

## design loads, methods, structural codes & tracing

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### Design Methods

- different approaches to meeting strength/safety requirements
  - allowable stress design (elastic)
  - ultimate strength design
  - limit state design
  - plastic design
  - load and resistance factor design
- assume a behavior at failure or other threshold and include a margin of safety

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### Design

- factors out of the designer's control
  - loads
  - occurrence
- factors within the designer's control
  - choice of material
  - “cost” of failure (F.S., probability, location)
  - economic design method
  - analysis method

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### Load Types

- $D$  = dead load
- $L$  = live load
- $L_r$  = live roof load
- $W$  = wind load
- $S$  = snow load
- $E$  = earthquake load
- $R$  = rainwater load or ice water load
- $T$  = effect of material & temperature
- $H$  = hydraulic loads from soil ( $F$  from fluids)

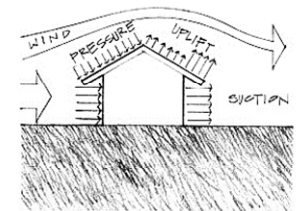


Figure 1.13 Wind loads on a structure.

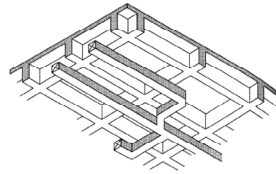
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# Dead Loads

- fixed elements
  - structure itself
  - internal partitions
  - hung ceilings
  - all internal and external finishes
  - HVAC ductwork and equipment
  - permanently mounted equipment
- $F = mg$  (GRAVITY)



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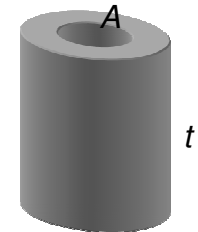
# Weight of Materials

- for a volume
  - $W = \gamma V$  where  $\gamma$  is weight/volume
  - $W = \gamma A$  for an extruded area with height of  $t$

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Table 5.1 Selected building material weights.

Assembly	lb/ft <sup>2</sup>	kN/m <sup>2</sup>	Assembly	lb/ft <sup>2</sup>	kN/m <sup>2</sup>
<b>Roofs:</b>			<b>Floors:</b>		
3-ply and gravel	5.5	0.26	Concrete plank	6.5	0.31
5-ply and gravel	6.5	0.31	Concrete slab	12.5/in.	0.59/mm
Wood shingles	2	0.10	Steel decking		
Asphalt shingles	2	0.10	w/concrete	35-45	1.68-2.16
Corrugated metal	1-2.5	0.05-0.12	Wood joists	2-3.5	0.10-0.17
Plywood	3/inch	0.0057/mm	Hardwood floors	4/in.	0.19/mm
Insulation			Ceramic tile		
–fiberglass batt	0.5	0.0025	w/thin set	15	0.71
Insulation—rigid	1.5	0.075	Lightweight concrete	8/in.	0.38/mm
			Timber decking	2.5/in.	0.08/mm

