#### ARCHITECTURAL STRUCTURES:

FORM, BEHAVIOR, AND DESIGN

ARCH 331

DR. ANNE NICHOLS

SUMMER 2014

Lecture SIX



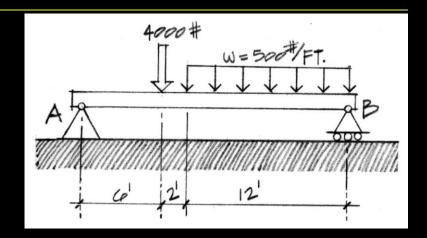
http://nisee.berkeley.edu/godden

# beams -

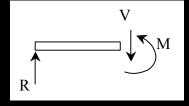
# internal forces & diagrams

#### Beams

- span horizontally
  - floors
  - bridges
  - roofs



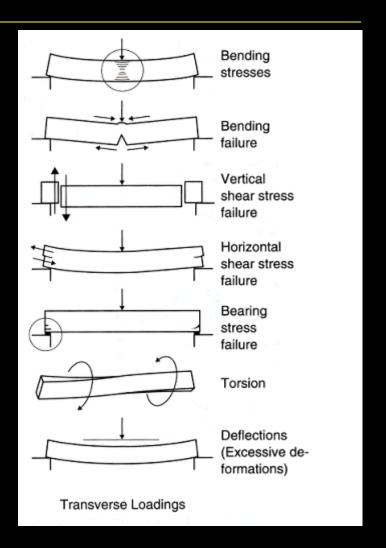
- loaded transversely by gravity loads
- may have internal axial force
- will have internal shear force



will have internal moment (bending)

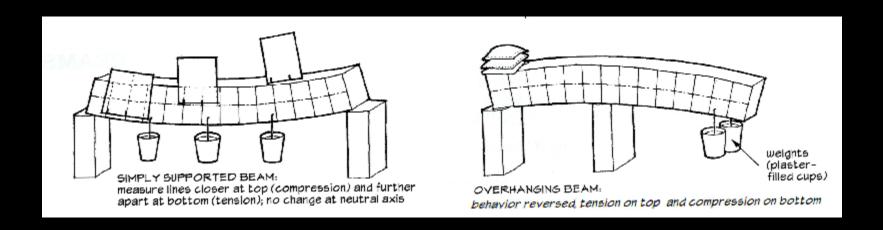
#### Beams

- transverse loading
- sees:
  - bending
  - shear
  - deflection
  - torsion
  - bearing
- behavior depends on cross section shape

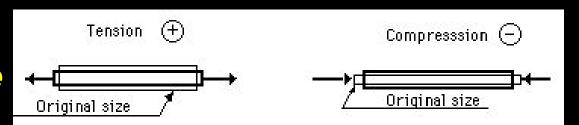


#### Beams

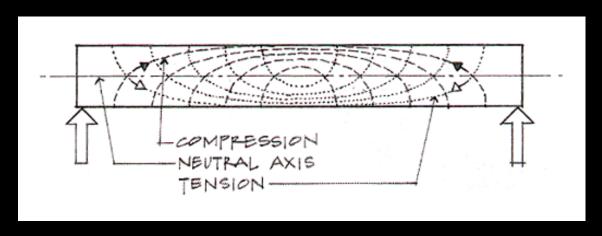
- bending
  - bowing of beam with loads
  - one edge surface stretches
  - other edge surface squishes

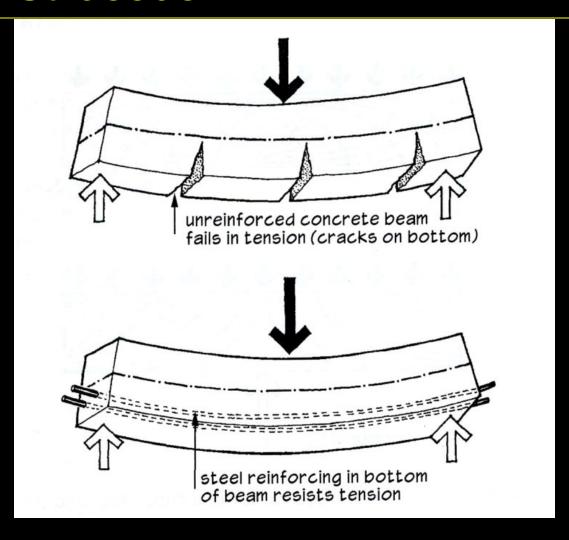


- stress = relative force over an area
  - tensile
  - compressive
  - bending

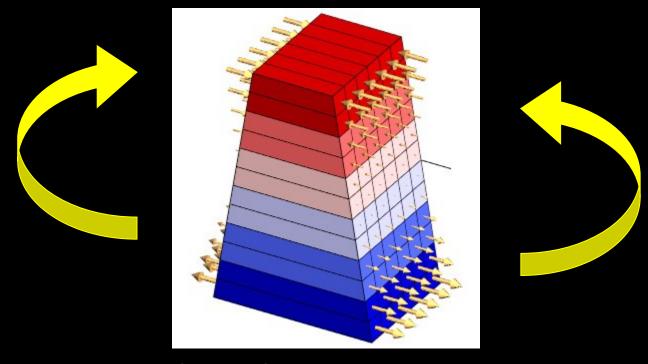


• tension and compression + ...



