

ARCHITECTURAL STRUCTURES:
FORM, BEHAVIOR, AND DESIGN

ARCH 331

DR. ANNE NICHOLS

SUMMER 2013

lecture
eighteen

steel construction
bolts, welds & light gages



nrmc.org

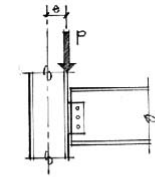
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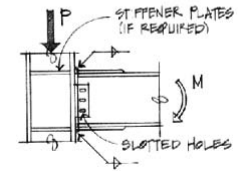
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Connections

- needed to:
 - support beams by columns
 - connect truss members
 - splice beams or columns
- transfer load
- subjected to
 - tension or compression
 - shear
 - bending



(a) Framed beam (shear) connection.
 e = Eccentricity; $M = P \times e$



(b) Moment connection (rigid frame).
 M = Moment due to beam bending

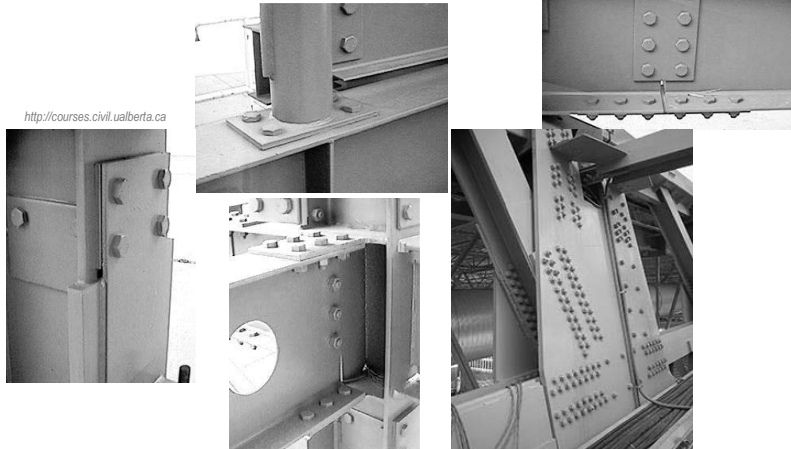
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Bolts

- bolted steel connections



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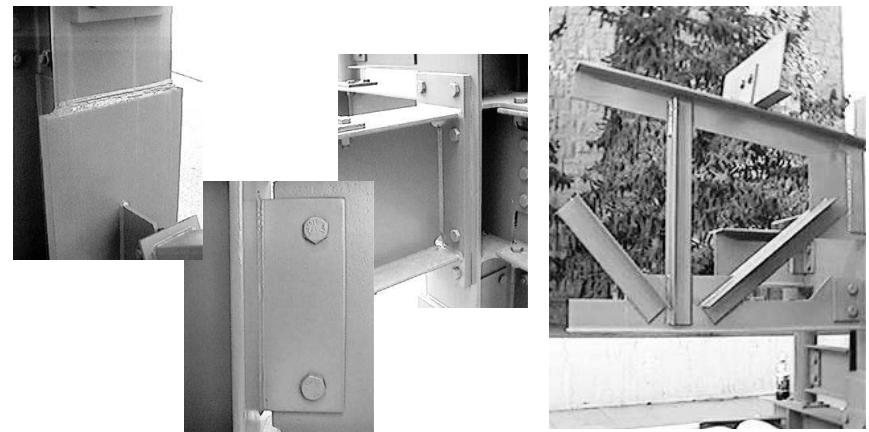
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(AISC - Steel Structures of the Everyday)

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Welds

- welded steel connections



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Bolts

- types

- materials

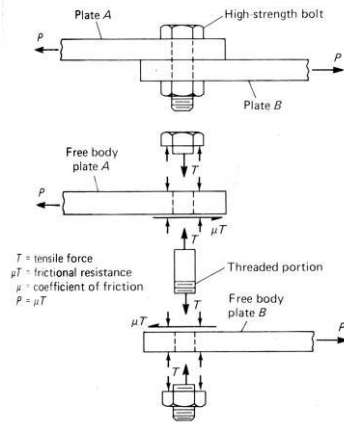
- high strength
- A307, A325, A490

- location of threads

- included - N
- excluded - X

- friction or bearing (SC)