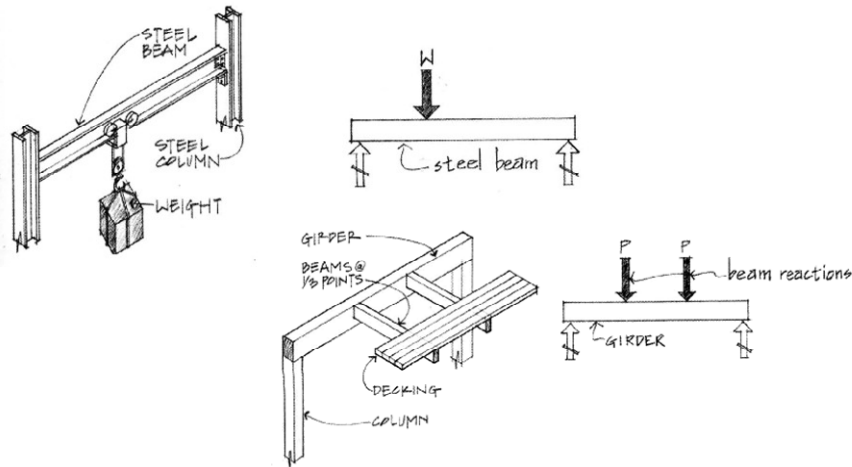


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# Concentrated Loads

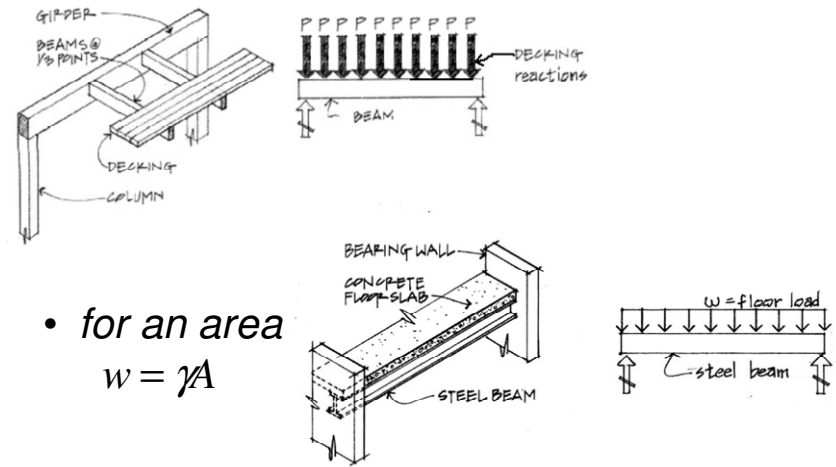


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# Distributed Loads



- for an area
  - $w = \gamma A$

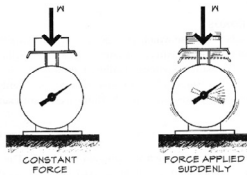
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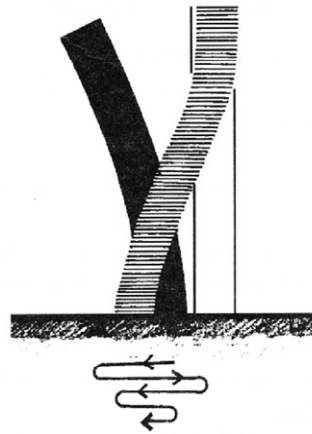
# Dynamic Loads

- time, velocity, acceleration
- kinetics
  - forces causing motion
  - $W = m \cdot g$
  - work
  - conservation of energy



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# Load Locations

- centric
- eccentric
- bending of flexural load
- torsional load
- combined loading

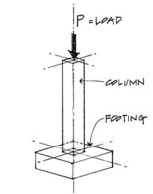
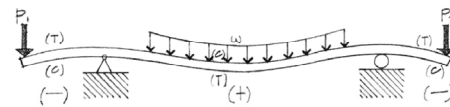
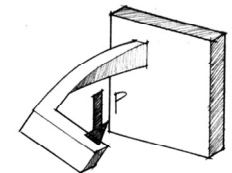


Figure 5.3 Centric loads.



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# Load Paths

- tributary areas
- transfer

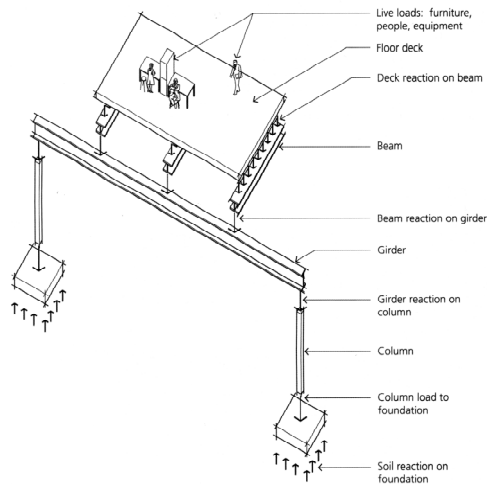


Fig. 1.3 Load paths through structures

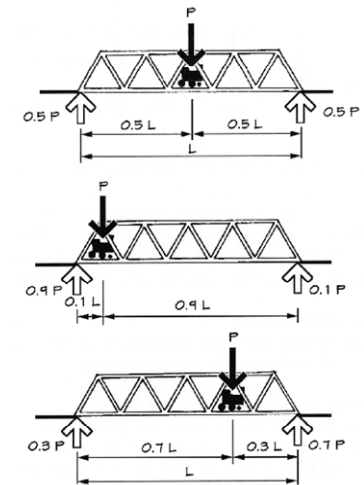
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# Live Loads

- occupancy
- movable furniture and equipment
- construction / roof traffic –  $L_r$
- minimum values
- reduction allowed as area increases



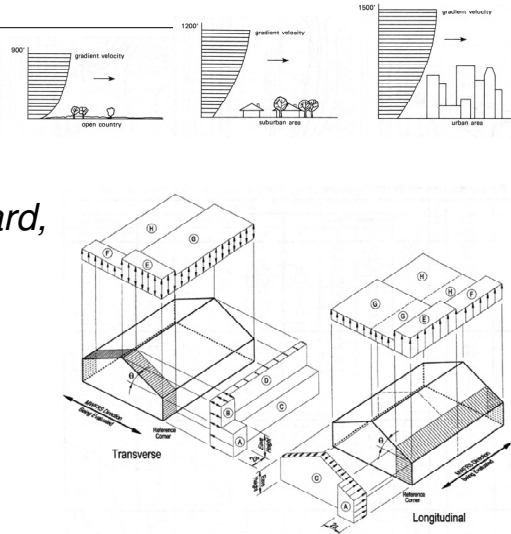
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## Wind Load

- wind speed
- gusting
- terrain
- windward, leeward, up and down!
- drag
- rocking
- harmonic
- torsion



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## Snow Load

- latitude
- solar exposure
- wind speed
- roof slope



Moscow 2006 (BBC News)

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## Seismic Load

- earthquake acceleration
  - $F = ma$
  - movement of ground (3D)
  - building mass responds
  - static models often used,  $V$  is static shear
  - building period,  $T \approx 0.1N$ , determines  $C$
  - building resistance –  $R_W$
  - $Z$  (zone),  $I$  (importance factor)

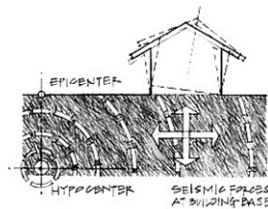


Figure 1.14 Earthquake loads on a structure.

