Architectural Structures: Form, Behavior, and Design

Arch 331 Dr. Anne Nichols Summer 2013

electure eleven



design loads, methods, structural codes & tracing

Design

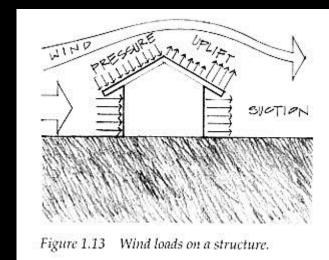
- factors out of the designer's control
 - loads
 - occurrence
- factors within the designer's control
 - choice of material
 - "cost" of failure (F.S., probability, location)
 - economic design method
 - analysis method

Design Methods

- different approaches to meeting strength/safety requirements
 - allowable stress design (elastic)
 - ultimate strength design
 - limit state design
 - plastic design
 - load and resistance factor design
- assume a behavior at failure or other threshold and include a margin of safety

Load Types

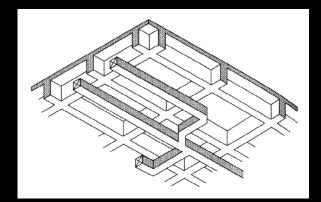
- D = dead load
- L = live load
- $L_r = live roof load$
- *W* = wind load
- S = snow load
- *E* = earthquake load



- *R* = rainwater load or ice water load
- *T* = effect of material & temperature
- *H* = hydraulic loads from soil (*F* from fluids)

Dead Loads

- fixed elements
 - structure itself
 - internal partitions
 - hung ceilings



- all internal and external finishes
- HVAC ductwork and equipment
- permanently mounted equipment
- F = mg (GRAVITY)

Weight of Materials

for a volume

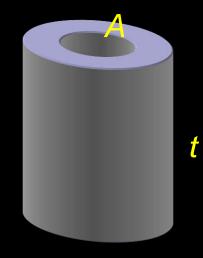
 $- W = \gamma V \quad \text{where } \gamma \text{ is weight/volume} \\ - W = \gamma t A \quad \text{for an extruded area with} \\ & \text{height of t} \end{cases}$

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Table 5.1 Selected building material weights.

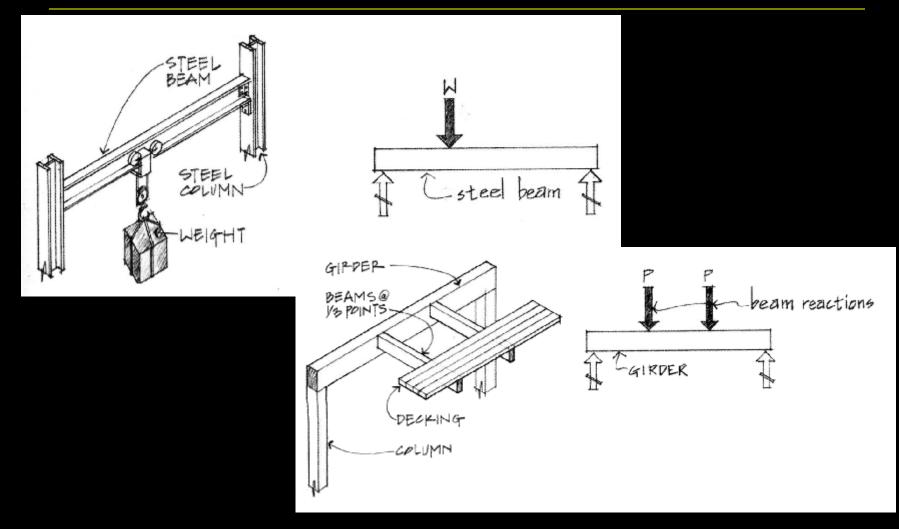
| lb./ _{ft.2} | ^{kN} /m ² |
|----------------------|--|
| | |
| 5.5 | 0.26 |
| 6.5 | 0.31 |
| 2 | 0.10 |
| 2 | 0.10 |
| 1 - 2.5 | 0.05-0.12 |
| 3/inch | 0.0057/mm |
| | |
| 0.5 | 0.0025 |
| 1.5 | 0.075 |
| | 5.5 6.5 2 2 1–2.5 3/inch 0.5 |

| Assembly | lb./ _{ft.2} | $\frac{kN}{m^2}$ |
|-----------------------------|----------------------|------------------|
| Floors: | | |
| Concrete plank | 6.5 | 0.31 |
| Concrete slab | 12.5/in. | 0.59/mm |
| Steel decking w/concrete | 35-45 | 1.68–2.16 |
| Wood joists | 2-3.5 | 0.10-0.17 |
| Hardwood floors | 4/in. | 0.19/mm |
| Ceramic tile w/thin set | 15 | 0.71 |
| Lightweight concrete | 8/in. | 0.38/mm |
| Timber decking | 2.5/in. | 0.08/mm |

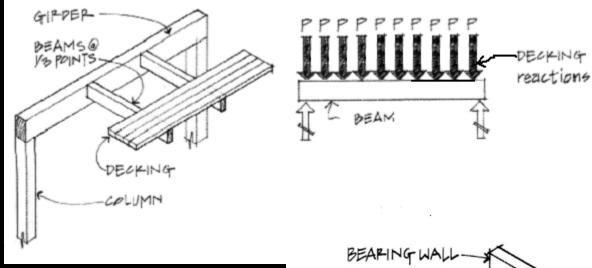


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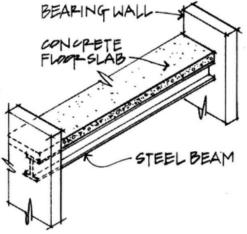
Concentrated Loads

