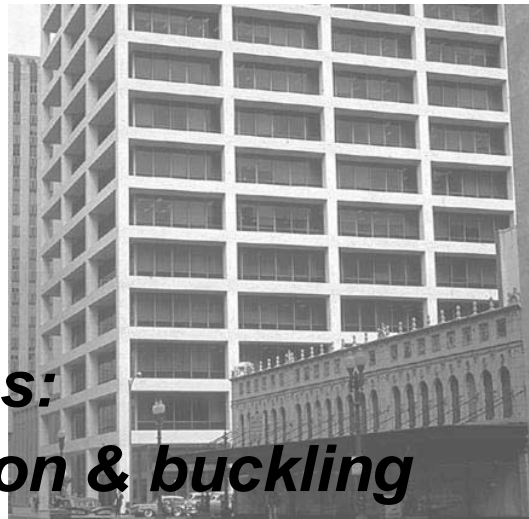


lecture
ten

rigid frames:
compression & buckling



Rigid Frames 1
Lecture 10

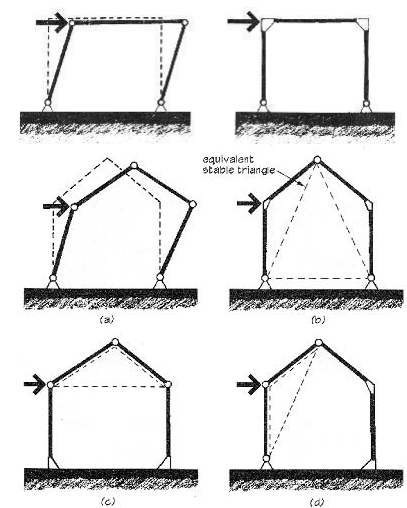
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<http://nisee.berkeley.edu/godden>

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Rigid Frames

- rigid frames have no pins
- frame is all one body
- joints transfer moments and shear
- typically statically indeterminate
- types
 - portal
 - gable



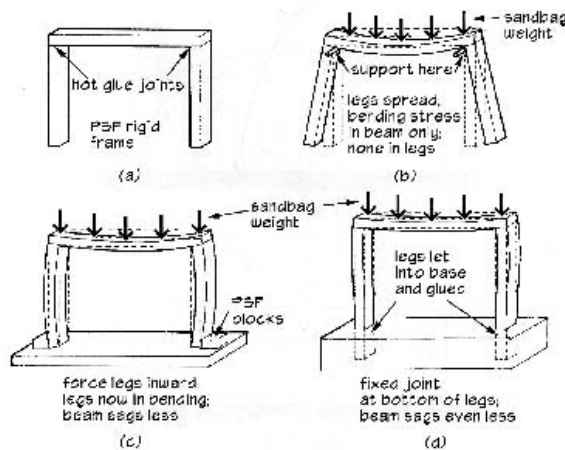
Rigid Frames 2
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Rigid Frames

- behavior



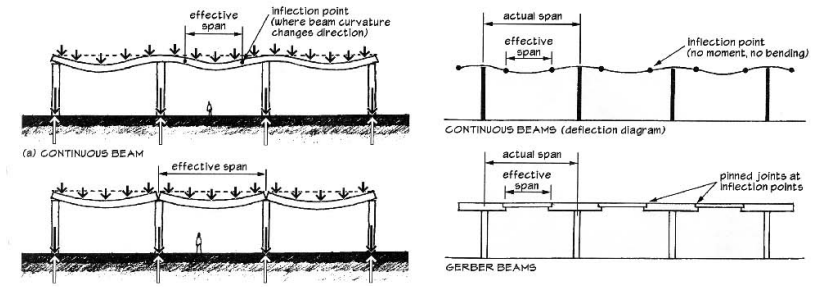
Rigid Frames 3
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Rigid Frames

- moments get redistributed
- deflections are smaller
- effective column lengths are shorter
- very sensitive to settling