$$V = \frac{ZICW}{R_W}$$

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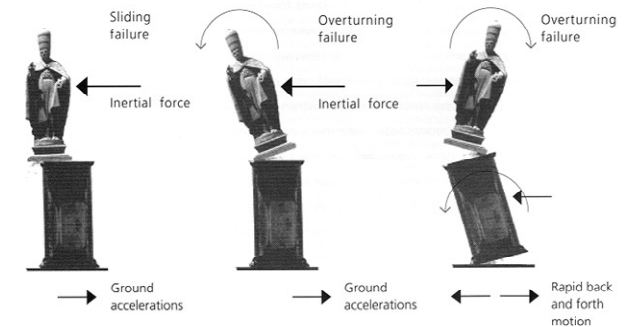
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## Dynamic Response



Statue in front of the cathedral of Palermo, Sicily



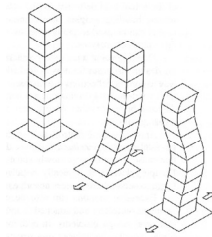
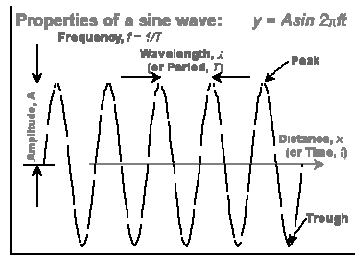
Lateral ground motions associated with earthquakes cause inertial forces to develop that are dependent on the weight of the structure. Sliding failures can occur.

The lateral ground motions can also cause a sculpture to overturn. The magnitude of the overturning effect depends on the weight of the sculpture and its height above the ground.

Back and forth ground motions can cause different parts of the sculpture to move in different directions. Overturning or cracking of elements can consequently occur.

## Dynamic Response

- *period of vibration or frequency*
  - wave
  - sway/time period
- *damping*
  - reduction in sway
- *resonance*
  - amplification of sway



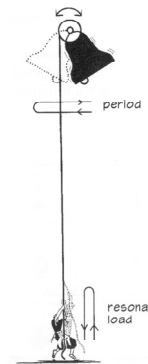
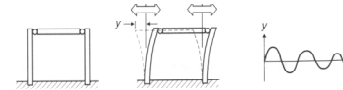
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## Frequency and Period

- *natural period of vibration*
  - avoid resonance
  - hard to predict seismic period
  - affected by soil
  - short period
    - high stiffness
  - long period
    - low stiffness



“To ring the bell, the sexton must pull on the downswing of the bell in time with the natural frequency of the bell.”

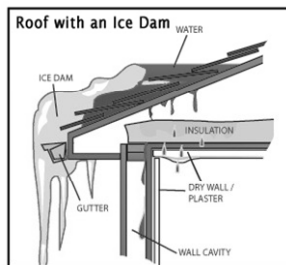
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## Water Load

- *rainwater – clogged drains*
- *ponding*
- *ice formation*



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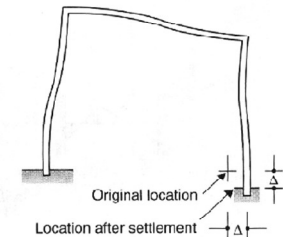
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## Thermal Load

- *stress due to strain*
- *restrained expansion or contraction*
- *temperature gradients*
- *composite construction*



(a) Single-bay frame.

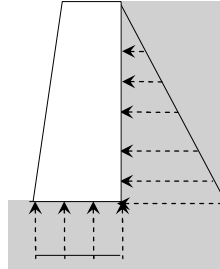
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## Hydraulic Loads

- pressure by water in soil,  $H$
- fluid pressure,  $F$ 
  - normal to surface
- flood



## Building Codes

- occupancy
- construction types
- structural chapters
  - loads, tests, foundations
- structural materials, assemblies
  - roofs
  - concrete
  - masonry
  - steel

OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (lbs.)
1. Apartments (see residential)	—	—
2. Access floor systems		
Office use	50	2,000
Computer use	100	2,000
3. Armories and drill rooms	150	—
4. Assembly areas and theaters		
Fixed seats (fastened to floor)	60	—
Lobbies	100	—
Movable seats	100	—
Stages and platforms	125	—
Fellow spots, projections and control rooms	50	—
Caiswalks	40	—

## Building Codes

- documentation
  - laws that deal with planning, design, construction, and use of buildings
  - regulate building construction for
    - fire, structural and health safety
  - cover all aspect of building design
  - references standards
    - acceptable minimum criteria
    - material & structural codes

## Prescribed Loads

- ASCE-7
  - live load (not roof) reductions allowed
- International Building Code
  - occupancy
  - wind: pressure to static load
  - seismic: shear load function of mass and response to acceleration
  - fire resistance

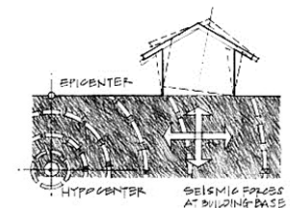


Figure 1.14 Earthquake loads on a structure.