- always tightened



Bolted Connection Design

- considerations

- bearing stress

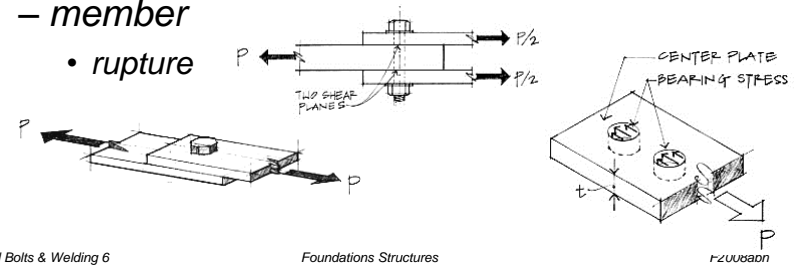
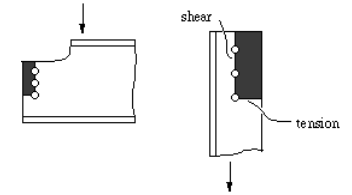
- yielding

- shear stress

- single & double

- member

- rupture



Bolts

- rarely fail in bearing

- holes considered 1/8" larger

- shear & tension

$$R_a \leq R_n / \Omega \quad R_u \leq \phi_v R_n$$

$$\phi_v = 0.75$$

- single shear or tension

$$R_n = F_n A_b$$

- double shear

$$R_n = F_n 2A_b$$

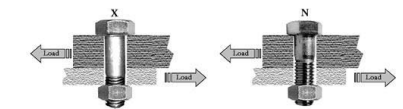
Bolts

Table 7-1
Available Shear Strength of Bolts, kips

Nominal Bolt Diameter, d, in.		3/8		1/2		3/4		1				
Nominal Bolt Area, in. ²		0.307		0.442		0.601		0.786				
ASTM Desig.	Thread Cond.	F _u /k (ksi)	F _y /k (ksi)	F _u /k (ksi)	F _y /k (ksi)	F _u /k (ksi)	F _y /k (ksi)	F _u /k (ksi)	F _y /k (ksi)			
Group A	N	27.0	40.5	S	8.29	12.4	11.9	17.0	16.2	24.3	21.2	31.8
	X	34.0	51.0	D	16.6	24.8	23.9	35.8	32.5	48.7	42.4	63.8
Group B	N	34.0	51.0	S	10.4	15.7	15.0	22.5	20.4	30.7	26.7	40.0
	X	42.0	63.0	D	20.9	31.3	30.1	45.1	40.9	61.3	53.4	80.1
AST7	-	13.5	20.3	D	4.14	6.25	5.87	8.97	8.11	12.2	10.8	15.9
	-	13.5	20.3	D	8.29	12.5	11.9	17.0	16.2	24.4	21.2	31.8

Table 7-2
Available Tensile Strength of Bolts, kips

Nominal Bolt Diameter, d, in.		3/8		1/2		3/4		1			
Nominal Bolt Area, in. ²		0.307		0.442		0.601		0.786			
ASTM Desig.	Thread Cond.	F _u /k (ksi)	F _y /k (ksi)	F _u /k (ksi)	F _y /k (ksi)	F _u /k (ksi)	F _y /k (ksi)	F _u /k (ksi)	F _y /k (ksi)		
Group A	N	45.0	67.5	13.8	20.7	19.9	29.8	27.1	40.6	35.3	53.0
	X	56.5	84.8	17.3	26.0	25.0	37.4	34.0	51.0	44.4	66.6
AST7	-	22.5	33.8	6.90	10.4	9.94	14.9	13.5	20.3	17.7	26.5
	-	22.5	33.8	6.90	10.4	9.94	14.9	13.5	20.3	17.7	26.5