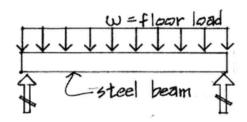


Distributed Loads



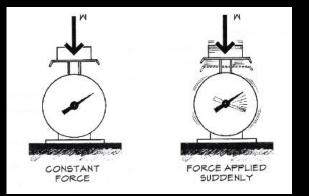
• for an area $w = \gamma A$

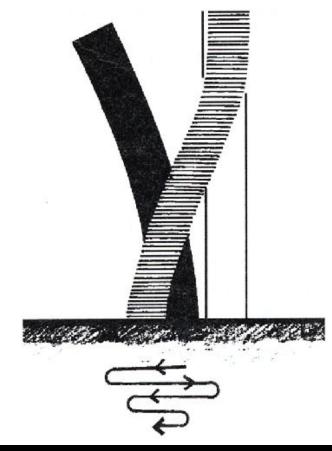




Dynamic Loads

- time, velocity, acceleration
- kinetics
 - forces causing motion $W = m \cdot g$
 - work
 - conservation of energy

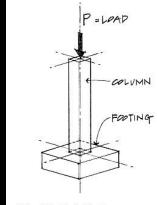




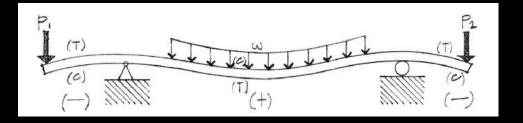
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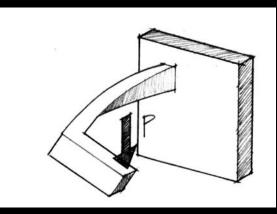
Load Locations

- centric
- eccentric
- bending or flexural load
- torsional load
- combined loading





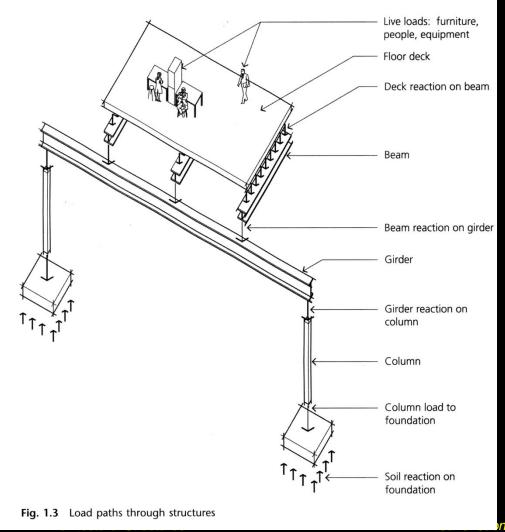




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Load Paths

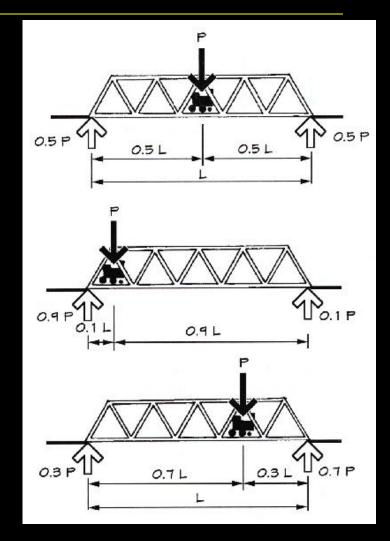
- tributary areas
- transfer



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Live Loads

- occupancy
- movable furniture and equipment
- construction / roof traffic – L_r
- minimum values
- reduction allowed as area increases

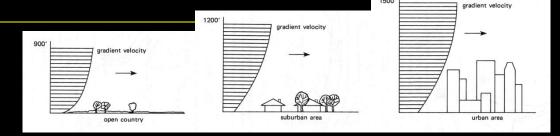


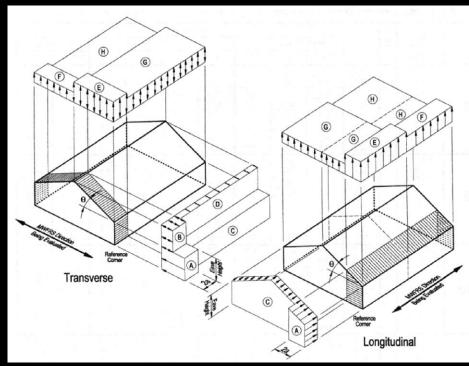
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Wind Load

- wind speed
- gusting
- terrain
- windward, leeward, up and down!
- drag
- rocking
- harmonic
- torsion





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Snow Load

- latitude
- solar exposure
- wind speed
- roof slope





Moscow 2006 (BBC News)

Seismic Load

- earthquake acceleration
 - -F = ma
 - movement of ground (3D)
 - building mass responds
 - static models often used,
 V is static shear
 - building period, T ≈ 0.1N,
 determines C
 - building resistance R_W

-Z (zone), I (importance factor)

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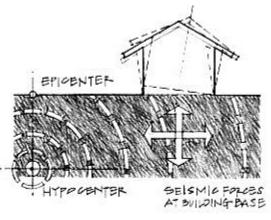
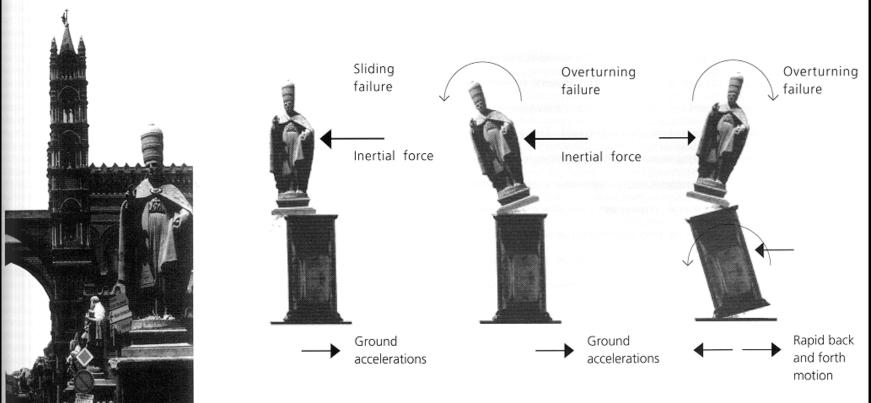


Figure 1.14 Earthquake loads on a structure.