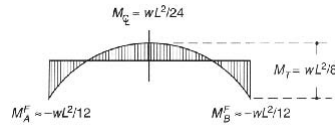
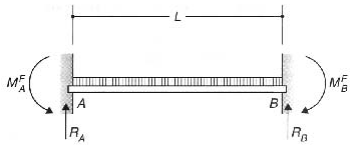
Rigid Frames 4
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Moment Redistribution

- continuous slabs & beams with uniform loading
 - joints similar to fixed ends, but can rotate
- change in moment to center = $\frac{wL^2}{8}$
 - M_{max} for simply supported beam



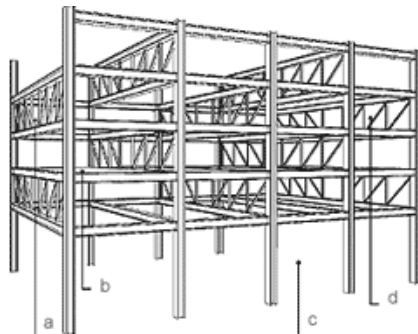
Rigid Frames 5
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Rigid Frames

- staggered truss
 - rigidity
 - clear stories



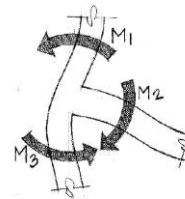
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Rigid Frames

- resists lateral loadings
- shape depends on stiffness of beams and columns
- 90° maintained



Rigid Frames 6
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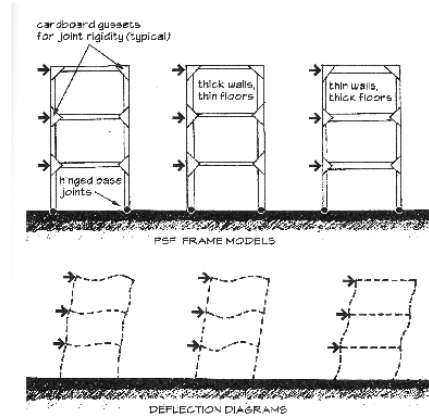
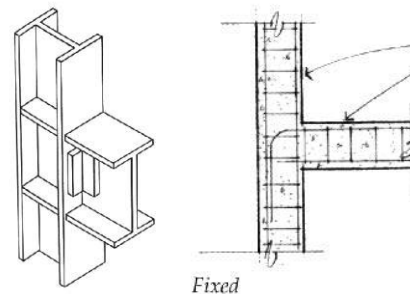


Figure 9.19: Modal demonstration of the effects of varying the stiffness of beams and columns when a building frame is subjected to lateral loads.

Rigid Frames

- connections
 - steel
 - concrete



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Rigid Frames 8
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Braced Frames

- pin connections
- bracing to prevent lateral movements



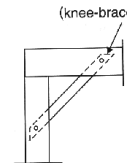
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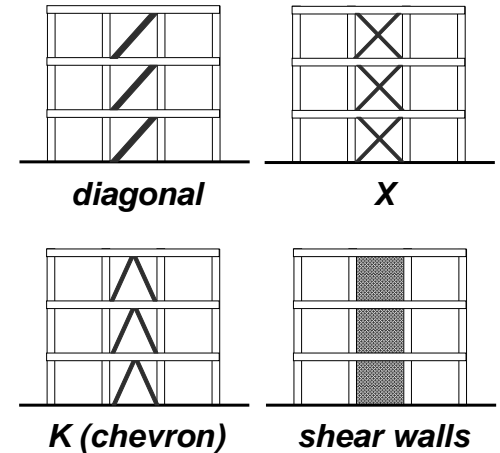
Braced Frames

- types of bracing
 - knee-bracing
 - diagonal
 - X
 - K or chevron
 - shear walls



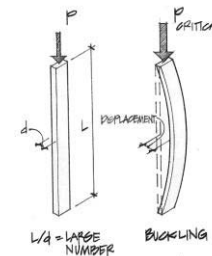
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Compression Members