Bolts

- bearing

$$R_a \leq \frac{R_n}{\Omega} \quad R_u \leq \phi R_n$$

$$\phi = 0.75$$

– deformation is concern

$$R_n = 1.2L_c t F_u \leq 2.4dt F_u$$

– deformation isn't concern

$$R_n = 1.5L_c t F_u \leq 3.0dt F_u$$

– long slotted holes

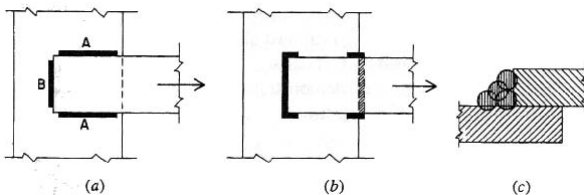
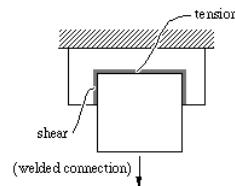
$$R_n = 1.0L_c t F_u \leq 2.0dt F_u$$

L_c – clear length to edge or next hole (ex. $1\frac{1}{4}$ ", 3")

Welded Connection Design

- considerations

- shear stress
- yielding
- rupture



Bolts

Table 7-5 Available Bearing Strength at Bolt Holes Based on Edge Distance kips/in. thickness

Hole Type	Edge Distance L_e , in.	F_u , ksi	Nominal Bolt Diameter, d, in.											
			$\frac{3}{8}$		$\frac{1}{2}$		$\frac{3}{4}$		1		1 1/4		1 1/2	
			ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
STD	1 1/4	58	31.5	47.3	29.4	44.0	27.2	40.8	25.0	37.5	23.0	34.5	21.5	32.0
		65	35.3	53.0	32.9	49.4	30.5	45.7	28.0	42.0	26.0	39.0	24.0	36.0
SSLT	2	58	43.5	65.3	52.2	78.3	53.3	79.9	51.1	76.7	48.8	73.1	46.8	70.1
		65	49.6	73.1	58.5	87.8	59.7	89.6	57.3	85.8	53.6	80.4	51.1	76.7
SSLP	1 1/4	58	28.3	42.4	26.1	39.2	23.9	35.9	20.7	31.0	18.8	28.1	16.8	25.1
		65	31.7	47.5	29.3	43.9	26.8	40.2	23.2	34.7	21.5	32.0	19.5	29.0
OVS	1 1/4	58	29.4	44.0	27.2	40.8	26.0	37.5	21.8	32.8	20.0	30.0	18.0	27.0
		65	32.9	49.4	30.5	45.7	28.0	42.0	24.4	36.6	22.0	33.0	20.0	29.0
LSLP	1 1/4	58	43.5	65.3	52.2	78.3	50.0	75.0	48.8	73.1	46.8	70.1	44.8	69.1
		65	48.8	73.1	58.5	87.8	56.1	84.1	52.4	78.6	50.4	76.6	48.4	74.6
LSLT	2	58	26.3	39.4	24.5	36.7	22.7	34.0	20.8	29.3	19.0	27.5	17.2	25.7
		65	29.5	44.2	27.4	41.1	25.4	38.1	23.4	33.6	21.4	29.6	19.4	25.6
STD, SSLT, OVS, LSLP	$L_e \geq 4d_{nom}$	58	43.5	65.3	52.2	78.3	60.9	91.4	69.6	104	88.0	131	117	
		65	49.6	73.1	58.5	87.8	68.3	102	78.0	117	98.0	141	127	
LSLT	$L_e \geq 4d_{nom}$	58	36.3	54.4	43.5	65.3	50.8	76.1	58.0	87.0	53.0	78.0	51.0	76.0
		65	40.8	60.9	48.8	73.1	58.9	85.3	65.0	91.5	61.0	87.5	63.0	89.5