 $V = \frac{ZICW}{R_{W}}$

Dynamic Response



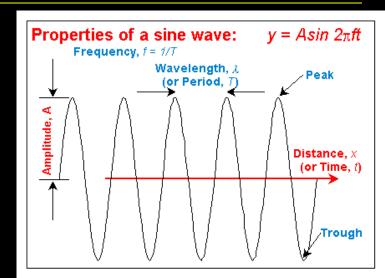
Statue in front of the cathedral of Palermo, Sicily

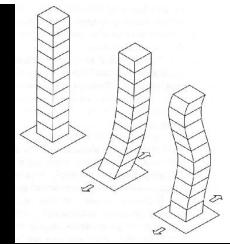
Lateral ground motions associated with earthquakes cause inertial forces to develop that are dependent on the weight of the structure. Sliding failures can occur. The lateral ground motions can also cause a sculpture to overturn. The magnitude of the overturning effect depends on the weight of the sculpture and its height above the ground. Back and forth ground motions can cause different parts of the sculpture to move in different directions. Overturning or cracking of elements can consequently occur.

Dynamic Response

- period of vibration or frequency
 - wave
 - sway/time period
- damping

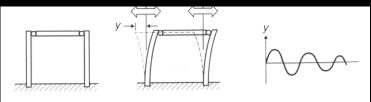
 reduction in sway
- resonance
 - amplification of sway





Frequency and Period

natural period of vibration



avoid resonance

- hard to predict seismic period
- affected by soil
- short period
 - high stiffness
- long period
 - low stiffness

"To ring the bell, the sexton must pull on the downswing of the bell in time with the natural frequency of the bell."

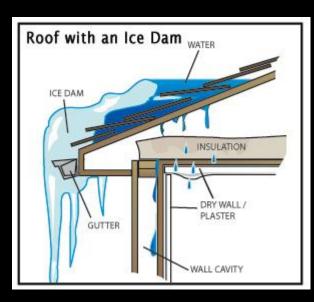
0

period

resonant Ioad

Water Load

- rainwater clogged drains
- ponding
- ice formation



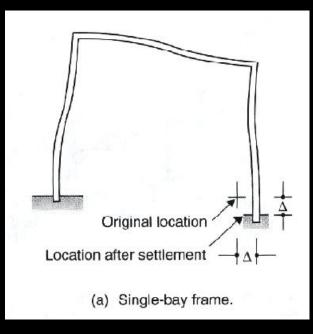


mrfussycontracting.com

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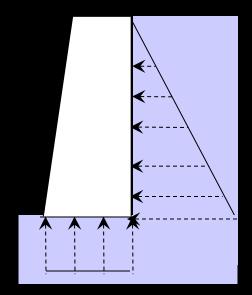
Thermal Load

- stress due to strain
- restrained expansion or contraction
- temperature gradients
- composite construction



Hydraulic Loads

- pressure by water in soil, H
- fluid pressure, F
 normal to surface
- flood



Building Codes

- documentation
 - laws that deal with planning, design, construction, and use of buildings
 - regulate building construction for
 - fire, structural and health safety
 - cover <u>all</u> aspect of building design
 - references standards
 - acceptable minimum criteria
 - material & <u>structural</u> codes

Building Codes

- occupancy
- construction types
- structural chapters

 loads, tests, foundations
- structural materials, assemblies
 - roofs
 - concrete
 - masonry
 - steel

| | r | |
|---|--|---|
| OCCUPANCY OR USE | UNIFORM (psf) | CONCENTRATED (lbs.) |
| Apartments (see residential) | _ | _ |
| Access floor systems | | |
| Office use | 50 | 2,000 |
| Computer use | 100 | 2,000 |
| Armories and drill rooms | 150 | |
| Assembly areas and theaters | | |
| Fixed seats (fastened to floor) | 60 | |
| Lobbies | 100 | |
| Movable seats | 100 | |
| Stages and platforms | 125 | |
| Follow spot, projections and control rooms | 50 | |
| Catwalks | 40 | |
| | Apartments (see residential) Access floor systems Office use Computer use Armories and drill rooms Assembly areas and theaters Fixed seats (fastened to floor) Lobbies Movable seats Stages and platforms Follow spot, projections and control rooms | OCCUPANCY OR USE(psf)Apartments (see residential)—Access floor systems—Office use50Computer use100Armories and drill rooms150Assembly areas and theaters—Fixed seats (fastened to floor)60Lobbies100Movable seats100Stages and platforms125Follow spot, projections and control rooms50 |

Prescribed Loads

- ASCE-7
 - live load (not roof) reductions allowed
- International Building Code
 - occupancy
 - wind: pressure to static load
 - seismic: shear load
 function of mass and
 response to acceleration



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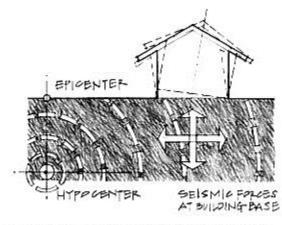


Figure 1.14 Earthquake loads on a structure.