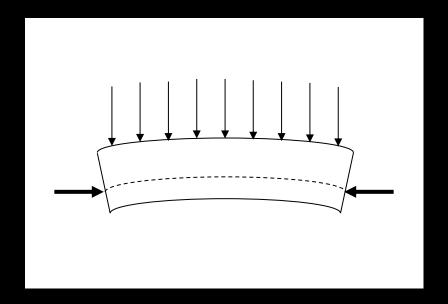


- tension and compression
  - causes moments

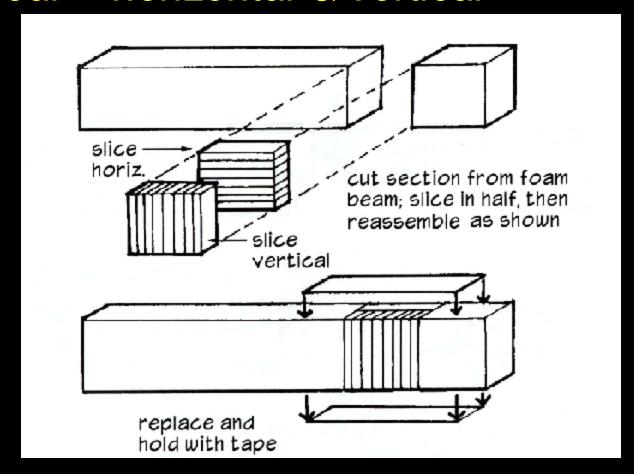


Copyright © 1996-2000 Kirk Martini.

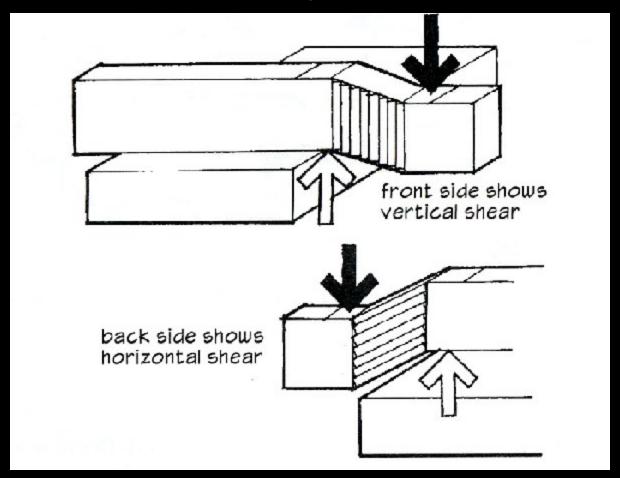
- prestress or post-tensioning
  - put stresses in tension area to "pre-compress"



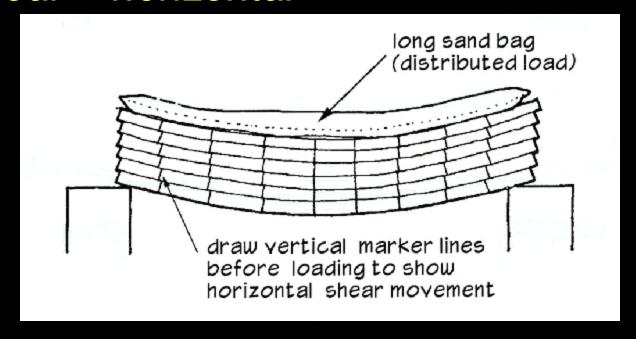
shear – horizontal & vertical



shear – horizontal & vertical



shear – horizontal



### Beam Deflections

- depends on
  - load
  - section
  - material

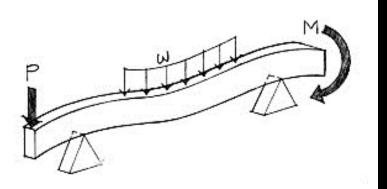
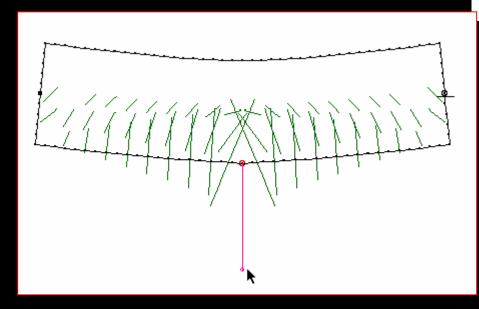


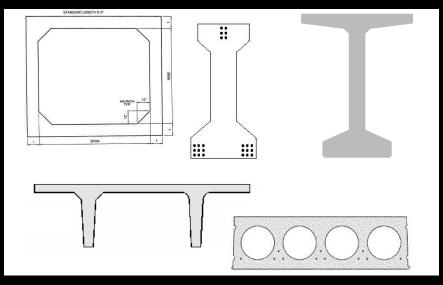
Figure 5.4 Bending (flexural) loads on a beam.