## Structural Codes

- prescribe loads and combinations
- prescribe design method
- prescribe stress and deflection limits
- backed by the profession
- may require design to meet performance standards
- related to material or function

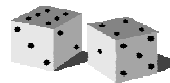
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## Design Methods

- probability of loads and resistance
- material variability
- overload, fracture, fatigue, failure
- allowable stress design



$$f_{actual} = \frac{P}{A} \leq f_{allowed} = \frac{f_{capacity}}{F.S.}$$

- limit state design
  - design loads & capacities

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## Structural Codes

- Design Codes
  - Wood
    - NDS
  - Steel
    - AISC
  - Concrete
    - ACI
    - AASHTO
  - Masonry
    - MSJC



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## Allowable Stress Design

- historical method
- a.k.a. working stress, strength design
- stresses stay in ELASTIC range

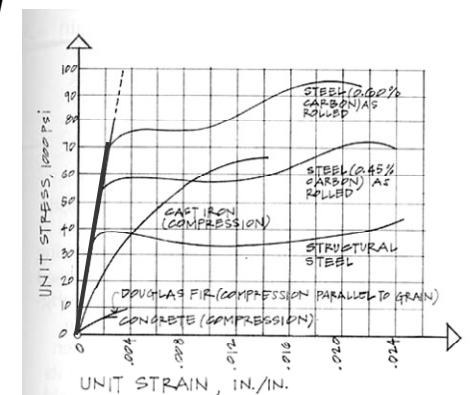


Figure 5.20 Stress-strain diagram for various materials.

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## ASD Load Combinations

ASCE-7  
(2010)

- $D$
- $D + L$
- $D + 0.75(L_r \text{ or } S \text{ or } R)$
- $D + 0.75L + 0.75(L_r \text{ or } S \text{ or } R)$
- $D + (0.6W \text{ or } 0.7E)$ 
  - $D + 0.75L + 0.75(0.6W) + 0.75(L_r \text{ or } S \text{ or } R)$
  - $D + 0.75L + 0.75(0.7E) + 0.75S$
- $0.6D + 0.6W$
- $0.6D + 0.7E$

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## Limit State Design

- *a.k.a. strength design*
- *stresses go to limit (strain outside elastic range)*
- *loads may be factored*
- *resistance or capacity reduced by a factor*
- *based on material behavior*
- *“state of the art”*

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## Limit State Design

- *load and resistance factor design (LRFD)*
  - *loads:*
    - *not constant,*
    - *possibly more influential on failure*
    - *happen more or less often*
  - **UNCERTAINTY**

$$\gamma_D R_D + \gamma_L R_L \leq \phi R_n$$

$\phi$  - Resistance factor

$\gamma$  - Load factor for (D)ead & (L)ive load

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## LRFD Load Combinations

ASCE-7  
(2010)

- $1.4D$
- $1.2D + 1.6L + 0.5(L_r \text{ or } S \text{ or } R)$
- $1.2D + 1.6(L_r \text{ or } S \text{ or } R) + (L \text{ or } 0.5W)$
- $1.2D + 1.0W + L + 0.5(L_r \text{ or } S \text{ or } R)$
- $1.2D + 1.0E + L + 0.2S$
- $0.9D + 1.0W$
- $0.9D + 1.0E$ 
  - *F has same factor as D in 1-5 and 7*
  - *H adds with 1.6 and resists with 0.9 (permanent)*

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## Deflection Limits

- based on service condition, severity

Use	LL only	DL+LL
Roof beams:		
Industrial	L/180	L/120
Commercial		
plaster ceiling	L/240	L/180
no plaster	L/360	L/240
Floor beams:		
Ordinary Usage	L/360	L/240
Roof or floor (damageable elements)		L/480

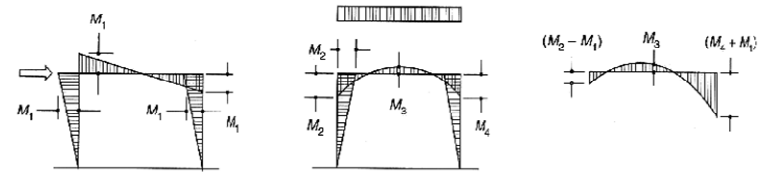
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## Load Conditions

- loads, patterns & combinations
  - usually uniformly distributed gravity loads
  - worst case for largest moments...
  - wind direction can increase moments



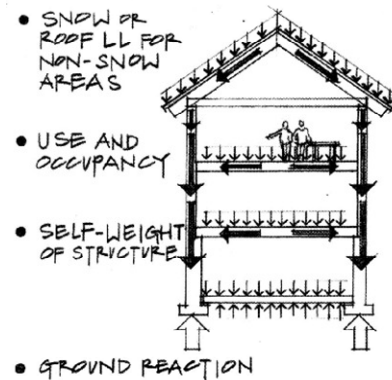
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## Structural Loads

- gravity acts on mass ( $F=m \cdot g$ )
- force of mass
  - acts at a point
    - ie. joist on beam
  - acts along a "line"
    - ie. floor on a beam
  - acts over an area
    - ie. people, books, snow on roof or floor



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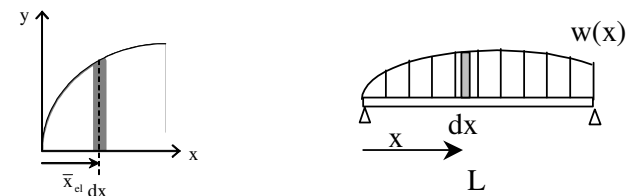
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## Equivalent Force Systems

- replace forces by resultant
- place resultant where  $M = 0$
- using calculus and area centroids

$$W = \int_0^L w dx = \int dA_{\text{loading}} = A_{\text{loading}}$$



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## Area Centroids

- Table 7.1 – pg. 242

Centroids of Common Shapes of Areas and Lines

Shape		$\bar{x}$	$\bar{y}$
Triangular area		$\frac{b}{3}$	$\frac{h}{3}$
Quarter-circular area		$\frac{4r}{3\pi}$	$\frac{4r}{3\pi}$
Semicircular area		0	$\frac{4r}{3\pi}$
Semiparabolic area		$\frac{3a}{8}$	$\frac{3h}{5}$
Parabolic area		0	$\frac{3h}{5}$