Table 7-3 (continued) Slip-Critical Connections Available Shear Strength, kips (Class A Faying Surface, $\mu = 0.30$)

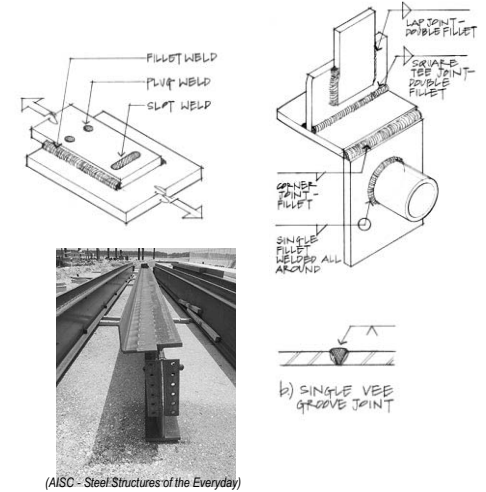
Hole Type	Loading	Nominal Bolt Diameter, d, in.	Group B Bolts											
			$\frac{3}{8}$		$\frac{1}{2}$		$\frac{3}{4}$		1		1 1/4		1 1/2	
			Minimum Group B Bolt Pretension, kips											
			24	35	49	64	84	104	124	144	164	184	204	
STD/SSLT	S	58	5.42	8.14	7.91	11.0	11.1	16.6	14.5	21.7	21.8	33.2	28.9	43.4
		65	10.8	16.3	15.8	23.7	22.1	33.2	28.9	43.4	43.4	66.4	57.8	86.8
OVS/SSLP	S	58	4.82	6.92	6.74	10.1	8.44	14.1	12.3	18.4	18.5	27.8	24.7	36.9
		65	9.25	13.8	13.5	20.2	19.9	29.2	24.7	36.9	36.9	55.6	49.4	73.8
LSL	D	58	3.80	5.70	5.54	8.31	7.76	11.6	10.1	15.2	15.3	23.0	20.3	30.4
		65	7.60	11.4	11.1	16.6	15.5	23.3	20.3	30.4	30.4	46.6	40.6	60.8

Welded Connection Design

- weld terms

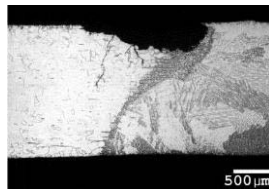
- butt weld
- fillet weld
- plug weld
- throat

- field welding
- shop welding



Welded Connection Design

- weld process
 - melting of material
 - melted filler - electrode
 - shielding gas / flux
 - potential defects
- weld materials
 - E60XX
 - E70XX
 - $F_{EXX} = 70 \text{ ksi}$



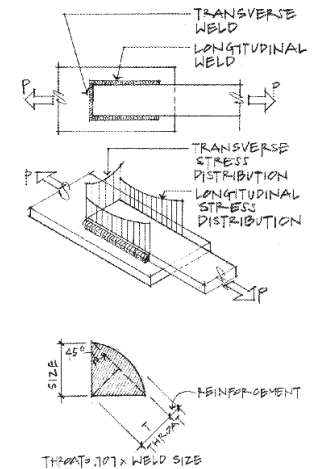
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Welded Connection Design

- shear failure assumed
- throat
 - $T = 0.707 \times \text{weld size}$
- area
 - $A = T \times \text{length of weld}$
- weld metal generally stronger than base metal (ex. $F_y = 50 \text{ ksi}$)



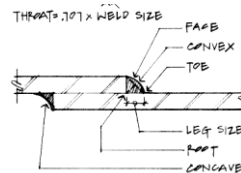
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Welded Connection Design

- minimum
 - table
- maximum
 - material thickness (to 1/4")
 - 1/16" less
- min. length
 - 4 x size min.
 - $\geq 1 \frac{1}{2}$ "



Material Thickness of Thicker Part Joined, in. (mm)	Minimum Size of Fillet Weld ^(a) in. (mm)
To 1/8 (6) Inclusive	1/8 (3)
Over 1/8 (6) to 1/2 (13)	3/16 (5)
Over 1/2 (13) to 3/4 (19)	1/4 (6)
Over 3/4 (19)	5/16 (8)

(a) Leg dimension of fillet welds. Single pass welds must be used.
(b) See Section J2.2c for maximum size of fillet welds.