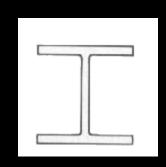
### Beam Deflections

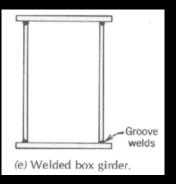
• "moment of inertia"











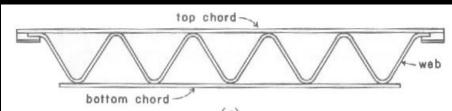
## Beam Styles

vierendeel



http://nisee.berkeley.edu/godden

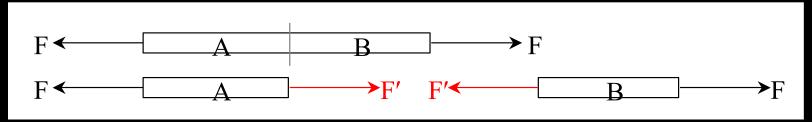
- open web joists
- manufactured



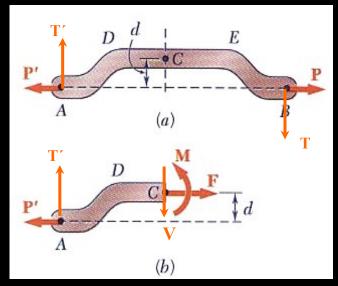


#### Internal Forces

- trusses
  - axial only, (compression & tension)

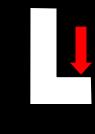


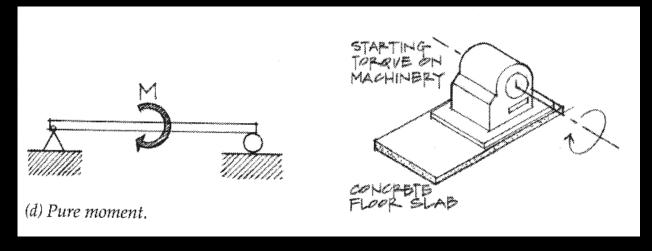
- in general
  - axial force
  - shear force, V
  - bending moment, M



### Beam Loading

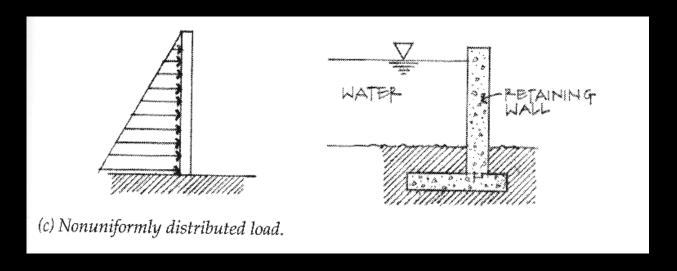
- concentrated force
- concentrated <u>moment</u>
  - spandrel beams





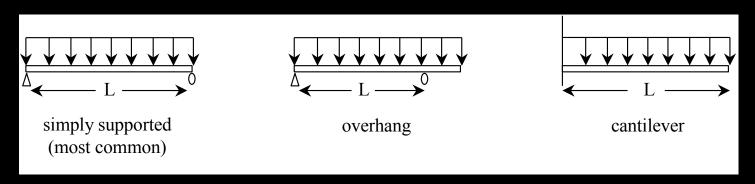
### Beam Loading

- uniformly distributed load (line load)
- non-uniformly distributed load
  - hydrostatic pressure =  $\gamma h$
  - wind loads

