

lecture
twenty five



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concrete construction: flat spanning systems

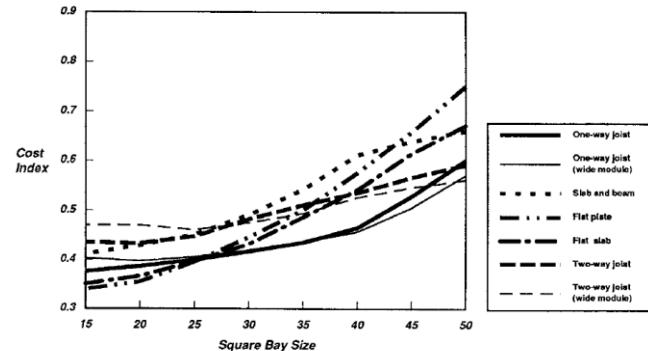
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Reinforced Concrete Design

- economical & common
- resist lateral loads



Concrete Spans 2
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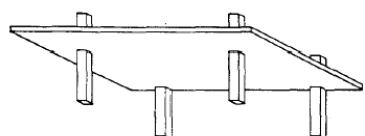
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Reinforced Concrete Design

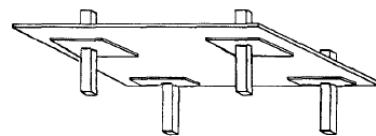
- flat plate

- 5"-10" thick
- simple formwork
- lower story heights



- flat slab

- same as plate
- 2 1/4"-8" drop panels



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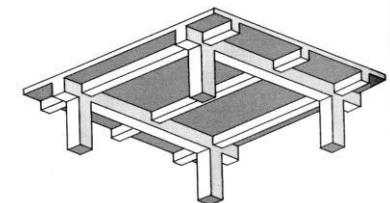
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Reinforced Concrete Design

- beam supported

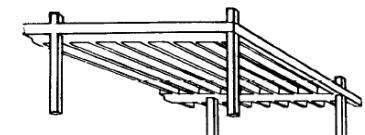
- slab depth ~ L/20
- 8"-60" deep



The Architect's Studio Companion

- one-way joists

- 3"-5" slab
- 8"-20" stems
- 5"-7" webs



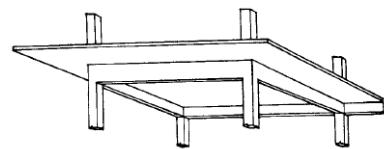
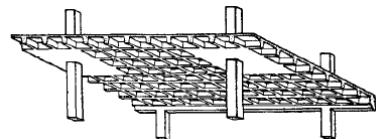
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Reinforced Concrete Design

- two-way joist
 - “waffle slab”
 - 3”-5” slab
 - 8”-24” stems
 - 6”-8” webs
- beam supported slab
 - 5”-10” slabs
 - taller story heights



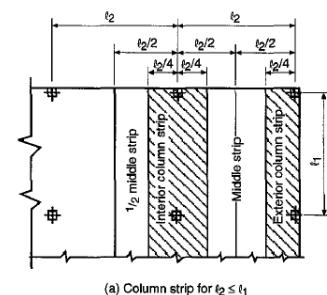
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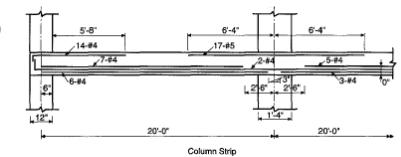
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Reinforced Concrete Design

- simplified frame analysis
 - strips, like continuous beams
- moments require flexural reinforcement
 - top & bottom
 - both directions of slab
 - continuous, bent or discontinuous



(a) Column strip for $l_2 \leq l_1$



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Reinforced Concrete Design

- one-way slabs (wide beam design)
 - approximate analysis for moment & shear coefficients
 - two or more spans
 - ~ same lengths
 - w_u from combos
 - uniform loads with $L/D \leq 3$
 - ℓ_n is clear span (+M) or average of adjacent clear spans (-M)

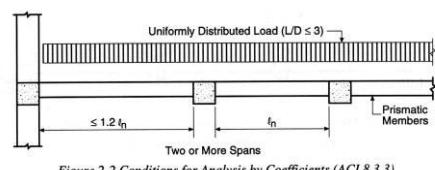


Figure 2-2 Conditions for Analysis by Coefficients (ACI 8.3.3)