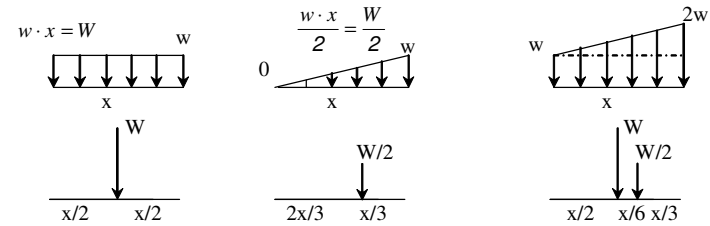
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## Equivalent Load Areas

- area is width x “height” of load
- $w$  is load per unit length
- $W$  is total load



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## Distributed Area Loads

- $w$  is also load per unit area

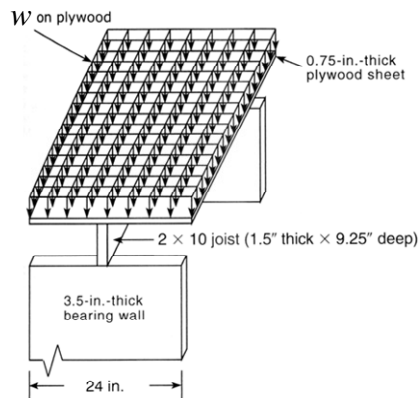


Figure 2.7 Area-distributed load (pressure) on floor decking.

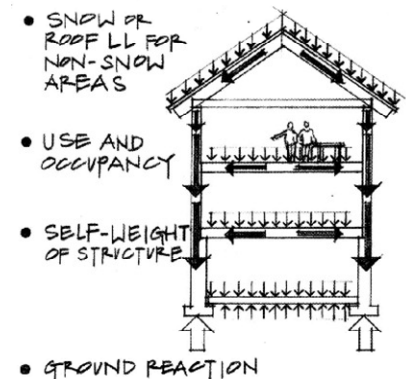
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## Load Tracing

- how loads are transferred
  - usually starts at top
  - distributed by supports as actions
  - distributed by tributary areas



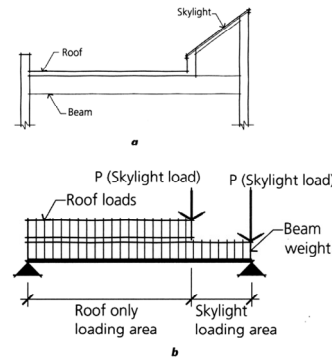
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## Load Tracing

- areas see distributed area load
- beams or trusses see distributed line loads
- “collectors” see forces
  - columns
  - supports



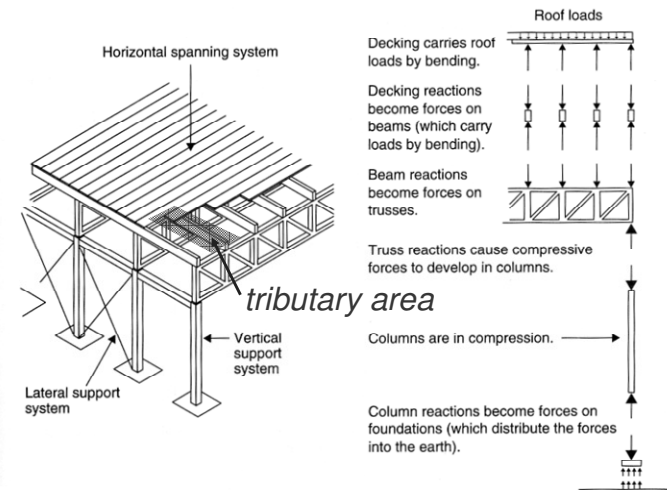
Figs. 1.1a, 1.1b Structural loading diagram of an architectural condition

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## Load Tracing



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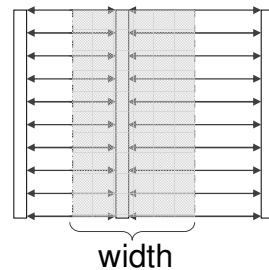
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## Load Tracing

- tributary load
  - think of water flow
  - “concentrates” load of area into center

$$w = \left( \frac{\text{load}}{\text{area}} \right) \times (\text{tributary width})$$



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## Load Tracing



www.columbia.edu

Patcenter  
Rogers 1986

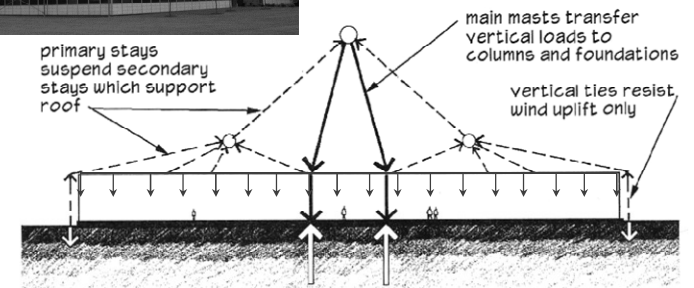


Figure 3.5: Patcenter, load path diagram.