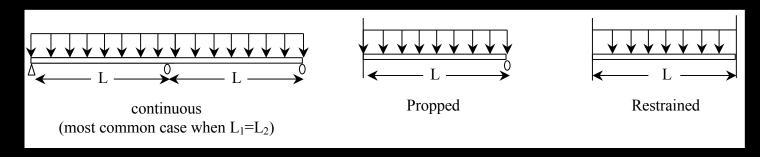


### Beam Supports

statically determinate

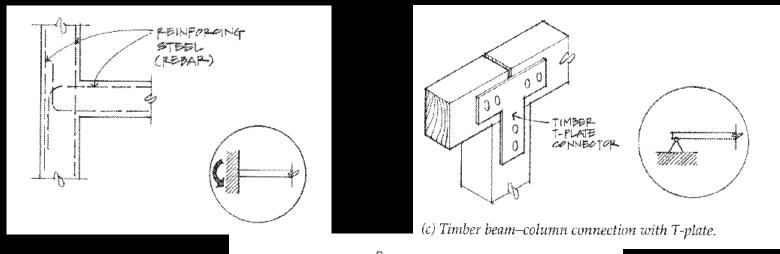


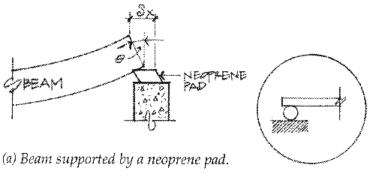
statically indeterminate



### Beam Supports

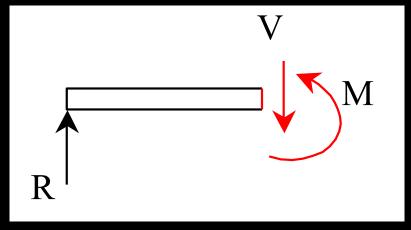
• in the real world, modeled type





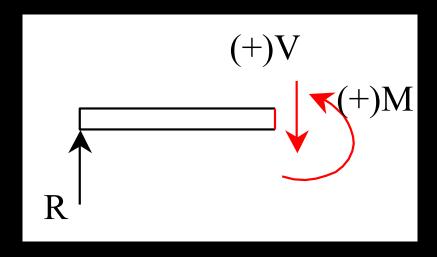
#### Internal Forces in Beams

- like method of sections / joints
  - no axial forces
- section <u>must</u> be in equilibrium
- want to know where biggest internal forces and moments are for designing



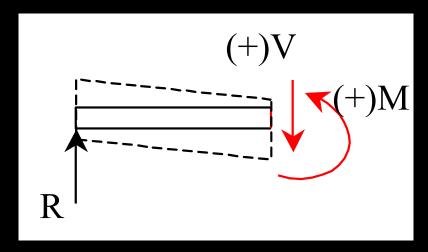
### V & M Diagrams

- tool to locate  $V_{max}$  and  $M_{max}$  (at V = 0)
- necessary for designing
- have a <u>different sign convention</u> than external forces, moments, and reactions

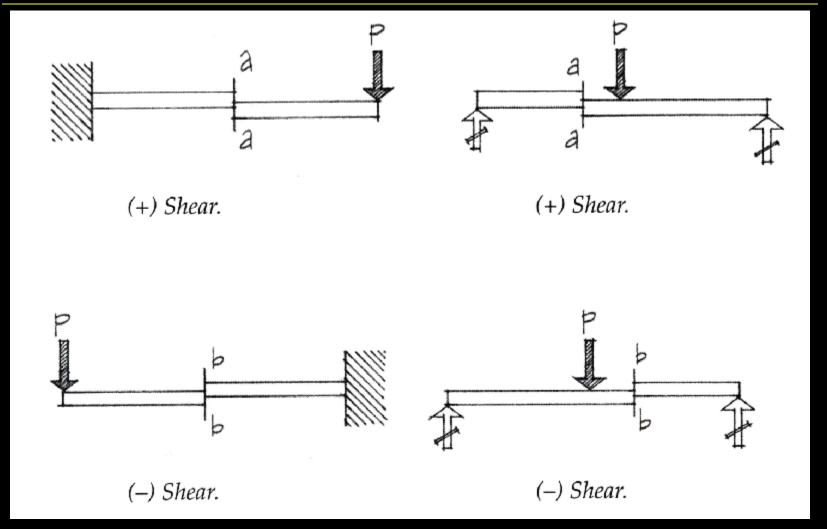


### Sign Convention

- shear force, V:
  - cut section to LEFT
  - if ∑F<sub>y</sub> is positive by statics, V acts down and is POSITIVE
  - beam has to resist shearing apart by V