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Welded Connection Design

- shear

$$R_a \leq \frac{R_n}{\Omega} \quad R_u \leq \phi R_n$$

$$\phi = 0.75$$

$$R_n = 0.6 F_{EXX} \underbrace{Tl}_{\text{area}} = Sl$$

- table for ϕS

Weld Size (in.)	E60XX (k/in.)	E70XX (k/in.)
1/16	2.39	4.18
1/8	4.77	5.57
3/16	5.97	6.96
1/4	7.16	8.35
5/16	5.57	9.74
3/8	8.35	11.14
7/16	11.93	13.92
1/2	14.32	16.70

(not considering increase in throat with submerged arc weld process)

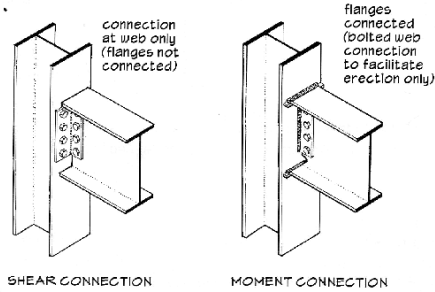
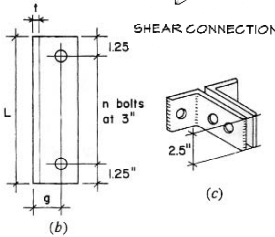
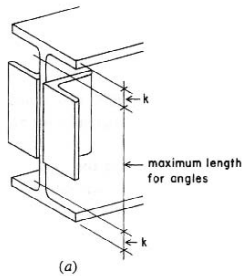
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Framed Beam Connections

- angles
 - bolted
 - welded



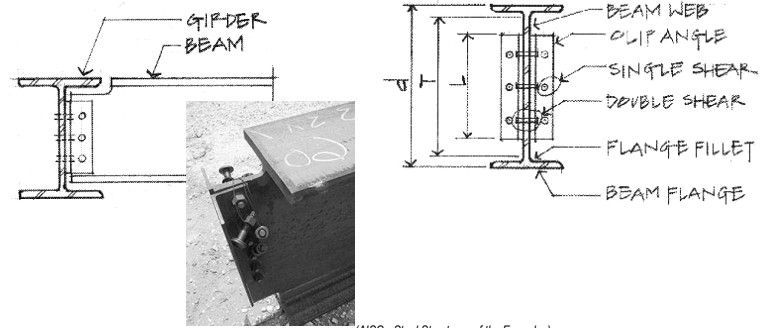
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Framed Beam Connections

- terms
 - coping



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Framed Beam Connections

- tables for standard bolt sizes & spacings
- # bolts
- bolt diameter, angle leg thickness
- bearing on beam web