Structural Codes

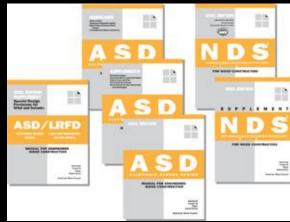
- prescribe loads and combinations
- prescribe design method
- prescribe stress and deflection limits
- backed by the profession
- may require design to meet performance standards
- related to material or function

Structural Codes

 Design Codes - Wood • NDS – Steel • AISC - Concrete • AC/ • AASHTO - Masonry • MSJC

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I by The Manoery Diamberts Joint Converting





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international

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Design Methods

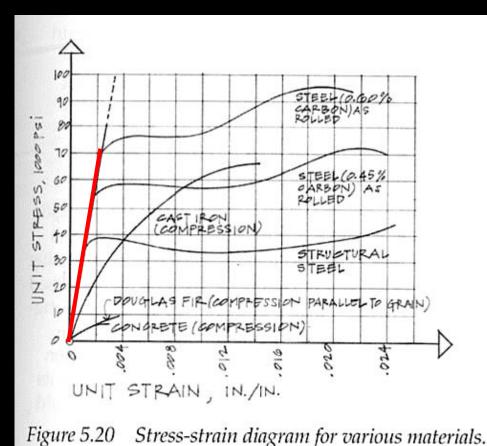
- probability of loads and resistance
- material variability
- overload, fracture, fatigue, failure
- allowable stress design

$$f_{actual} = \frac{P}{A} \le f_{allowed} = \frac{f_{capacity}}{F.S.}$$

limit state design design loads & capacities

Allowable Stress Design

- historical method
- a.k.a. working stress, strength design
- stresses stay in ELASTIC range



ASD Load Combinations





- $D + 0.75(L_r \text{ or } S \text{ or } R)$
- $D + 0.75L + 0.75(L_r \text{ or } S \text{ or } R)$
- D + (0.6W or 0.7E)- $D + 0.75L + 0.75(0.6W) + 0.75(L_r \text{ or } S \text{ or } R)$ - D + 0.75L + 0.75(0.7E) + 0.75S
- 0.6D + 0.6W
- 0.6D + 0.7E Methods & Codes 29 Lecture 11

Limit State Design

- a.k.a. strength design
- stresses go to limit (strain outside elastic range)
- loads may be factored
- resistance or capacity reduced by a factor
- based on material behavior
- "state of the art"

Limit State Design

- load and resistance factor design (LRFD)
 - loads:
 - not constant,
 - possibly more influential on failure
 - happen more or less often
 - UNCERTAINTY
 - $\gamma_D R_D + \gamma_L R_L \le \phi R_n$
 - ϕ Resistance factor
 - γ Load factor for (D)ead & (L)ive load

LRFD Load Combinations



- 1.4D
- $1.2D + 1.6L + 0.5(L_r \text{ or } S \text{ or } R)$
- $1.2D + 1.6(L_r \text{ or } S \text{ or } R) + (L \text{ or } 0.5W)$
- $1.2D + 1.0W + L + 0.5(L_r \text{ or } S \text{ or } R)$
- 1.2D + 1.0E + L + 0.2S
- 0.9D + 1.0W
- 0.9D + 1.0E
 - F has same factor as D in 1-5 and 7
 - H adds with 1.6 and resists with 0.9 (permanent)

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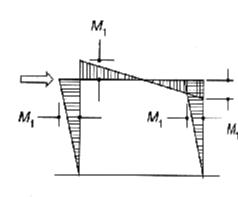
Deflection Limits

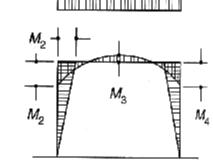
based on service condition, severity

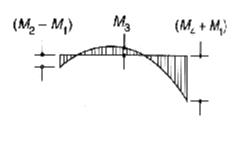
| Use | LL only | DL+LL |
|-------------------------------------|---------|-------|
| Roof beams: | | |
| Industrial | L/180 | L/120 |
| Commercial | | |
| plaster ceiling | L/240 | L/180 |
| no plaster | L/360 | L/240 |
| Floor beams: | | |
| Ordinary Usage | L/360 | L/240 |
| Roof or floor (damageable elements) | | L/480 |

Load Conditions

- loads, patterns & combinations
 - usually uniformly distributed gravity loads
 - worst case for largest moments...
 - wind direction can increase moments

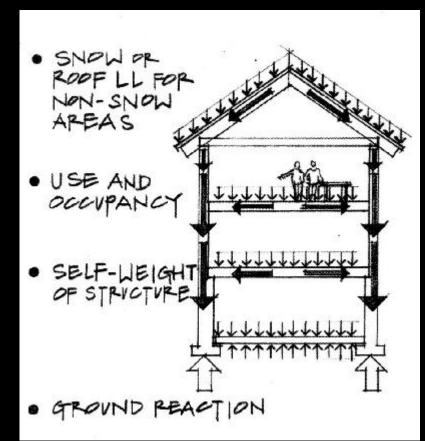






Structural Loads

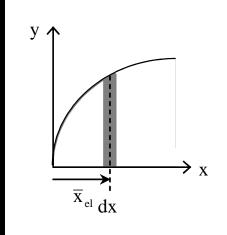
- gravity acts on mass (F=m*g)
- force of mass
 - acts at a point
 - ie. joist on beam
 - acts along a "line"
 - *ie. floor on a beam*
 - acts over an area
 - *ie. people, books, snow on roof or floor*

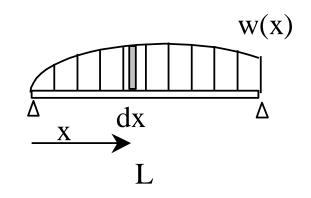


Equivalent Force Systems

- replace forces by resultant
- place resultant where M = 0
- using <u>calculus</u> and area centroids