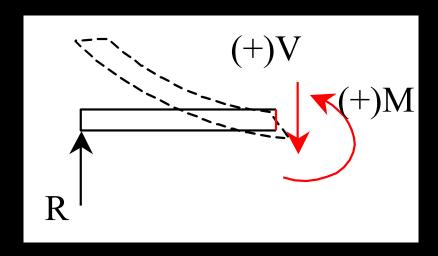


# Shear Sign Convention

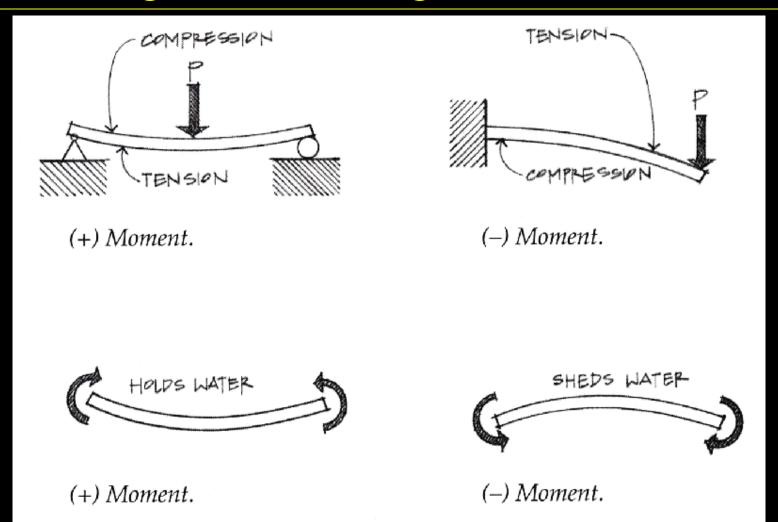


### Sign Convention

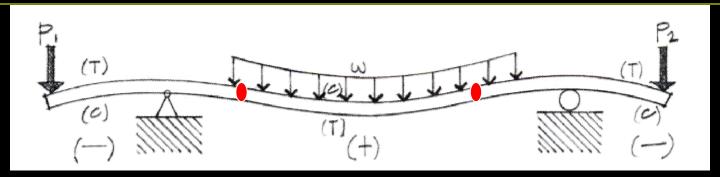
- bending moment, M:
  - cut section to LEFT
  - if ∑M<sub>cut</sub> is clockwise, M acts ccw and is POSITIVE – flexes into a "smiley" beam has to resist bending apart by M



# Bending Moment Sign Convention



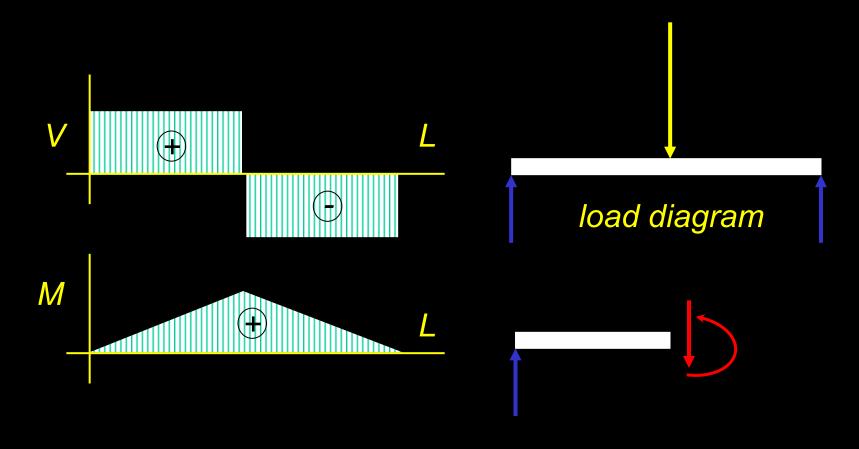
### Deflected Shape



- positive bending moment
  - tension in bottom, compression in top
- negative bending moment
  - tension in top, compression in bottom
- zero bending moment
  - inflection point

# Constructing V & M Diagrams

along the beam length, plot V, plot M



### Mathematical Method

cut sections with x as width

write functions of V(x) and M(x)

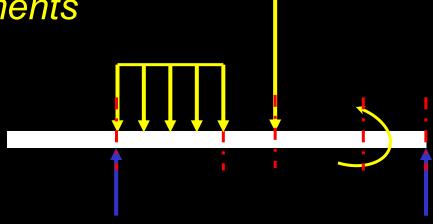
### Method 1: Equilibrium

cut sections at important places

 plot V & M M

### Method 1: Equilibrium

- important places
  - supports
  - concentrated loads
  - start and end of distributed loads
  - concentrated moments
- free ends
  - zero forces



### Method 2: Semigraphical

#### by knowing

- area under loading curve = change in V
- area under shear curve = change in M
- concentrated forces cause "jump" in V
- concentrated moments cause "jump" in M

$$V_{D} - V_{C} = -\int_{C}^{X_{D}} w dx \qquad M_{D} - M_{C} = \int_{C}^{X_{D}} V dx$$