Table 10-1 (continued)
All-Bolted Double-Angle Connections
3/4-in. Bolts

Beam: $F_y = 50$ ksi, $F_u = 65$ ksi
Angle: $F_y = 36$ ksi, $F_u = 58$ ksi

A Rows W4, 21, 18, 16	Bolt Group	Thread Cond.	Hole Type	Bolt and Angle Available Strength, kips								
				Angle Thickness, in.								
				1/4	1/2	3/4	1	1 1/4	1 1/2			
Group A	N	STD	STD	87.1	101	83.9	126	95.5	143	95.5	143	
		X	STD	87.1	101	83.9	126	101	151	129	180	
		SC	STD	50.6	75.9	50.6	75.9	50.6	75.9	50.6	75.9	
		Class A	OVS	43.1	64.5	43.1	64.5	43.1	64.5	43.1	64.5	
		Class B	SFLT	50.6	75.9	50.6	75.9	50.6	75.9	50.6	75.9	
		Class C	SFLT	67.1	101	83.9	126	84.4	127	84.4	127	
	Group B	N	STD	STD	67.1	101	83.9	126	101	151	120	180
			X	STD	67.1	101	83.9	126	101	151	134	201
			SC	STD	63.3	94.9	63.3	94.9	63.3	94.9	63.3	94.9
		Class A	OVS	53.9	80.7	53.9	80.7	53.9	80.7	53.9	80.7	
			Class B	SFLT	63.3	94.9	63.3	94.9	63.3	94.9	63.3	94.9
			Class C	SFLT	67.1	101	83.9	126	101	151	105	158

Beam Web Available Strength per Inch Thickness, kips/in.

Hole Type	STD		OVS		SFLT								
	1 1/2	1 3/4	1 1/2	1 3/4	1 1/2	1 3/4							
Coped at Top Flange Only	1 1/4	167	250	175	252	234	184	246	184	245	172	257	
	1 1/2	189	254	177	256	198	238	187	250	185	249	174	251
	1 3/4	171	257	180	269	181	241	189	254	188	253	177	265
Coped at Both Flanges	1 1/4	174	261	182	273	183	245	171	257	171	256	179	268
	1 1/2	181	272	189	284	171	256	179	268	178	267	186	278
	1 3/4	201	301	209	313	189	265	189	297	188	286	206	309
Uncoped	1 1/4	158	234	156	234	146	219	156	234	156	234		
	1 1/2	169	249	166	249	156	234	156	234	166	249		
	1 3/4	171	256	171	256	161	241	181	241	171	256		

Support Available Strength per Inch Thickness, kips/in.

Hole Type	STD		OVS		SFLT							
	1 1/2	1 3/4	1 1/2	1 3/4	1 1/2	1 3/4						
1 1/4	167	250	175	252	234	184	246	184	245	172	257	
1 1/2	189	254	177	256	198	238	187	250	185	249	174	251
1 3/4	171	257	180	269	181	241	189	254	188	253	177	265

Notes:
 STD = Standard holes
 OVS = Overlapped holes
 SFLT = Slotted holes transverse to direction of load
 N = Through included
 X = Through excluded
 SC = Slip critical

* Tabulated values include 1/4-in. reduction in end distance, L_{eh} , to account for possible undercuts in beam length.
 Note: Slip-critical bolt values assume no more than one fillet has been provided or bolts have been added to distribute loads in the flanges.

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Framed Beam Connections

- welded example (shear)



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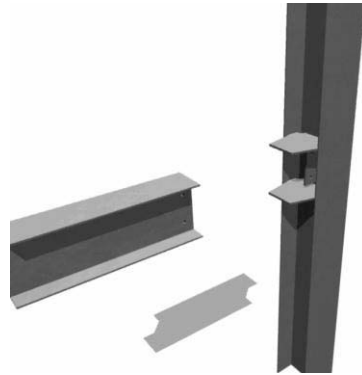
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Framed Beam Connections

- welded moment example



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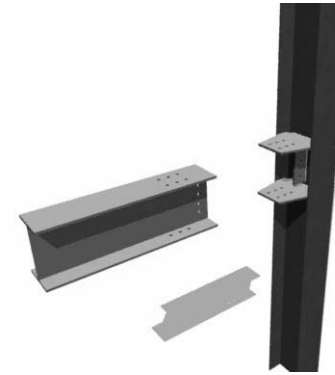
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Framed Beam Connections