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# Load Tracing



<http://en.structurae.de>

**Alamillo Bridge  
Calatrava 1992**

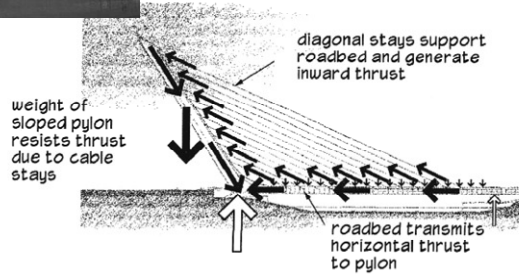


Figure 3.12: Alamillo bridge, load path diagram.

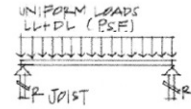
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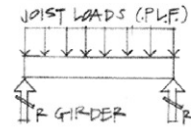
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# Load Paths

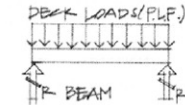
- floors and framing



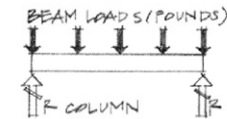
(a) FBD—decking.



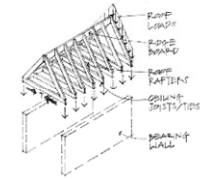
(c) FBD—beams.



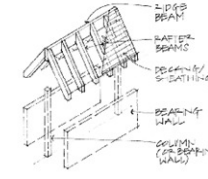
(b) FBD—joists.



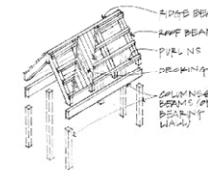
(d) FBD—girder.



(a)



(c)



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# Load Paths

- wall systems

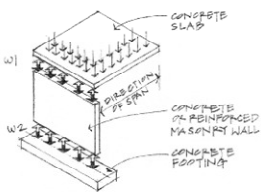


Figure 4.12 Uniform wall load from a slab.

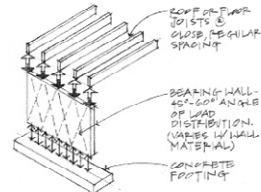


Figure 4.13 Uniform wall load from rafters and joists.

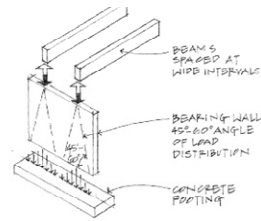


Figure 4.14 Concentrated loads from widely spaced beams.

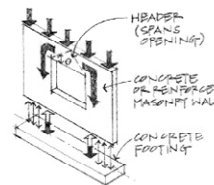


Figure 4.15 Arching over wall openings.

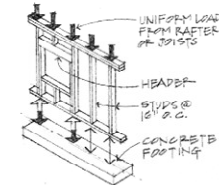


Figure 4.16 Stud wall with a window opening.

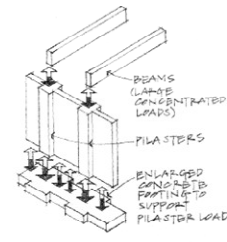


Figure 4.17 Pilasters supporting concentrated beam loads.

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# Load Paths

- foundations

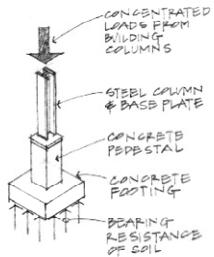


Figure 4.24 Spread footing.

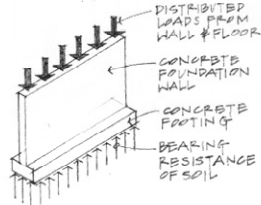


Figure 4.25 Wall footing.

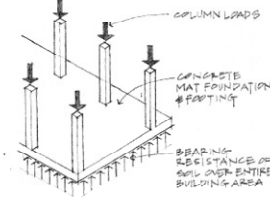


Figure 4.26 Mat or raft foundation.

# Load Paths

- deep foundations

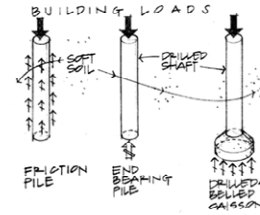


Figure 4.27 Pile foundations.

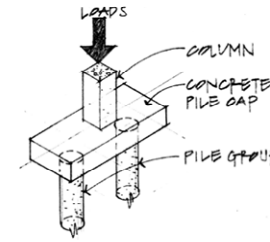


Figure 4.28 Pile cap on one pile group.

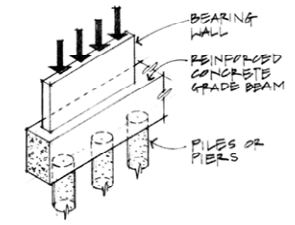


Figure 4.29 Grade beam supporting a bearing wall.

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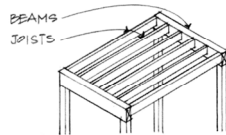
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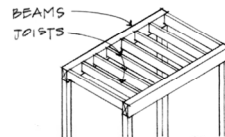
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# Spans

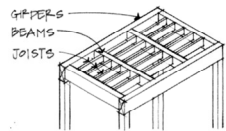
- direction
- depth



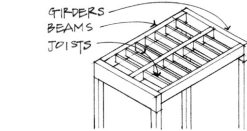
(a) Long, lightly loaded joists bearing on shorter beams create a more uniform structural depth. Space can be conserved if the joists and beams are flush framed.