$$x_{C}$$

### Method 2

### relationships

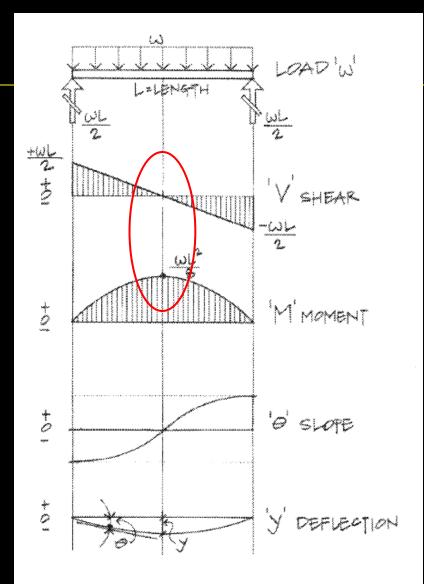
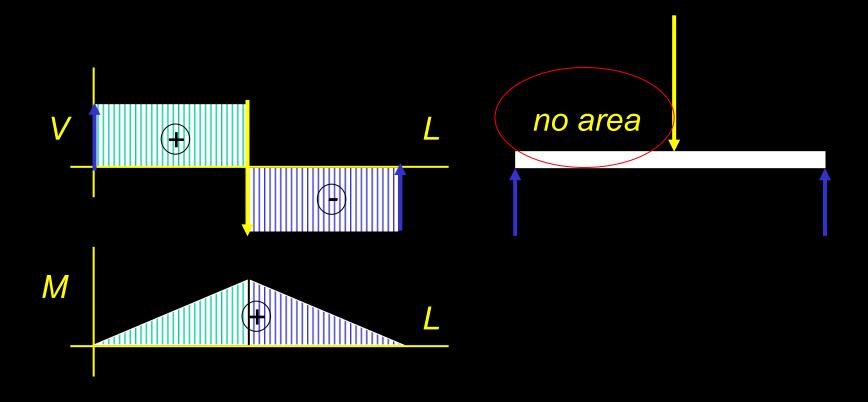


Figure 7.11 Relationship of load, shear, moment, slope, and deflection diagrams.

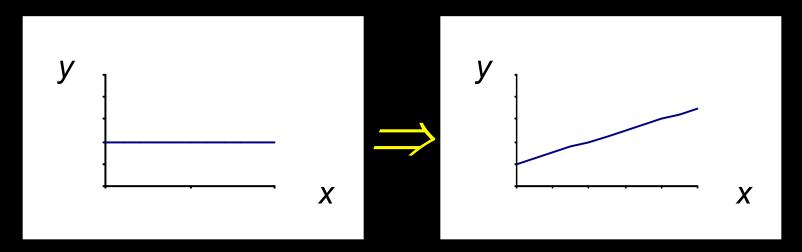
### Method 2: Semigraphical

•  $M_{max}$  occurs where V = 0 (calculus)



### Curve Relationships

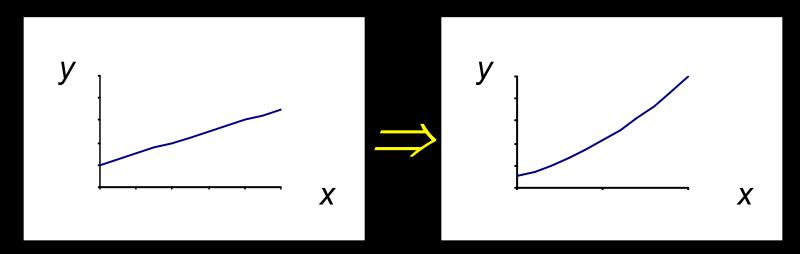
- integration of functions
- line with 0 slope, integrates to sloped



ex: load to shear, shear to moment

### Curve Relationships

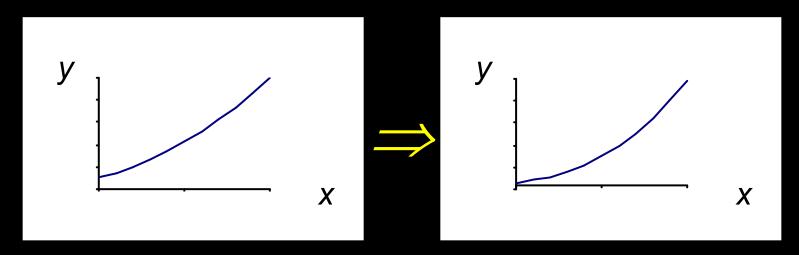
line with slope, integrates to parabola



ex: load to shear, shear to moment

### Curve Relationships

parabola, integrates to 3<sup>rd</sup> order curve



ex: load to shear, shear to moment

#### Basic Procedure with Sections

1. Find reaction forces & moments

Plot axes, underneath beam load diagram

V:

- 2. Starting at left
- 3. Shear is 0 at free ends
- 4. Shear has 2 values at point loads
- 5. Sum vertical forces at each section

#### Basic Procedure with Sections

**M**:

- 6. Starting at left
- 7. Moment is 0 at free ends
- 8. Moment has 2 values at moments
- 9. Sum moments at each section
- 10. Maximum moment is where shear = 0! (locate where V = 0)