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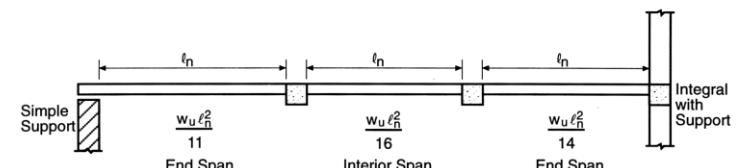


Figure 2-3 Positive Moments—All Cases

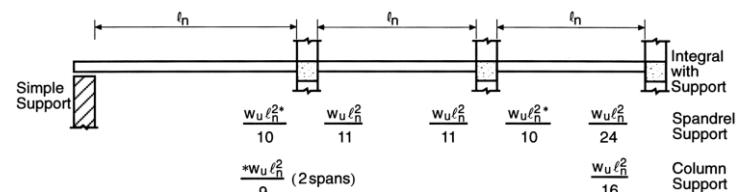


Figure 2-4 Negative Moments—Beams and Slabs

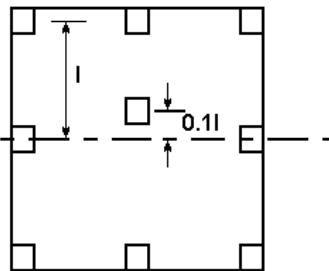
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Reinforced Concrete Design

- two-way slabs - Direct Design Method
 - 3 or more spans each way
 - uniform loads with $L/D \leq 2$
 - rectangular panels with long/short span ≤ 2
 - successive spans can't differ $> \text{longer}/3$
 - column offset no more than 10% span



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Reinforced Concrete Design

Table 4-6 Two-Way Beam-Supported Slab

Span ratio $\frac{l_2}{l_1}$	Slab Moments	End Span		Interior Span	
		1 Exterior Negative	2 Positive	3 First Interior Negative	4 Positive
0.5	Total Moment	0.16 M_o	0.57 M_o	0.70 M_o	0.35 M_o
	Column Strip Beam Slab	0.12 M_o 0.02 M_o	0.43 M_o 0.08 M_o	0.54 M_o 0.09 M_o	0.27 M_o 0.05 M_o
	Middle Strip	0.02 M_o	0.06 M_o	0.07 M_o	0.03 M_o
1.0	Column Strip Beam Slab	0.10 M_o 0.02 M_o	0.37 M_o 0.06 M_o	0.45 M_o 0.08 M_o	0.22 M_o 0.04 M_o
	Middle Strip	0.04 M_o	0.14 M_o	0.17 M_o	0.09 M_o
	Column Strip Beam Slab	0.06 M_o 0.01 M_o	0.22 M_o 0.04 M_o	0.27 M_o 0.05 M_o	0.14 M_o 0.02 M_o
2.0	Middle Strip	0.09 M_o	0.31 M_o	0.38 M_o	0.19 M_o
	Column Strip Beam Slab	0.01 M_o	0.04 M_o	0.05 M_o	0.04 M_o

Notes: (1) Beams and slab satisfy stiffness criteria: $\alpha_1 l_2 / l_1 \geq 1.0$ and $\beta_1 \geq 2.5$.

(2) Interpolate between values shown for different $\frac{l_2}{l_1}$ ratios.

(3) All negative moments are at face of support.

(4) Concentrated loads applied directly to beams must be accounted for separately.

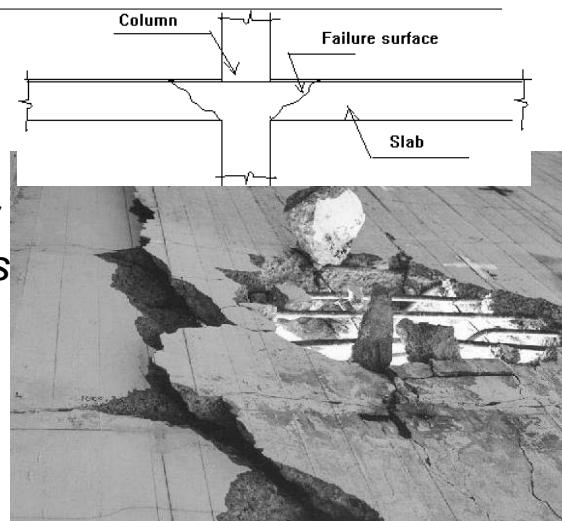
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Shear in Concrete

