping
  - interlocking
  - butting
- mechanical
  - "third-elements"



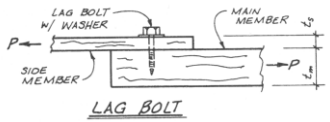
- transfer load at a point, line or surface
  - generally more than a point due to stresses

Wood Connectors

- adhesives
  - used in a controlled environment
  - can be used with nails
- mechanical
  - bolts
  - lag bolts or lag screws
  - nails
  - split ring and shear plate connectors
  - timber rivets



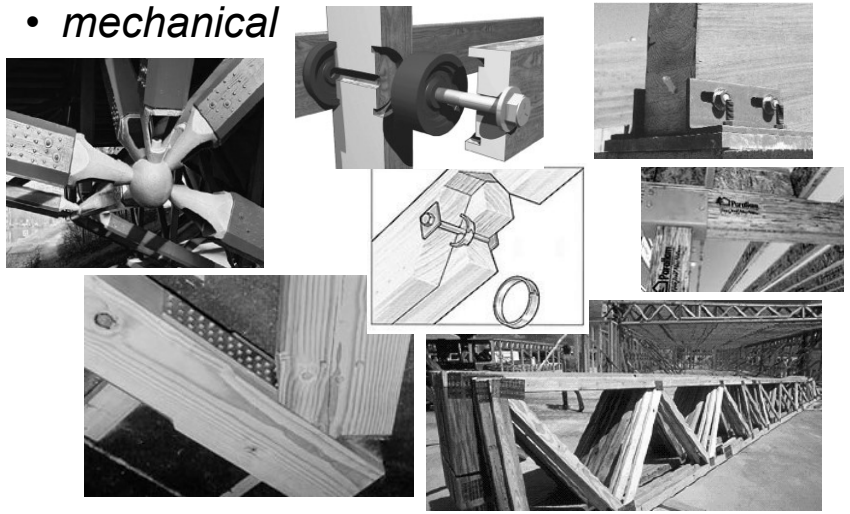
NAIL CONNECTION



LAG BOLT

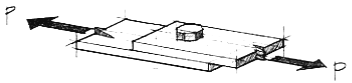
Wood Connections

- mechanical

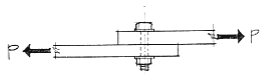


Bolted Joints

- *connected members in tension cause shear stress*

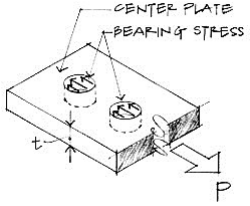


(a) Two steel plates bolted using one bolt.



(b) Elevation showing the bolt in

- *connected members in compression cause bearing stress*

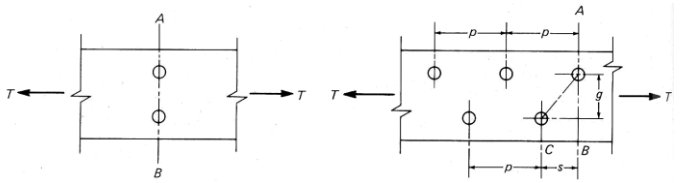


Bearing stress on plate.

Tension Members

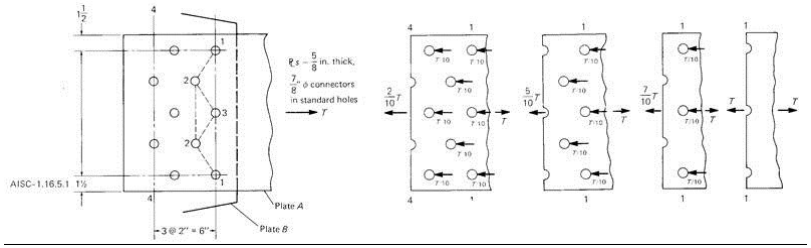
- *members with holes have reduced area*
- *increased tension stress*
- *A_e is effective net area*

$$f_t = \frac{P}{A_e}\left(or \frac{T}{A_e} \right)$$



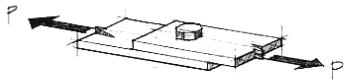
Effective Net Area

- *likely path to "rip" across*
- *bolts divide transferred force too*

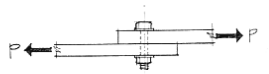


Single Shear

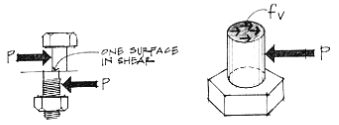
- *seen when 2 members are connected*



(a) Two steel plates bolted using one bolt.



(b) Elevation showing the bolt in shear.