- designed for strength & stresses
- designed for serviceability & deflection
- need to design for stability
 - ability to support a specified load without sudden or unacceptable deformations



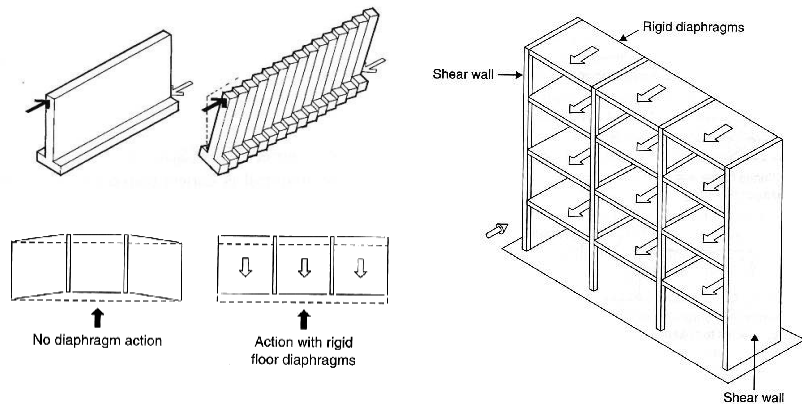
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Shear Walls

- resist lateral load in plane with wall



Rigid Frames 11
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Column Buckling

- axially loaded columns
- long & slender
 - unstable equilibrium = buckling
 - sudden and not good

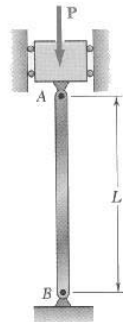


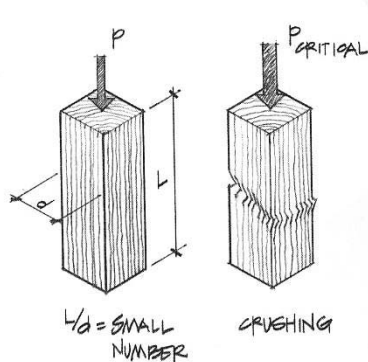
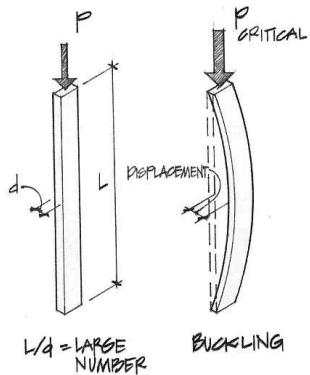
Fig. 10.1



Fig. 10.2

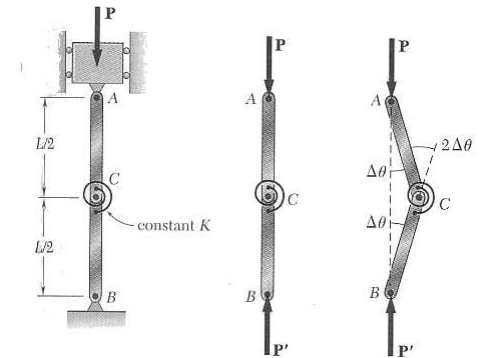
Effect of Length

- long & slender
- short & stubby



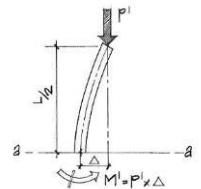
Modeling

- can be modeled with a spring at mid-height
- when moment from deflection exceeds the spring capacity ... “boing”
- critical load P



Buckling Load

- related to deflected shape ($P\Delta$)
- shape of sine wave
- Euler's Formula
- smallest I governs



$$P_{critical} = \frac{\pi^2 EI}{(L)^2}$$



Figure 9.3 Leonhard Euler (1707–1783).