## Basic Procedure by Curves

1. Find reaction forces & moments

Plot axes, underneath beam load diagram

V

- 2. Starting at left
- 3. Shear is 0 at free ends
- 4. Shear jumps with concentrated load
- 5. Shear changes with area under load

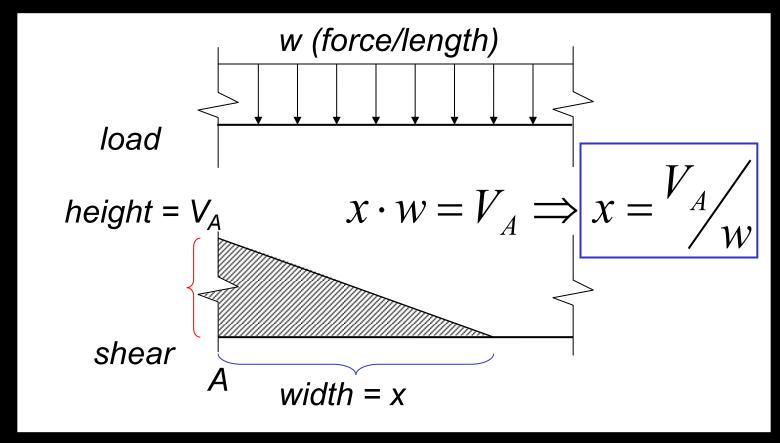
## Basic Procedure by Curves

**M**:

- 6. Starting at left
- 7. Moment is 0 at free ends
- 8. Moment jumps with moment
- 9. Moment changes with area under V
- 10. Maximum moment is where shear = 0! (locate where V = 0)

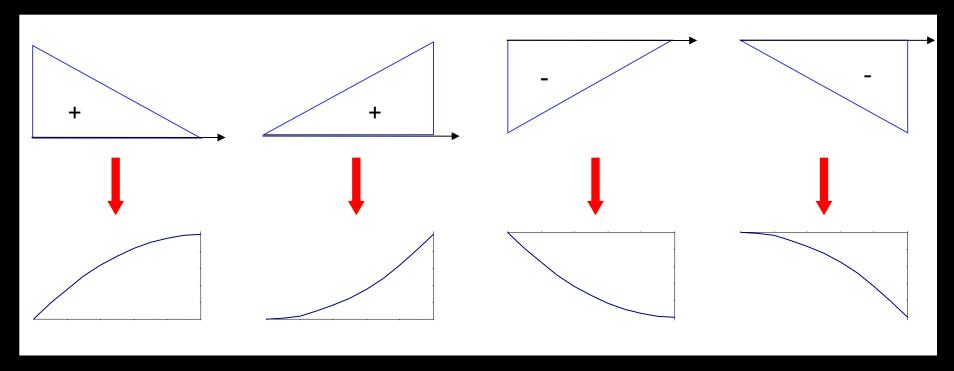
## Shear Through Zero

slope of V is w (-w:1)



# Parabolic Shapes

cases

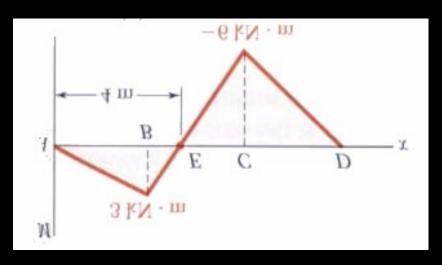


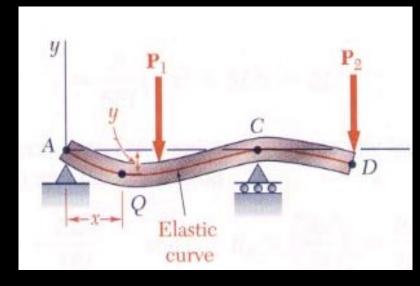
up fast, then slow

up slow, then fast down fast, then slow down slow, then fast

# Deflected Shape & M(x)

- -M(x) gives shape indication
- boundary conditions must be met

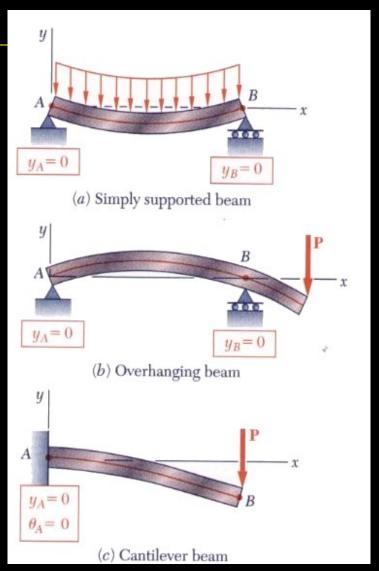




# **Boundary Conditions**

- at pins, rollers, fixed supports: y = 0
- at fixed supports:  $\theta = 0$
- at inflection points from symmetry:  $\theta = 0$