(c) *(d)*

Figure 5.11 A bolted connection—single shear.

f_v = Average shear stress through bolt cross section

A = Bolt cross-sectional area

$f_v = \frac{P}{A}$

$$f_v = \frac{P}{A} = \frac{P}{\pi\, d^2/4}$$

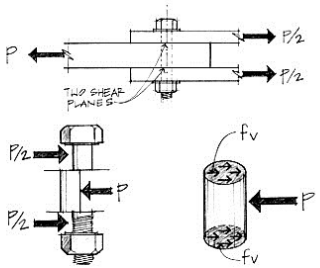
Double Shear

- *seen when 3 members are connected*

$$\Sigma F = 0 = -P + 2(P/2)$$

$f_v = \frac{P}{2A}$

(two shear planes)



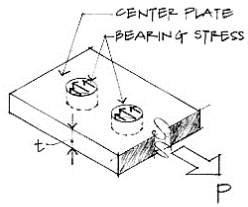
Free-body diagram of middle section of the bolt in shear.

Figure 5.12 A bolted connection in double shear.

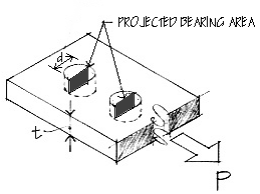
$$f_v = \frac{P}{2A} = \frac{P/2}{A} = \frac{P/2}{\pi d^2/4}$$

Bearing Stress

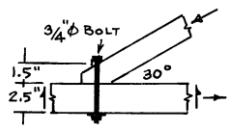
- *compression & contact*
- *stress limited by species & grain direction to load*
- *projected area*



Bearing stress on plate.

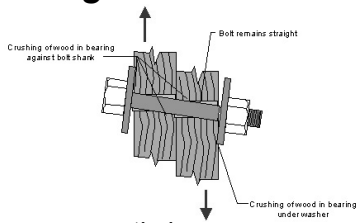


$$f_p = \frac{P}{A_{projected}} = \frac{P}{td}$$



Bolted Joints

- *twisting*



www.timber.org.au

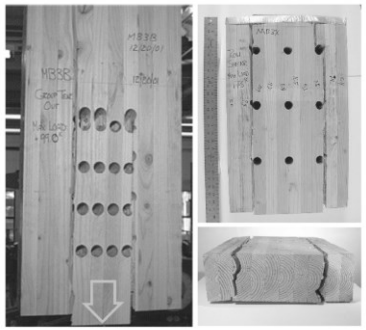


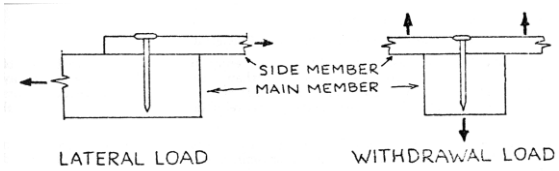
Figure 1.—Higher connection capacities can be achieved with increased fastener spacings.

Taylor & Line 2002

- *tear out*
  - *shear strength*
  - *end distance & spacing*

Nailed Joints

- *tension stress (pullout)*
- *shear stress nails presumed to share load by distance from centroid of nail pattern*



Nailed Joints

- *sized by pennyweight units / length*
- *embedment length*
- *dense wood, more capacity*

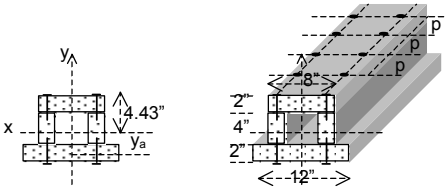
**NDS*

TABLE 7.1 Lateral Load Capacity of Common Wire Nails (lb/nail)

Side Member Thickness, t_s (in.)	Nail Length, L (in.)	Nail Diameter, D (in.)	Pennyweight	Load per Nail for Douglas Fir-Larch G = 0.50, Z (lb)
Structural Plywood Side Members				
⅜	2	0.113	6d	48
	2½	0.131	8d	63
	3	0.148	10d	76
½	2	0.113	6d	50
	2½	0.131	8d	65
	3	0.148	10d	78
	3½	0.162	16d	92

Connectors Resisting Beam Shear

- *plates with*
  - *nails*
  - *rivets*
  - *bolts*
- *splices*
- *V from beam load related to $V_{longitudinal}$*



$$\frac{V_{longitudinal}}{p} = \frac{VQ}{I}$$

$$nF_{connector} \geq \frac{VQ_{connected\ area}}{I} \cdot p$$

Vertical Connectors

- *isolate an area with vertical interfaces*

$$nF_{connector} \geq \frac{VQ_{connected\ area}}{I} \cdot p$$