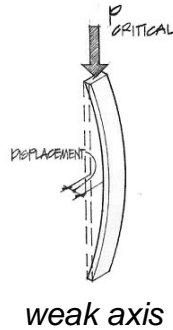
Critical Stress

- short columns

$$f_{critical} = \frac{P_{actual}}{A} < F_a$$

- slenderness ratio = L_e/r (L/d)

- radius of gyration = $r = \sqrt{\frac{I}{A}}$



$$f_{critical} = \frac{P_{critical}}{A} = \frac{\pi^2 E A r^2}{A (L_e)^2} = \frac{\pi^2 E}{\left(\frac{L_e}{r}\right)^2} \quad P_{critical} = \frac{\pi^2 E A}{\left(\frac{L_e}{r}\right)^2}$$

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Effective Length

- end conditions affect shape
- effective length factor, K $L_e = K \cdot L$

Buckled shape of column shown by dashed line	(a)	(b)	(c)	(d)	(e)	(f)
Theoretical K value	0.5	0.7	1.0	1.0	2.0	2.0
Recommended design values when ideal conditions are approximated	0.65	0.80	1.0	1.2	2.10	2.0

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Critical Stresses

- when a column gets stubby, F_y will limit the load
- real world has loads with eccentricity
- C_c for steel and allowable stress

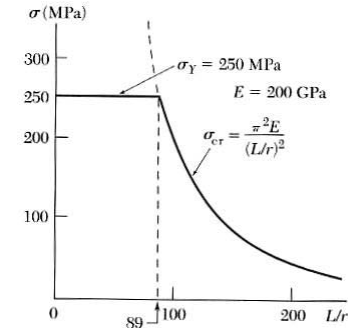


Fig. 10.9

$$\frac{L_e}{r} > C_c = \sqrt{\frac{2\pi^2 E}{F_y}}$$

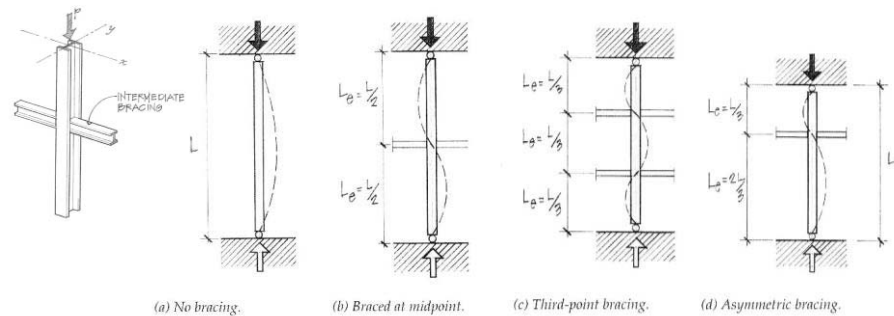
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Bracing

- bracing affects shape of buckle in one direction
- both should be checked!



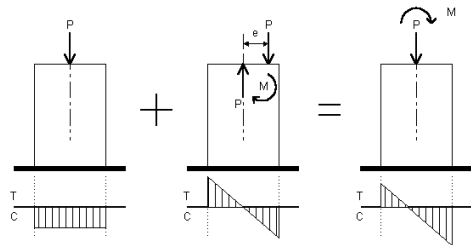
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Centric & Eccentric Loading

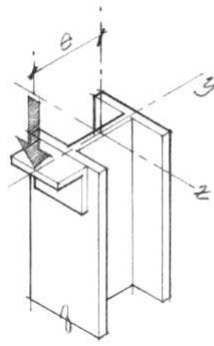
- **centric**
 - allowable stress from strength or buckling
- **eccentric**
 - combined stresses



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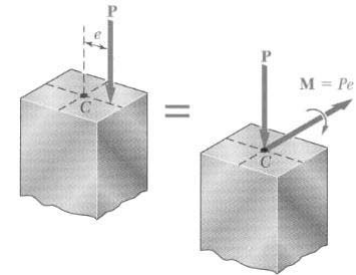