$$W = \int_0^L w dx = \int dA_{\text{loading}} = A_{\text{loading}}$$





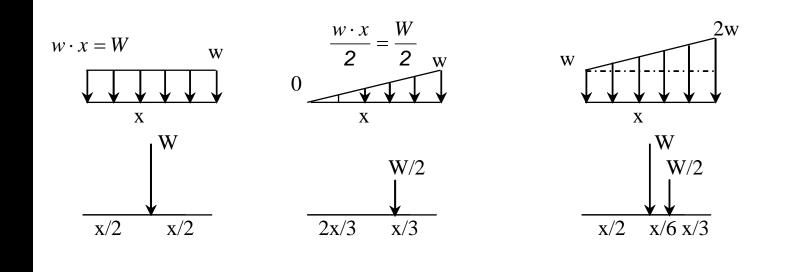
Area Centroids

• *Table 7.1 – pg. 242*

| Centroids of Common Shapes of Areas and Lines | | | |
|---|---|-----------------------------------|-------------------|
| Shape | | x | У |
| Triangular area | $\frac{1}{\sqrt{y}}$ | $\frac{b}{3}$ right triangle only | $\frac{h}{3}$ |
| Quarter-circular area | $\begin{array}{c} C \\ O \\ \hline \overline{x} \\ \hline \overline{x} \end{array} \leftarrow \begin{array}{c} C \\ \hline \overline{y} \\ O \\ \hline O \\ \hline \end{array}$ | $\frac{4r}{3\pi}$ | $\frac{4r}{3\pi}$ |
| Semicircular area | | 0 | $\frac{4r}{3\pi}$ |
| Semiparabolic area | $C \bullet \bullet C$ $C \bullet \bullet C$ h $C \bullet \bullet C$ h | $\frac{3a}{8}$ | $\frac{3h}{5}$ |
| Parabolic area | | 0 | $\frac{3h}{5}$ |

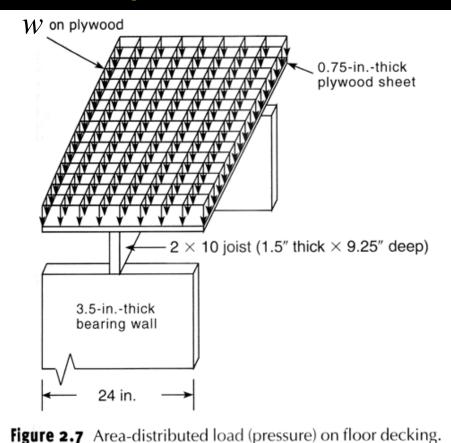
Equivalent Load Areas

- area is width x "height" of load
- <u>w</u> is load per unit length
- W is total load



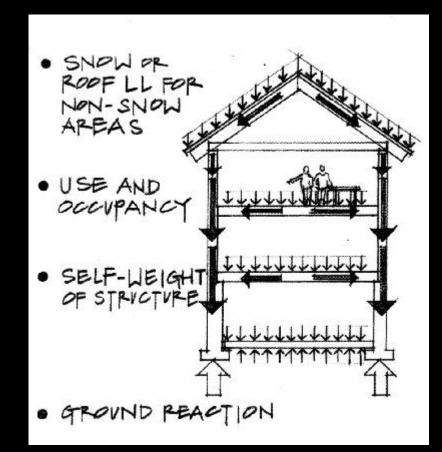
Distributed Area Loads

• w is also load per unit area



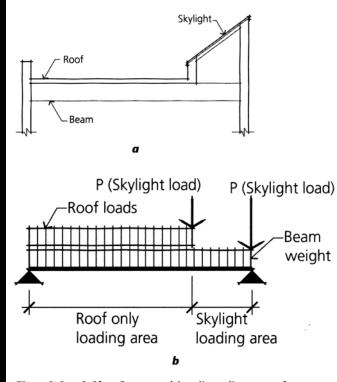
Load Tracing 39 Lecture 11

- how loads are transferred
 - usually starts at top
 - distributed by supports as <u>actions</u>
 - distributed by <u>tributary areas</u>

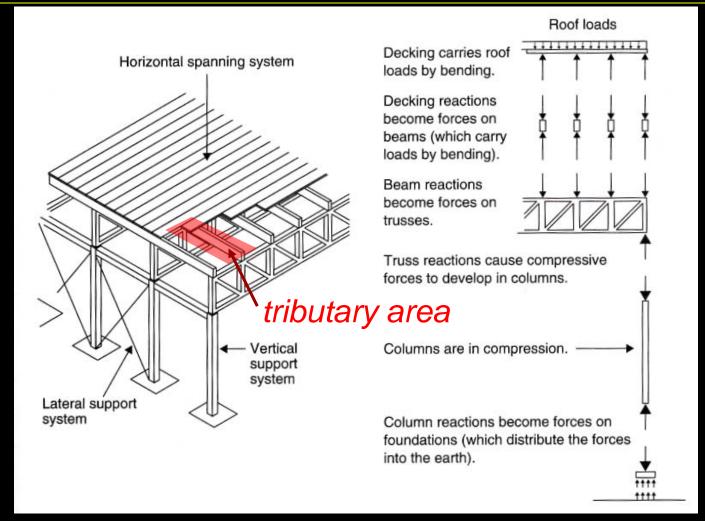


- areas see distributed area load
- beams or trusses see distributed line loads
- "collectors" see forces

 columns
 - supports

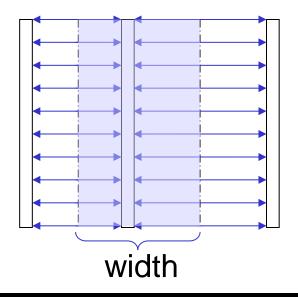


Figs. 1.1a, 1.1b Structural loading diagram of an architectural condition



- tributary load
 - think of water flow
 - "concentrates" load of area into center

$$w = \left(\frac{load}{area}\right) \times \left(tributary \ width\right)$$



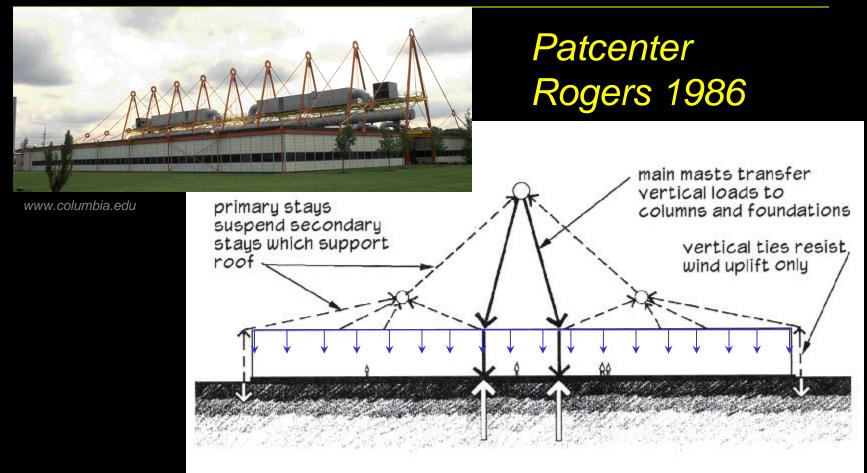


Figure 3.5: Patcenter, load path diagram.