- at columns
- want to avoid stirrups
- can use shear studs or heads



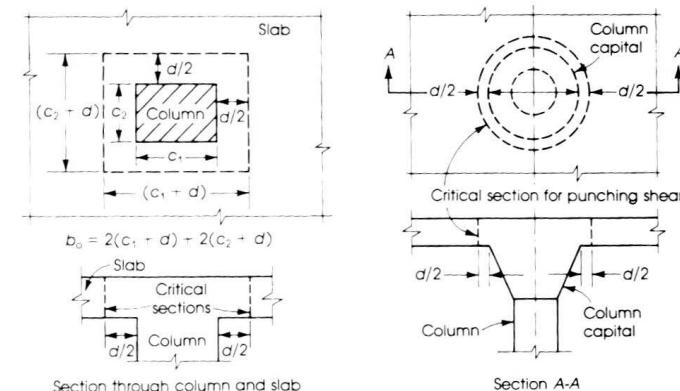
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Shear in Concrete

- critical section at $d/2$ from
 - column face, column capital or drop panel



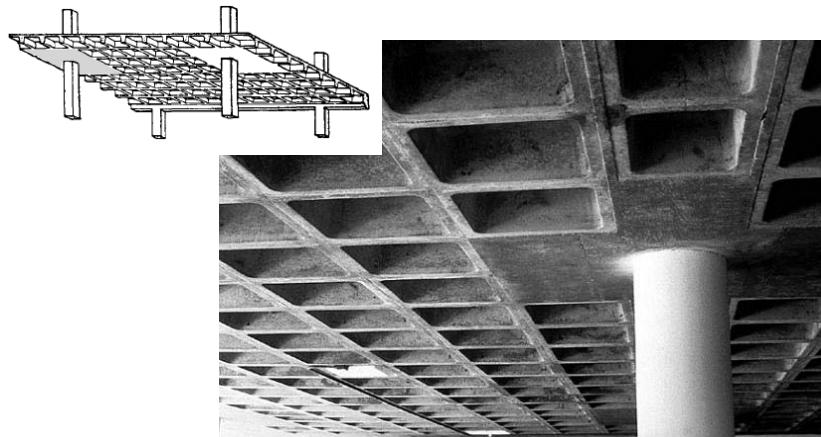
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Shear in Concrete

- at columns with waffle slabs



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Openings in Slabs

- careful placement of holes
- shear strength reduced
- bending & deflection can increase

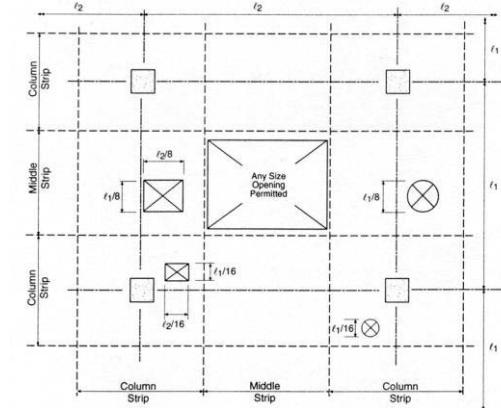


Figure 18-11 Openings in Slab Systems without Beams

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General Beam Design

- f'_c & f_y needed
- usually size just b & h
 - even inches typical (forms)
 - similar joist to beam depth
 - $b:h$ of 1:1.5-1:2.5
 - b_w & b_f for T
 - to fit reinforcement + stirrups
- slab design, t
 - deflection control & shear

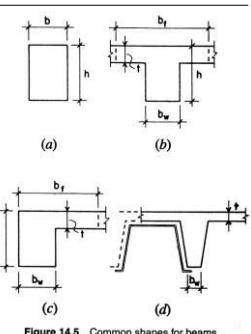


Figure 14.5 Common shapes for beams.

$$S = \frac{bh^2}{6}$$

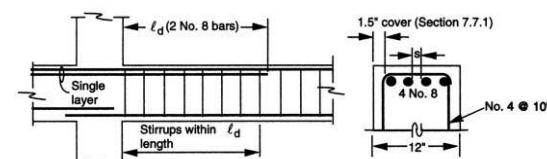
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General Beam Design (cont'd)