Combined Stresses

- axial + bending

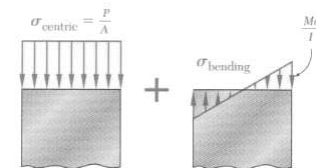
$$f_{\max} = \frac{P}{A} + \frac{Mc}{I}$$

$$M = P \cdot e$$



- design

$$f_{\max} \leq F_{cr} = \frac{f_{cr}}{F.S.}$$



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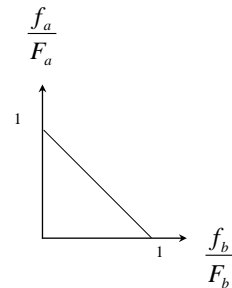
Stress Limit Conditions

- ASD interaction formula

$$\frac{f_a}{F_a} + \frac{f_b}{F_b} \leq 1.0$$

- with biaxial bending

$$\frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} + \frac{f_{by}}{F_{by}} \leq 1.0$$



interaction diagram

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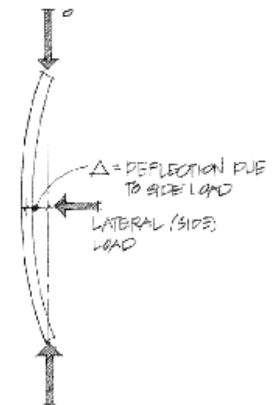
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Stress Limit Conditions

- in reality, as the column flexes, the moment increases

- P-Δ effect

$$\frac{f_a}{F_a} + \frac{f_b \times (\text{Magnification factor})}{F_{bx}} \leq 1.0$$



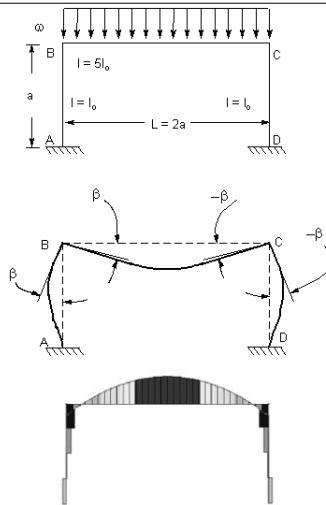
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Rigid Frame Analysis

- members see
 - shear
 - axial force
 - bending
- V & M diagrams
 - plot on "outside"



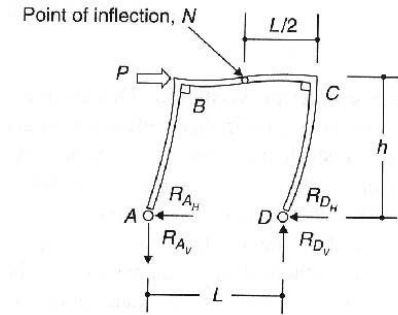
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Rigid Frame Analysis

- need support reactions
- free body diagram each member
- end reactions are equal and opposite on next member
- "turn" member like beam
- draw V & M



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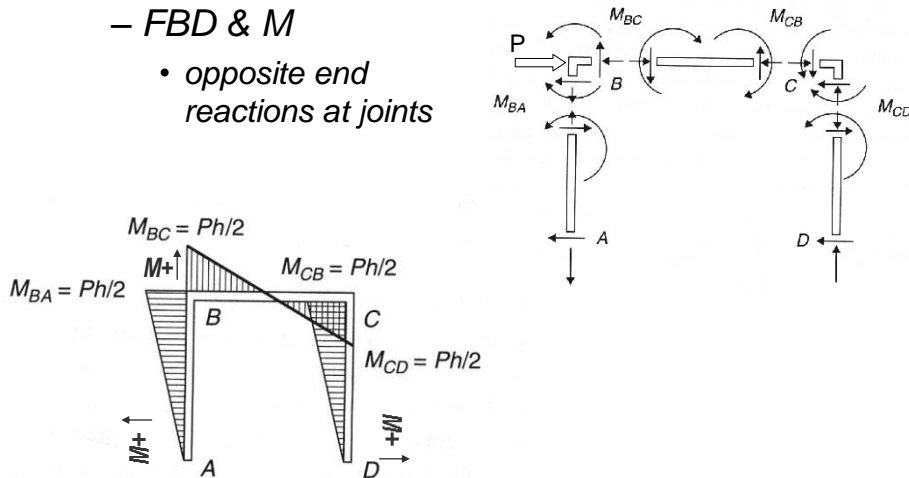
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Rigid Frame Analysis