(b) Short joists loading relatively long beams yield shallow joists and deep beams. The individual structural bays are more clearly expressed.



(c) Loads can be reduced on selected beams by introducing intermediate beams.



(d) The span capability of the decking material controls the spacing of the joists, while beam spacing is controlled by the allowable joist span.

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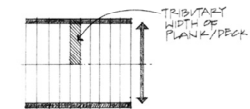
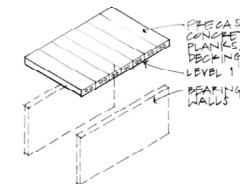
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# Levels

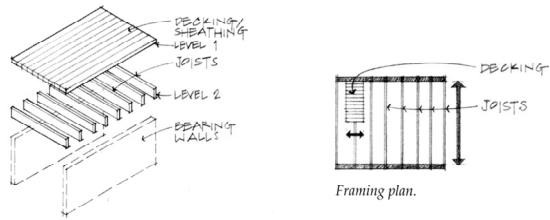
- determine span at top level
- find half way to next element
- \*include self weight
- look for “collectors”
- repeat
- one:



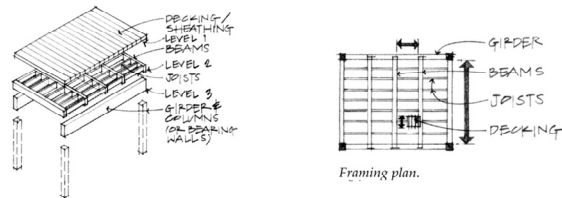
Framing plan.

## Levels

- two:



- three:



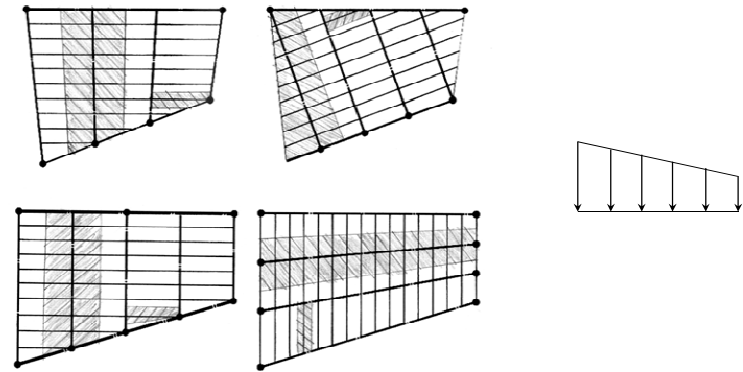
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## Irregular Configurations

- tracing still 1/2 each side



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## Slabs

- edge support

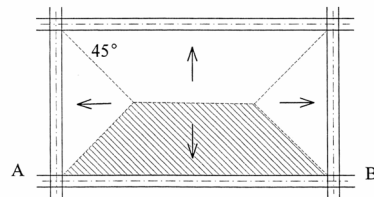


Figure 2-16: Supporting beams' contributing areas for reinforced concrete floor system.

- linear and uniform distribution

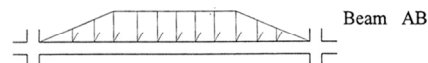


Figure 2-17: Trapezoidal distributed load for Beam AB of Fig. 2-16.

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## Girders and Transfer

- openings
  - no load & no half way
- girder actions at beam supports

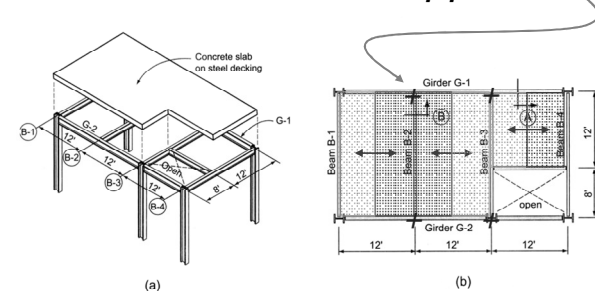


Figure 5.54 (a) Isometric view of partial steel framing arrangement. (b) Partial floor framing—office structure.

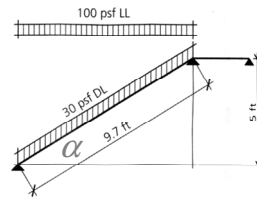
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## Sloped Beams

- stairs & roofs
- projected live load
- dead load over length



- perpendicular load to beam:

$$w_{\perp} = w \cdot \cos \alpha$$

- equivalent distributed load:

$$w_{adj.} = \frac{w}{\cos \alpha}$$

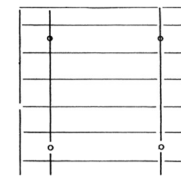
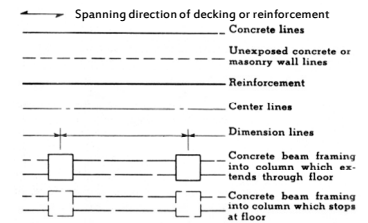
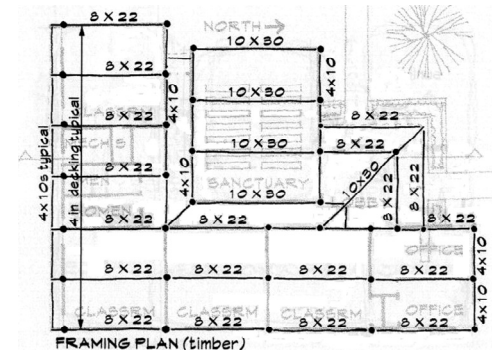
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## Framing Diagrams