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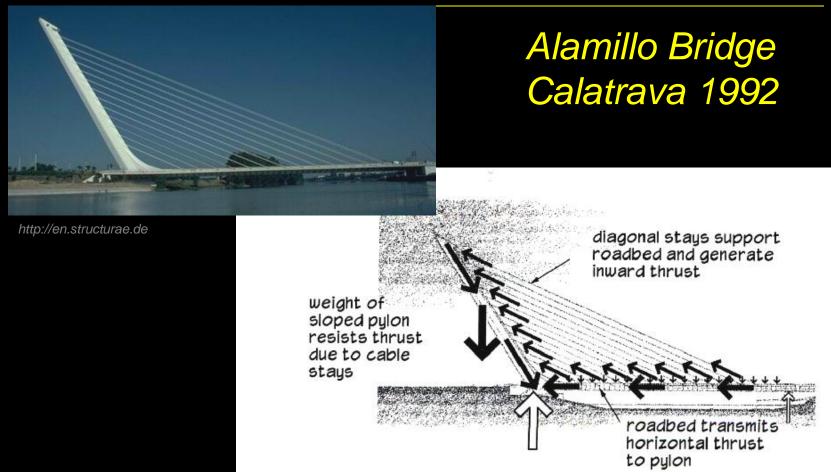
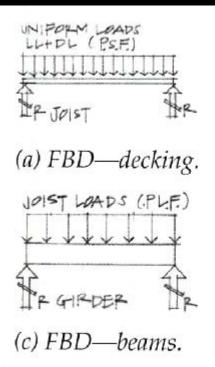
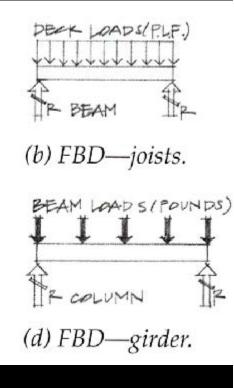
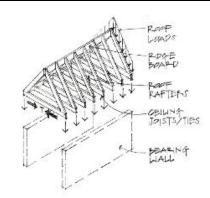


Figure 3.12: Alamillo bridge, load path diagram.

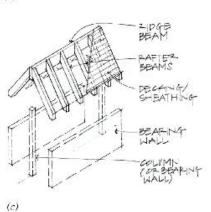
floors and framing

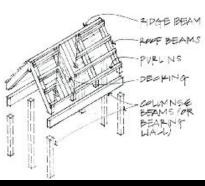






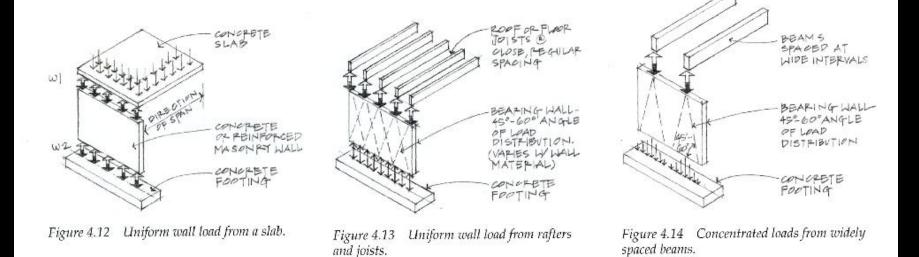
(a)





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wall systems



Load Tracing 47 Lecture 11

openings & pilasters

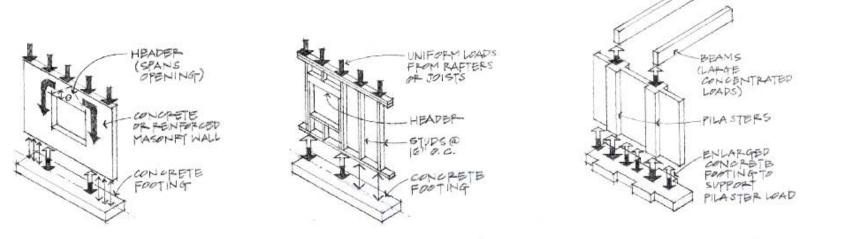


Figure 4.15 Arching over wall openings.

Figure 4.16 Stud wall with a window opening.

Figure 4.17 Pilasters supporting concentrated beam loads.

foundations

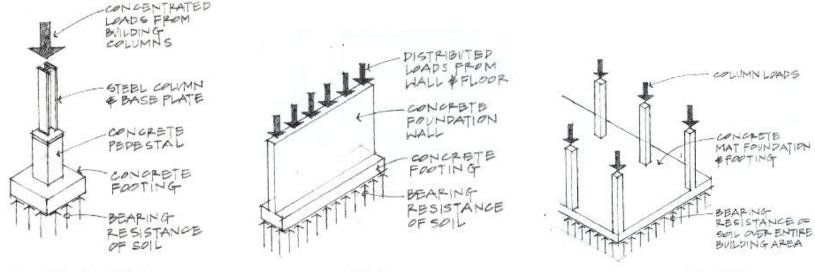
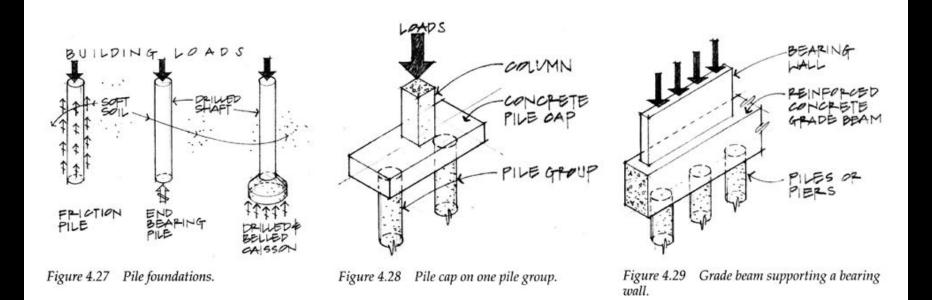


Figure 4.24 Spread footing.