– FBD & M

- opposite end reactions at joints



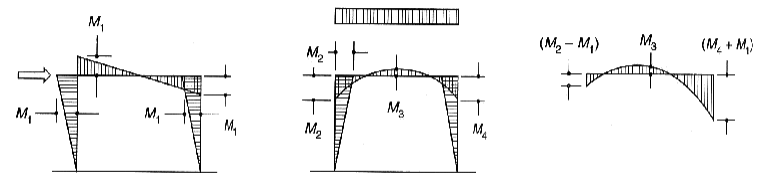
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Rigid Frame Design

- loads and combinations
 - usually uniformly distributed gravity loads
 - worst case for largest moments...
 - wind direction can increase moments



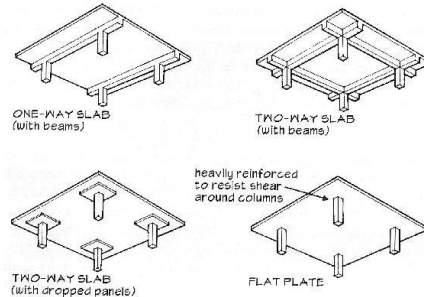
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Rigid Frame Design

- frames & floors
 - rigid frame can have slab floors or slab with connecting beams
- other
 - slabs or plates on columns



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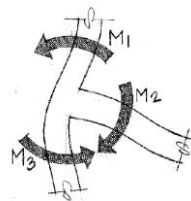
Rigid Frame Design

- columns in frames
 - ends can be "flexible"
 - stiffness affected by beams and column = EI/L

$$G = \Psi = \frac{\sum EI / l_c}{\sum EI / l_b}$$

for the joint

- l_c is the column length of each column
- l_b is the beam length of each beam
- measured center to center



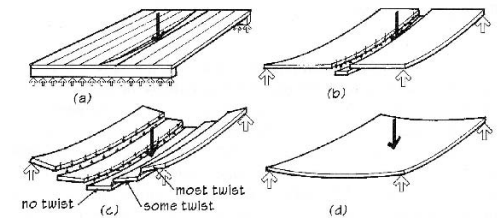
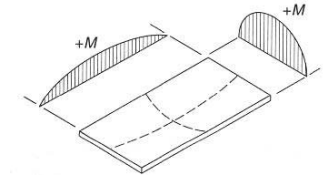
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Rigid Frame Design

- floors – plates & slabs
 - one-way behavior
 - side ratio > 1.5
 - "strip" beam
 - two-way behavior
 - more complex



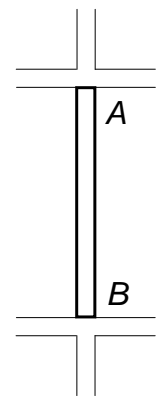
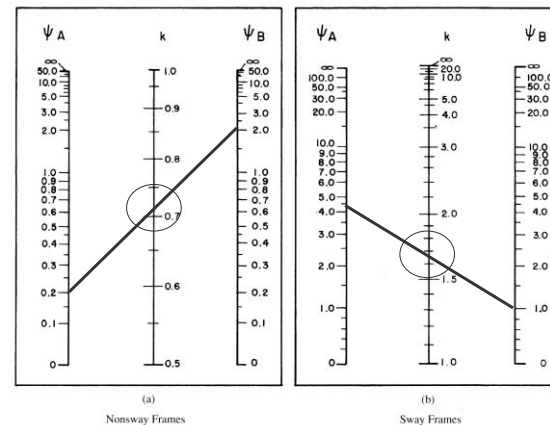
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Rigid Frame Design