- beam lines and “dots”
- breaks & ends



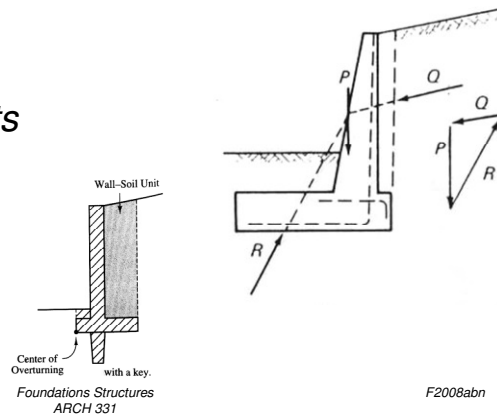
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## Retaining Walls

- purpose
  - retain soil or other material
- basic parts
  - wall & base
  - additional parts
    - counterfort
    - buttress
    - key



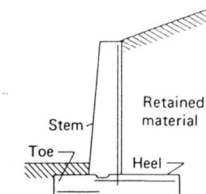
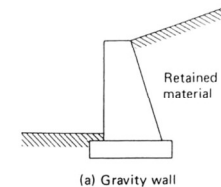
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## Retaining Wall Types

- “gravity” wall
  - usually unreinforced
  - economical & simple
- cantilever retaining wall
  - common



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## Retaining Wall Loads

- gravity
 
$$W = \gamma \times V$$
- fluid pressure
 
$$p = \omega' \times h$$

$$P = \frac{1}{2} p h \text{ at } h/3$$
- friction
 
$$F = \mu \times N$$
- soil bearing pressure,  $q$

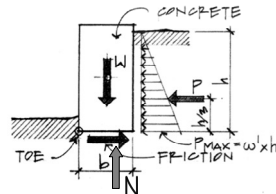


Figure 3.80 FBD of a gravity retaining wall.

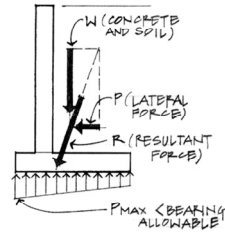


Figure 3.81 Bearing pressure under the wall footing.

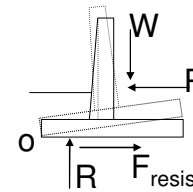
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## Retaining Wall Equilibrium

- sliding - overcome friction?
- overturning at toe (o) - overcome mass?



$$SF = \frac{M_{resist}}{M_{overturning}} \geq 1.5 - 2$$

$$SF = \frac{F_{horizontal-resist}}{F_{sliding}} \geq 1.25 - 2$$

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## Pressure Distribution

- want resultant of load from pressure inside the middle third of base (kern)
- triangular stress block with  $p_{max}$
- $x = 1/3$  x width of stress
- equivalent force location:

$$W \cdot x = \frac{p_{max} \cdot 3x \cdot x}{2 \cdot 3}$$

$$p_{max} = \frac{2W}{3x} = \frac{2W}{a} \text{ when } a \text{ is fully stressed}$$

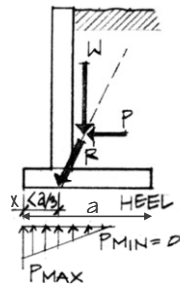


Figure 3.88 Tension possible at the heel.

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## Wind Pressure

- distributed load
- “collected” into V
- lateral loads must be resisted

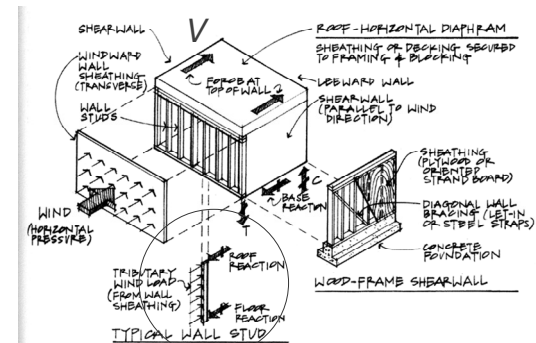


Figure 4.48 Exploded view of a light-framed wood building showing the various lateral resisting components.

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# Bracing Configurations

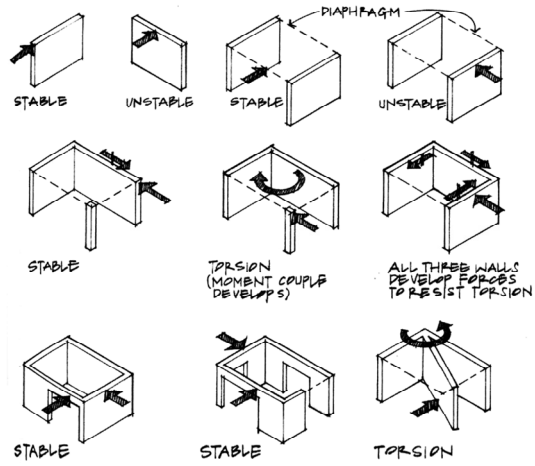


Figure 4.54 Various shearwall arrangements—some stable, others unstable.