Figure 4.25 Wall footing.

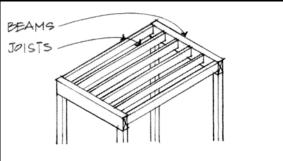
Figure 4.26 Mat or raft foundation.

deep foundations

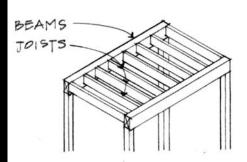


Spans

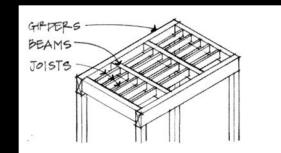
- direction
- depth



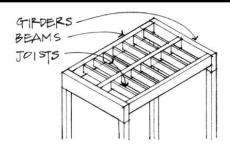
(a) Long, lightly loaded joists bearing on shorter beams create a more uniform structural depth. Space can be conserved if the joists and beams are flush framed.



(b) Short joists loading relatively long beams yield shallow joists and deep beams. The individual structural bays are more clearly expressed.



(c) Loads can be reduced on selected beams by introducing intermediate beams.

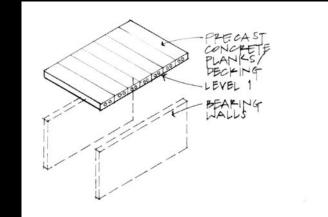


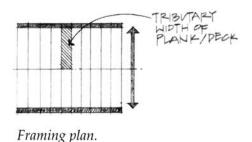
(*d*) The span capability of the decking material controls the spacing of the joists, while beam spacing is controlled by the allowable joist span.

Levels

- determine span at top level
- find half way to next element
- *include self weight
- look for "collectors"
- repeat

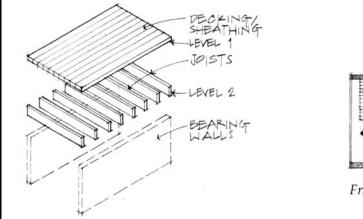
one:

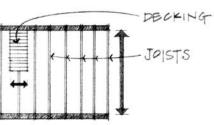




Levels

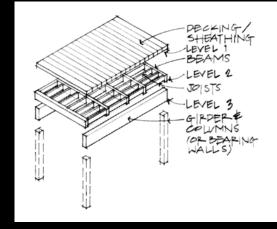
• *two:*

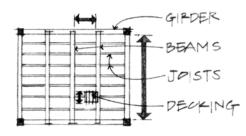




Framing plan.

• three:



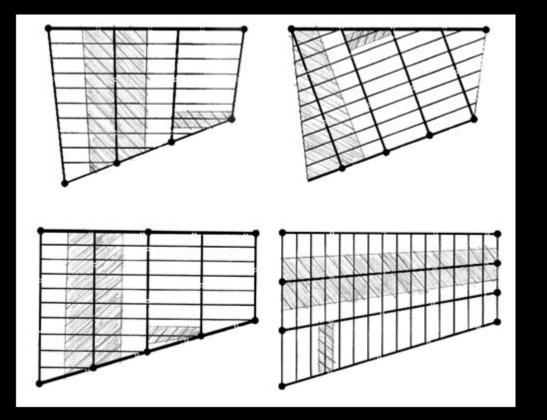


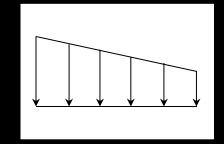
Framing plan.

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Irregular Configurations

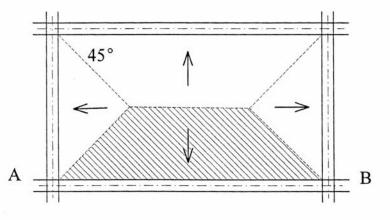
tracing still ½ each side

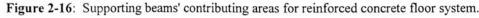




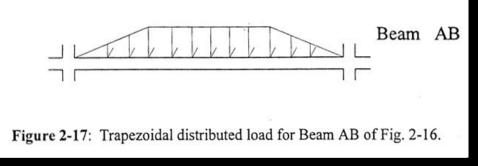
Slabs

edge support



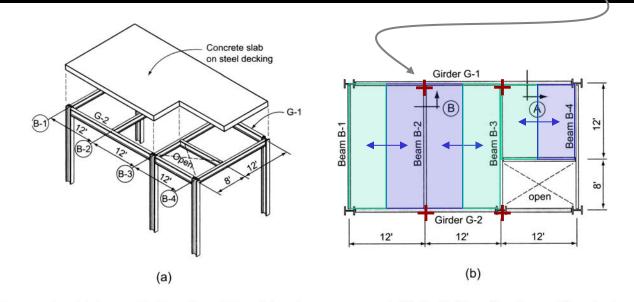


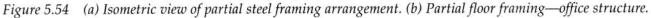
linear and uniform distribution



Girders and Transfer

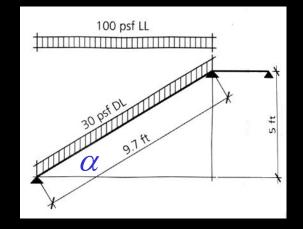
- openings
 - no load & no <u>half way</u>
- girder actions at beam supports -