- column effective length, k



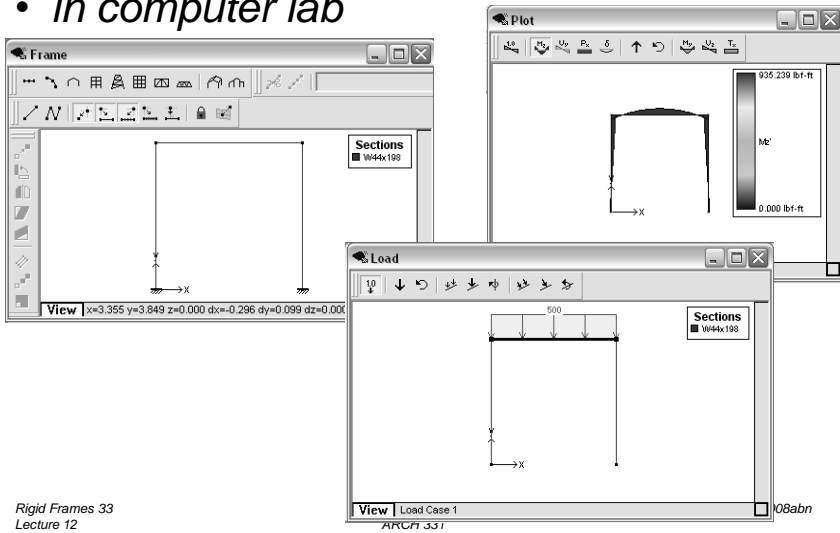
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Tools – Multiframe

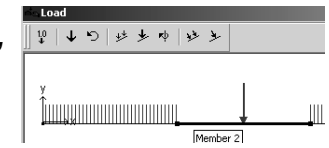
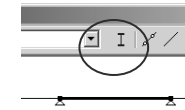
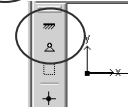
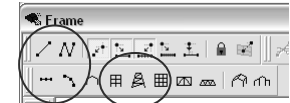
- in computer lab



Rigid Frames 33
Lecture 12

Tools – Multiframe

- frame window
 - define frame members
 - or pre-defined frame
 - select points, assign supports
 - select members, assign section
 - load window
 - select point or member, add point or distributed loads



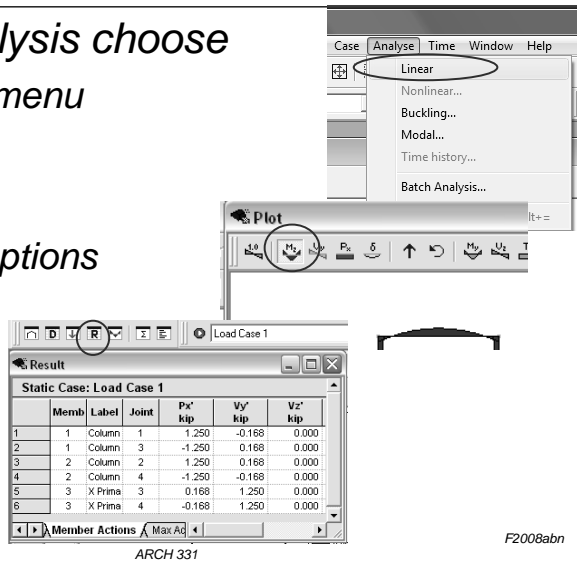
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Tools – Multiframe

- to run analysis choose
 - Analyze menu
 - Linear
- plot
 - choose options
- results
 - choose options



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