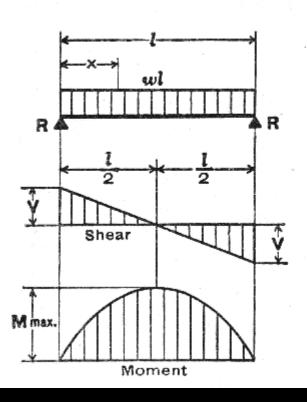
• 
$$y_{max}$$
 at  $\frac{dy}{dx} = 0$ 



## Tabulated Beam Formulas

#### how to read charts

#### SIMPLE BEAM—UNIFORMLY DISTRIBUTED LOAD



Total Equiv. Uniform Load . . . = 
$$wl$$

R = V . . . . . . . =  $\frac{wl}{2}$ 

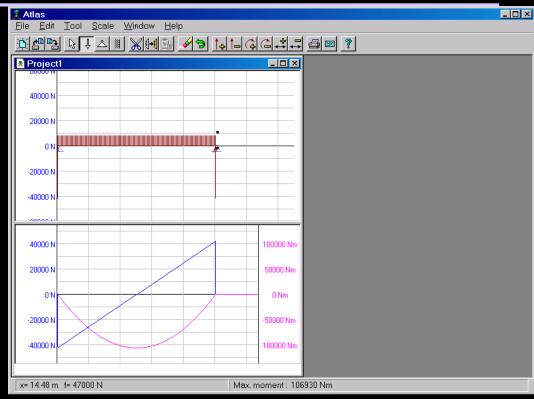
Vx . . . . . . . =  $w\left(\frac{l}{2} - x\right)$ 

M max. (at center) . . . =  $\frac{wl^2}{8}$ 

Mx . . . . . . . . . =  $\frac{wx}{2}(l-x)$ 
 $\Delta max$ . (at center) . . . =  $\frac{5wl^4}{384 \text{ El}}$ 
 $\Delta x$  . . . . . . . =  $\frac{wx}{24\text{El}}(l^3-2lx^2+x^3)$ 

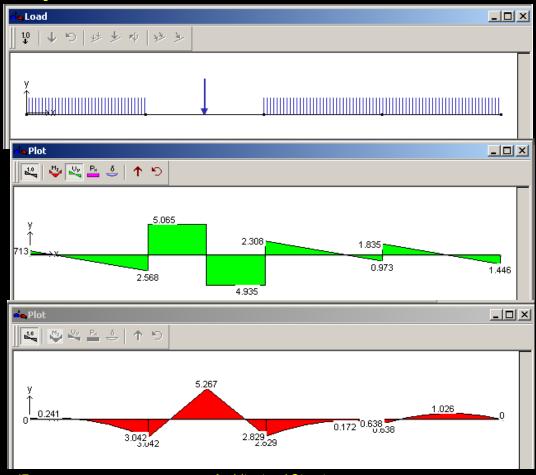
#### Tools

- software & spreadsheets help
- http://www.rekenwonder.com/atlas.htm



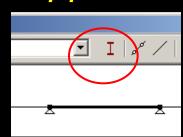
## Tools – Multiframe

• in computer lab

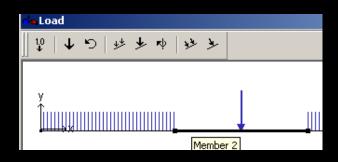


## Tools – Multiframe

- frame window
  - define beam members
- select points, assign supports
- select members,assign <u>section</u>



- load window
  - select point or member, add point or distributed loads

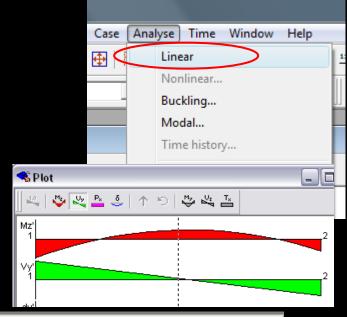


## Tools – Multiframe

- to run analysis choose
  - Analyze menu
    - Linear
- plot
  - choose options
  - double click (all)

Arc

- results
  - choose options



ri≤ Result ×						
Static Case: Load Case 1						
	Joint	Label	Rx' kip	Ry' kip	Mz' kip-ft	
1	1		0.000	-0.000	0.000	
2	2		0.000	9.250	0.000	
3	3		0.000	6.102	0.000	
4	4		0.000	3.093	0.000	
5	5		0.000	1.398	-0.000	
6	Total	(Global)	Rx=0.000	Ry=19.843		
▼ Displacements Reactions ▼						