- welded/bolted moment example



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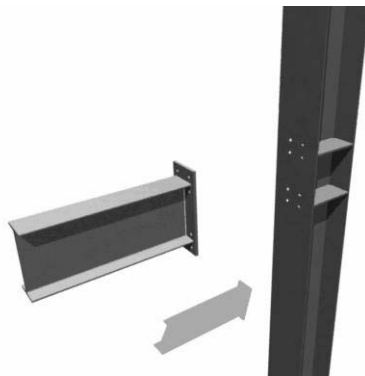
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Framed Beam Connections

- welded/bolted moment example



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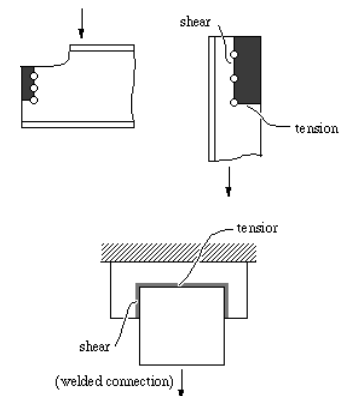
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Beam Connections

- LRFD provisions
 - shear yielding
 - shear rupture
 - block shear rupture
 - tension yielding
 - tension rupture
 - local web buckling
 - lateral torsional buckling



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Beam Connections

$$\phi = 0.75$$

$$R_n = 0.6F_u A_{nv} + U_{bs} F_u A_{nt} \leq 0.6F_y A_{gv} + U_{bs} F_u A_{nt}$$

– where U_{bs} is 1 for uniform tensile stress



Figure 2-1. Block Shear Rupture Limit State
(Photo by J.A. Swanson and R. Leon, courtesy of Georgia Institute of Technology)

block shear rupture

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Figure 2-14. Tension Fracture Limit State
(Photo by J.A. Swanson and R. Leon, courtesy of Georgia Institute of Technology)

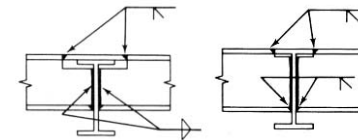
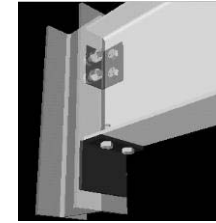
tension rupture

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Other Connections

- seated beam
- continuous
 - beam to column
 - beam to beam



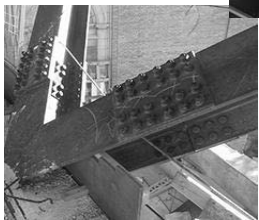
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Other Connections

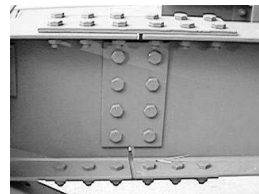
- splices



The Royal Ontario Museum Toronto, Canada
Daniel Libeskind
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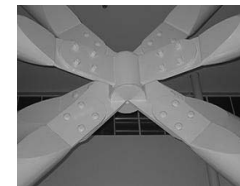
Steel Bolts & Welding 27
Lecture 21

Other Connections

- rigid frame knees
- gussets & joints

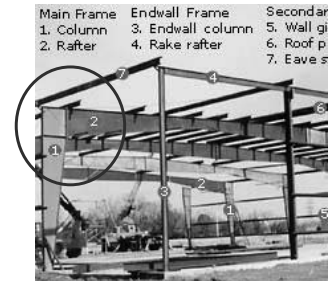


(AISC - Steel Structures of the Everyday)



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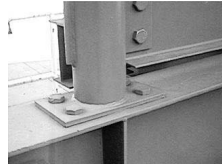
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Other Connections

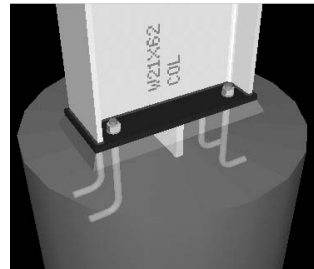
- *base plates*
 - *anchor bolts*
 - *bearing on steel*
 - *bending of plate*



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