Sloped Beams

- stairs & roofs
- projected live load
- dead load over length



• perpendicular load to beam:

 $w_{\perp} = w \cdot c \, o \, s \, \alpha$

• equivalent distributed load:

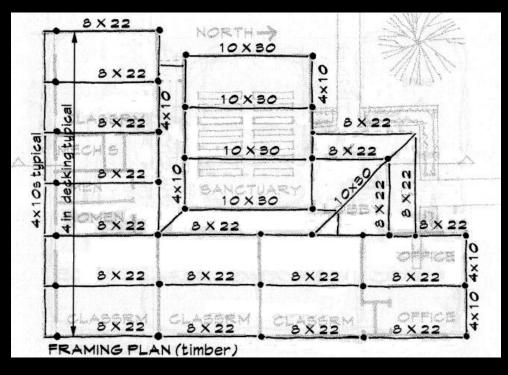
Wadj

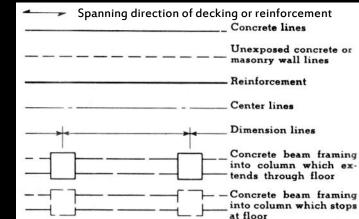
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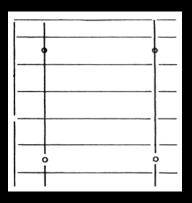
COS A

Framing Diagrams

- beam lines and "dots"
- breaks & ends

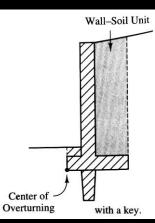


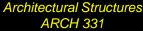


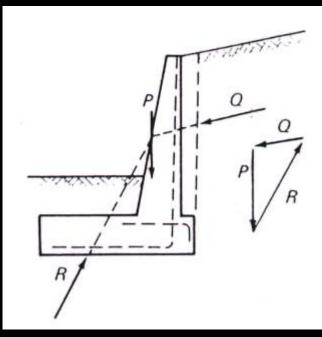


Retaining Walls

- purpose
 - retain soil or other material
- basic parts
 - wall & base
 - additional parts
 - counterfort
 - buttress
 - key

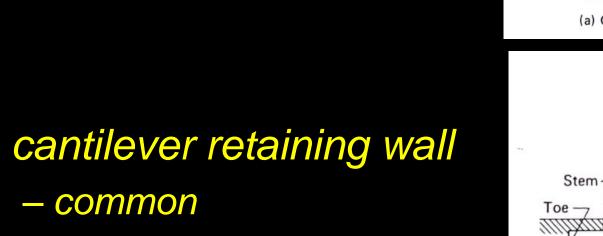


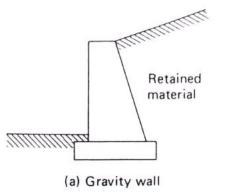


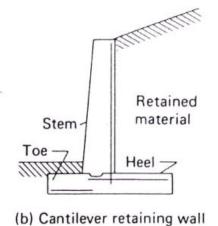


Retaining Wall Types

"gravity" wall
 usually unreinforced
 economical & simple







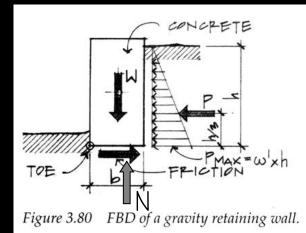
•

Retaining Wall Loads

- gravity $W = \gamma \times V$
- fluid pressure $p = \omega' \times h$ $P = \frac{1}{2} p h at h/3$
- friction

 $F = \mu \times N$

• soil bearing pressure, q



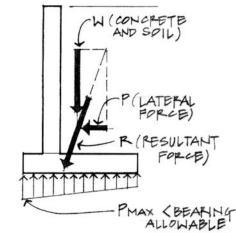
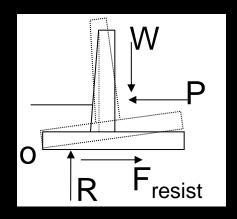


Figure 3.81 Bearing pressure under the wall footing.

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Retaining Wall Equilibrium

- sliding overcome friction?
- overturning at toe (o) overcome mass?



$$SF = \frac{M_{resist}}{M_{overturning}} \ge 1.5 - 2$$
$$SF = \frac{F_{horizontal-resist}}{F_{sliding}} \ge 1.25 - 2$$

Pressure Distribution

- want resultant of load from pressure inside the middle third of base (kern)
- triangular stress block with p_{max}
- x = 1/3 x width of stress
- equivalent force location:

$$W \cdot x = \frac{p_{max} 3x}{2} \cdot \frac{x}{3}$$
$$p_{max} = \frac{2W}{3x} = \frac{2W}{a}$$
 when a is fully stressed

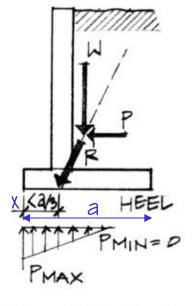


Figure 3.88 Tension possible at the heel.

Wind Pressure

- distributed load
- "collected" into V
- lateral loads must be resisted

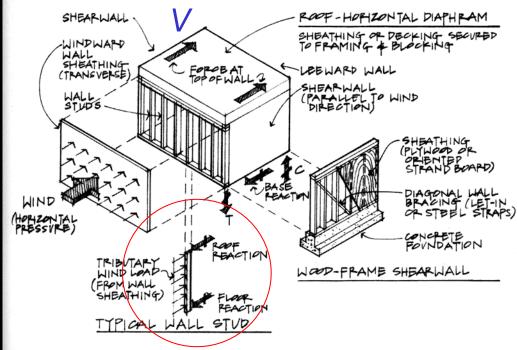


Figure 4.48 Exploded view of a light-framed wood building showing the various lateral resisting components.

Bracing Configurations