- custom design:
 - longitudinal steel
 - shear reinforcement
 - detailing



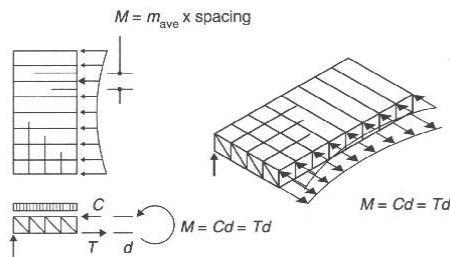
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Space “Frame” Behavior

- handle uniformly distributed loads well
- bending moment
 - tension & compression “couple” with depth
 - member sizes can vary, but difficult



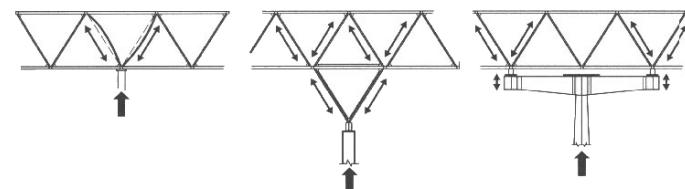
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Space “Frame” Behavior

- shear at columns
- support conditions still important
 - point supports not optimal
- fabrication/construction can dominate design



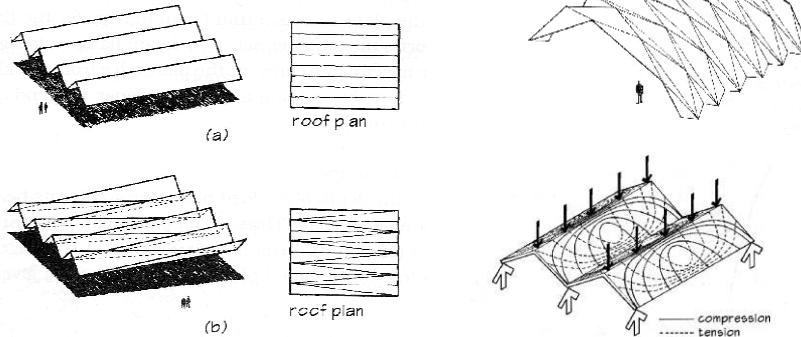
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Folded Plates

- increased bending stiffness with folding
- lateral buckling avoided



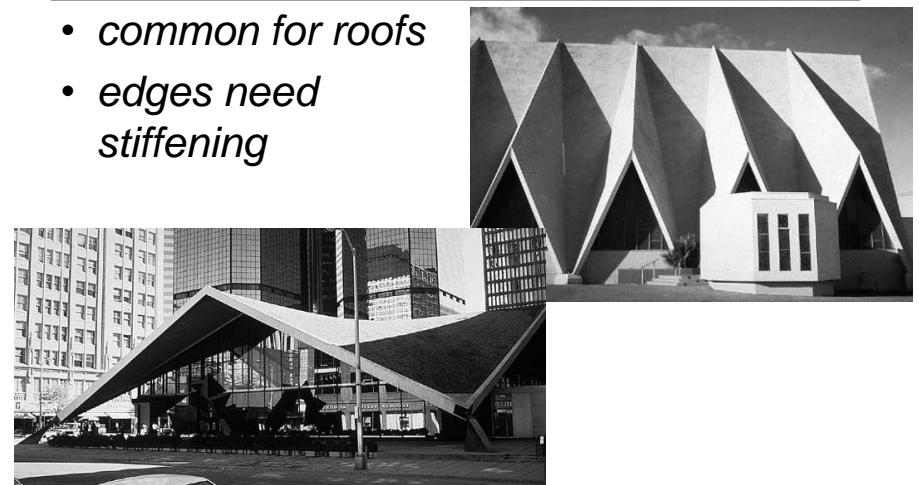
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Folded Plates

- common for roofs
- edges need stiffening



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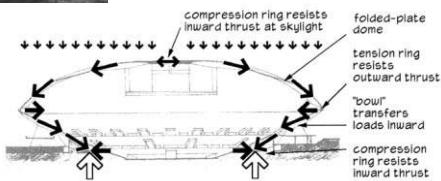
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Folded Plates



www.library.illinois.edu



- State Farm Center
(Assembly Hall), University of Illinois
- Harrison & Abramovitz 1963
- Edge-supported dome spanning 400 feet wound
with 614 miles of one-fifth inch steel wire

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